

ASPEN HILLS REDEVELOPMENT

Development of Regional Impact # 1353

COBB COUNTY, GA

TRAFFIC IMPACT STUDY

Prepared for:

Aspen Hills Redevelopment
3328 Peachtree Road, NE, Suite 300
Atlanta, Georgia 30326

Prepared by:



A&R Engineering Inc.

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May 18, 2007
A&R Project No: 07-017

EXECUTIVE SUMMARY

The purpose of this study is to determine the traffic impact that will result from the Aspen Hills Redevelopment proposed along South Cobb Drive (SR 280) to the southeast of the intersection of South Cobb Drive / Church Road / Kenwood Road / Oakdale Road, in Cobb County, Georgia. The proposed development will consist of 431,720 s.f of retail space. The site is currently occupied by the Aspen Hills Apartment complex and a gas station which will be demolished as part of the project. The traffic analysis evaluated the following scenarios: existing conditions, the year 2009 without additional traffic generated by the site, and the year 2009 with the traffic generated by the development.

From the existing condition analysis it was found that three intersections within the study area are not currently operating at the LOS standard of D. Analysis of the Base Year 2009 also revealed that three study intersections being analyzed will not meet the required LOS standard.

The Future 2009 conditions including the site-generated traffic were then evaluated using existing lane geometry. Four study network intersections will not meet the required LOS standard after the project is completed if no roadway improvements are implemented. Improvements were recommended to restore those intersections back the LOS standard. Additionally, recommendations to allow the site accesses to operate satisfactorily were identified. Details can be found in the site access analysis section of the report.

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1. PROJECT DESCRIPTION

The purpose of this study is to determine the traffic impact that will result from the Aspens Hills Redevelopment proposed along South Cobb Drive (SR 280) to the southeast of the intersection of South Cobb Drive / Church Road / Kenwood Road / Oakdale Road in Cobb County, Georgia. The proposed development will consist of 431,720 s.f of retail space. An existing gas station driveway that aligns across from Highland Parkway will serve as the primary access for the site. The gas station is proposed to be closed and demolished as part of the redevelopment. The site will also have right-in / right-out access along South Cobb Drive and three full access driveways along Church Road. The right in / right out access along South Cobb Drive currently serves the Aspen Hills Apartment Complex. The Apartment complex also has one access along Church Road. A location map for the site is shown in Figure 1.

1.1 Site Plan

A site plan for this project is shown in Figure 2. A larger size drawing and a digital copy of the site plan are also provided with this report.

1.2 Consistency with Adopted Comprehensive County Plan

The Aspen Hills Redevelopment DRI site is made up of four tracts. The main large tract is zoned RM-12 (residential multi-family) with two smaller tracts of TS (tourist services) and one tract of GC (General Commercial). The site will require rezoning to General Commercial to accommodate the proposed project.

1.3 Project Phasing

The project's impact will be evaluated in one phase, estimated for completion in the year 2009. This study will evaluate the traffic operations in the vicinity of the site for existing conditions year 2007, the year 2009 without additional traffic generated by the site, and the year 2009 with the additional traffic generated by the development.



LOCATION MAP

FIGURE 1

A&R Engineering Inc.



770.394.1616 fax 770.394.1314

CONSULTANT_____

SEAL _____

[illegible]

ASPEN HILLS

SOUTH COBB DRIVE
SMYRNA, GEORGIA

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0717701

ISSUE DATE: 05/16/07

DRAWN BY/CHECKED BY: _____
COMPILED BY: _____

DRAWING TITLE_____

PRELIMINARY

SITE PLAN

SHEET NUMBER _____

2000

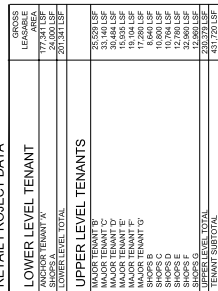
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PARKING			
LEVEL	LOWER	MIDDLE	UPPER
OVERALL SPACES	741	516	780

TOTAL SPACES / RATIO	
LOWER LEVEL (INCLUDE 10 MEN)	999 / 4.96
UPPER LEVEL (INCLUDE 10 MEN)	1,008 / 4.50
PROJECT TOTAL	2,037 / 4.72



SITE LOCATION MAP

NOTE
TOPOGRAPHICAL INFO IS EXISTING
NEW TOPOGRAPHICAL TO BE DETERMINED

2. TRIP GENERATION

Trip generation estimates for the project were based on the rates and equations published in the 7th edition of the Institute of Transportation Engineers (ITE) Trip Generation report. The ITE Trip Generation report contains traffic volume count data collected at similar facilities nationwide. The proposed development will consist of 431,720 s.f of retail space. Trip generation calculations for the project are shown in Table 1.

TABLE 1 TRIP GENERATION								
Land Use	Total Size	P.M. Peak Hour			SAT Peak Hour			24-Hour 2-way
		Enter	Exit	Total	Enter	Exit	Total	
820 – Shopping Center	431,720 s.f.	789	855	1,644	1,165	1,075	2,240	17,572

2.1 Net Trip Ends

Due to the nature of the development Pass-by reductions were applied. Pass-by reductions have been taken for the PM and SAT peak hours based on the equations published in ITE Trip Generation Handbook. In addition to the pass-by reductions, as per the GRTA LOU, 2% transit reduction has also been applied since the area is served by Cobb Community Transit. The redevelopment includes the closure and demolition of the existing Aspen Hills Apartment Complex and a gas station. Accordingly, the volumes being generated by these facilities has been deducted from the trip generation estimates for the project to determine the net trips generated by the site. Trip Generation with the applied reductions is shown in the Table 2.

TABLE 2 TRIP GENERATION WITH REDUCTIONS								
Land Use	Total Size	P.M. Peak Hour			SAT Peak Hour			24-Hour 2-way
		Enter	Exit	Total	Enter	Exit	Total	
820 - Shopping Center – 431,720 s.f.		789	855	1,644	1,165	1,075	2,240	17,572
- Pass-By Reduction (25% / 28%)*		-197	-214	-411	-326	-301	-627	-4,110**
- CCT Transit Reduction 2%		-12	-13	-25	-17	-15	-32	-269
Existing Apartment Trips		-114	-108	-222	-71	-71	-142	2,406***
Existing Gas Station Trips		-67	-114	-181	-93	-102	-195	2,202***
Total without reductions		789	855	1,644	1,165	1,075	2,240	17,572
Total with reductions		399	406	805	658	586	1,244	8,585

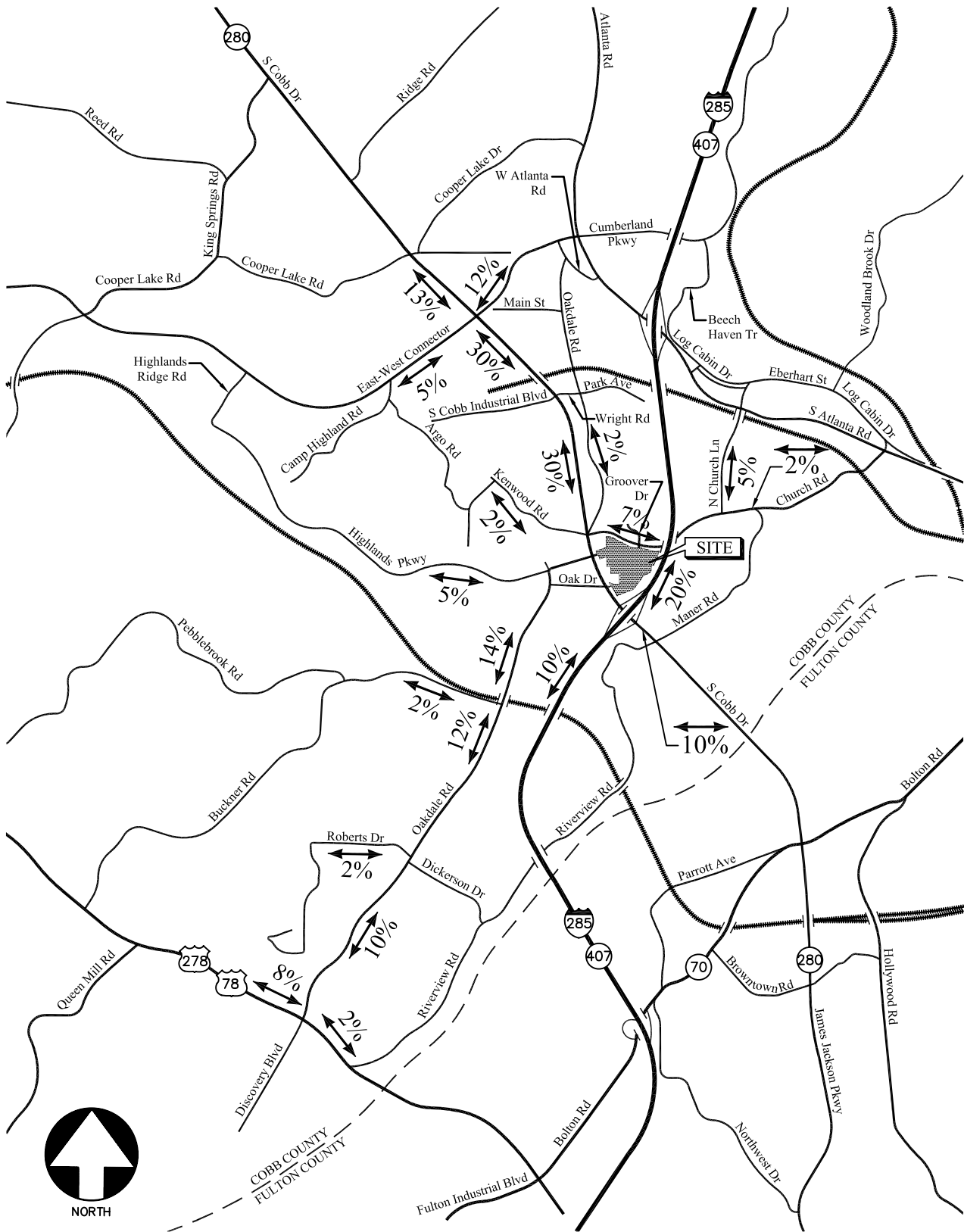
* (PM Pass-by % / SAT Pass-by %)

** The 24 hour pass by reduction has been calculated by assuming the total PM peak hour pass by trip reduction is 10% of the total daily pass-by reduction.

*** The 24-hour trips generated by these uses has been estimated by analyzing the ITE Trip generation rates for PM and 24-hours. The total PM volumes have been increased on this relationship to estimate the 24-hour volumes.

3. TRIP DISTRIBUTION & ASSIGNMENT

The trip distribution is the percentage of the traffic generated by the site that travels to and from the site on each segment of the surrounding roadway network. The trip distribution was based on the location of major roadways, highways and residential concentrations that will serve the development. The trip distribution is shown in Figure 3. The distribution was discussed and agreed upon in the methodology meeting. The site-generated volumes were then distributed to the surrounding roadway network based on the driver's destination, and the most easily accessible route.



TRIP DISTRIBUTION

FIGURE 3

4. STUDY NETWORK DETERMINATION

The study network was determined by evaluating the amount of traffic that the proposed development will add to each roadway segment in the area. According to GRTA requirements, a roadway segment carries a “significant” amount of traffic if the project contributes 7% or more trips to the two-way daily service volumes of the roadway at the appropriate level of service standard. Upon agreement with GRTA a level of service standard of “D” was used for determining the study area network.

The traffic generated by the proposed project was then assigned to the area roadways using the trip distribution to determine the site-generated traffic on each roadway segment. The boundaries of the study network extend to the most distant intersections where at least 7% of the service volumes on the segment are attributed to project traffic. The following intersections fell within the 7% rule and have been included in the traffic study:

- 1) South Cobb Drive / East – West Connector / Cumberland Parkway
- 2) South Cobb Drive / Church Road / Kenwood Road / Oakdale Road
- 3) South Cobb Drive / Highlands Parkway / Gas Station Driveway
- 4) South Cobb Drive / I-285 Northbound Ramps
- 5) South Cobb Drive / I-285 Southbound Ramps
- 6) Church Road / N Church Lane
- 7) Highlands Parkway / Oakdale Road
- 8) Oakdale Road / Veterans Memorial Parkway (US 278)
- 9) South Cobb Drive / Oak Drive / Tibarron Parkway
- 10) Church Road / Groover Drive / Tibarron Parkway

The study intersections are shown graphically in Figure 4. Other intersections within this corridor, such as unsignalized side streets, right-in / right-out driveways or private driveways were viewed as insignificant and have not been included in the study network. In addition to the above intersections, two site driveways along Church Road have been included in the analysis during the P.M. and SAT peak hours as agreed upon in the methodology meeting.

LEGEND

○ - Study intersections



STUDY INTERSECTIONS

FIGURE 4

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5. PLANNED & PROGRAMMED IMPROVEMENTS

The following improvements have been identified in the Atlanta Regional Commission's Transportation Improvement Program (TIP) and Regional Transportation Plan (RTP). These improvements are within the vicinity of the proposed development. Additional improvements for Cobb County have been identified, but they are not relevant to this project. Details of the planned programs can be found in the Appendix.

- CO-175A: SR 280 (South Cobb Drive) from SR 5 (Atlanta Road) in Cobb County to SR 70 (Bolton Road) in City of Atlanta
 - Includes widening of SR 280 (South Cobb Drive) from four lanes to six lanes.
- CO-328: Cumberland Parkway from SR 280 (South Cobb Parkway) to Atlanta Road
 - Includes widening of Cumberland Parkway from four lanes to six lanes.
- CO-AR-070B: I-285 West at Atlanta Road Bridge
 - Includes Construction of an interchange at I-285 West and East West Connector.

6. EXISTING CONDITIONS

An inventory was performed of the roadways in the area surrounding the site. The following is a brief description of each of these facilities.

6.1 Description of Transportation Facilities in Study Network

I-285

I-285 is an eight lane (four lanes in each direction) north-south interstate facility to the east of the proposed development.

SR 280 (South Cobb Drive)

SR 280 is a north-south six-lane undivided roadway with a posted speed limit of 45 mph in the vicinity of the site.

US 278 (US 78)

US 278 (US 78) is an east-west four-lane undivided roadway with a posted speed limit of 45 mph.

Cumberland Parkway

Cumberland Parkway is a four-lane undivided roadway with a posted speed limit of 45 mph.

Church Road

Church Road is an east-west two-lane roadway with a speed limit of 35 mph in the vicinity of the site. It runs between Atlanta Road in the east and South Cobb Drive in the west.

Oakdale Road

Oakdale Road is a north-south two-lane undivided roadway with a speed limit of 35 mph.

Kenwood Road

Kenwood Road is an east-west two-lane roadway with a posted speed limit of 25 mph. It runs from South Cobb Drive in the east and terminates into a dead end approximately 100 feet beyond Crowe Drive in the west.

Highlands Parkway

Highlands Parkway is a four-lane divided roadway with a speed limit of 45 mph. It extends between East-West Connector and South Cobb Drive.

Church Lane

Church Lane is a north-south two-lane roadway with a speed limit of 35 mph. It runs between Eberhart Street in the north and Church Road in the south.

Oak Drive

Oak Drive is an east-west two-lane roadway with a speed limit of 25 mph. It runs between South Cobb Drive in the east and Oakdale Road in the west.

Buckner Road

Buckner Road is a two-lane roadway. It runs between US 78 (US 278) and Oakdale Road.

Roberts Drive

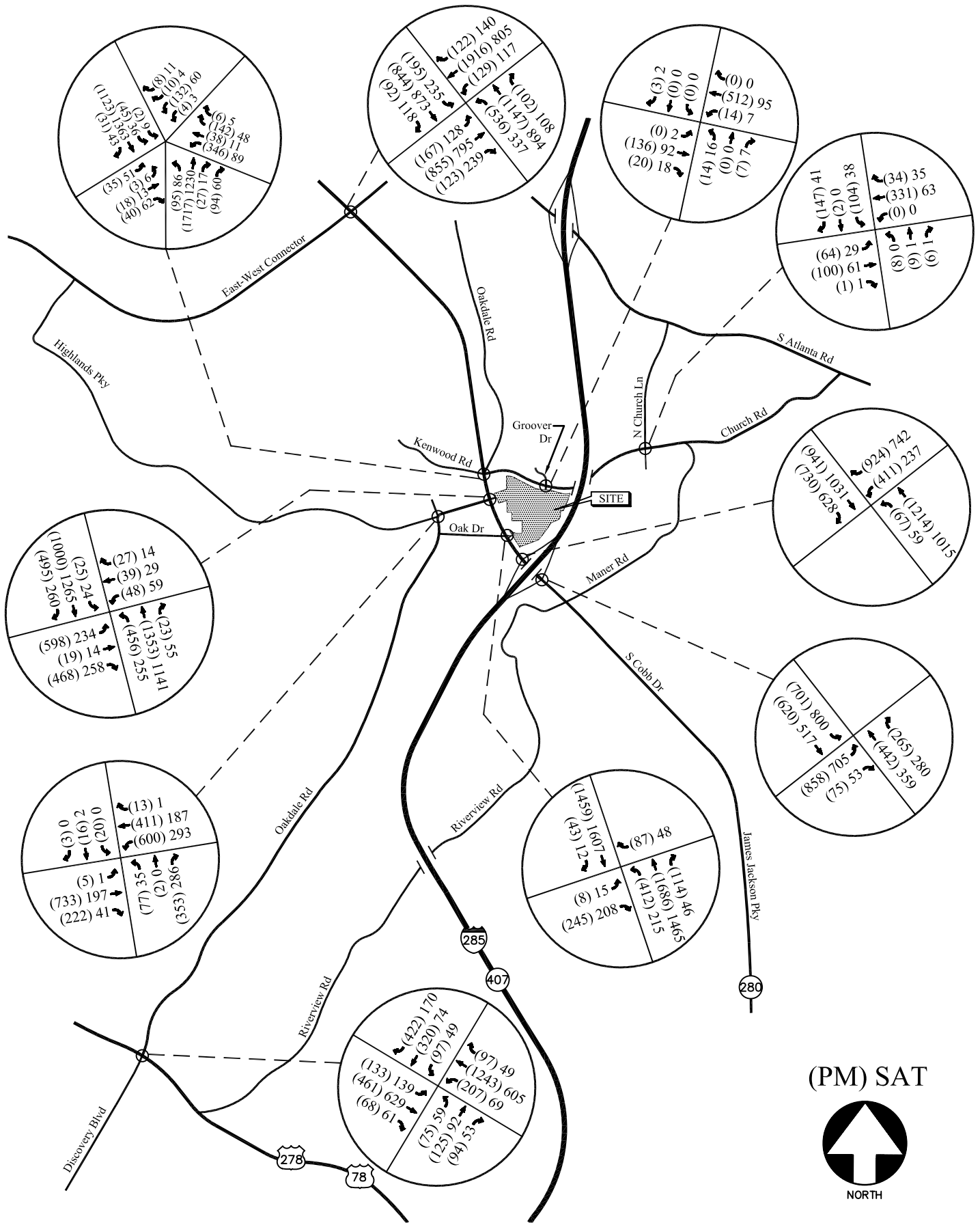
Roberts Drive is a two-lane roadway. It runs between Oakdale Road in the east and Brookside Drive in the west.

6.2 Analysis Summary

Existing traffic counts were collected at the intersections listed in Section 4 - Study Area Network. In addition to traffic counts, intersection geometry data was also obtained. Turning movement counts were collected during the agreed upon hours of 4:00 PM – 6:00 PM on weekdays and 12:00 PM – 2:00 PM on Saturday. The four consecutive 15-minute interval volumes that summed to produce the highest volume at each intersection during each two-hour period were then determined. These volumes make up the P.M. and Saturday peak hour traffic volumes for the intersections counted. The existing traffic volumes are shown in Figure 5 and the existing intersections traffic control and lane geometry for the study area network is shown in Figure 6.

The existing signal phasing and timing information for the PM and Saturday peak hours was obtained from the City of Smyrna and Cobb County for five signalized intersections along South Cobb Drive (I-285 NB Ramps, I-285 SB Ramps, Highland Parkway, Church Road / Kenwood Road, Cumberland Parkway / E-W Connector). This information was used to analyze the traffic operations at these five signalized intersections.

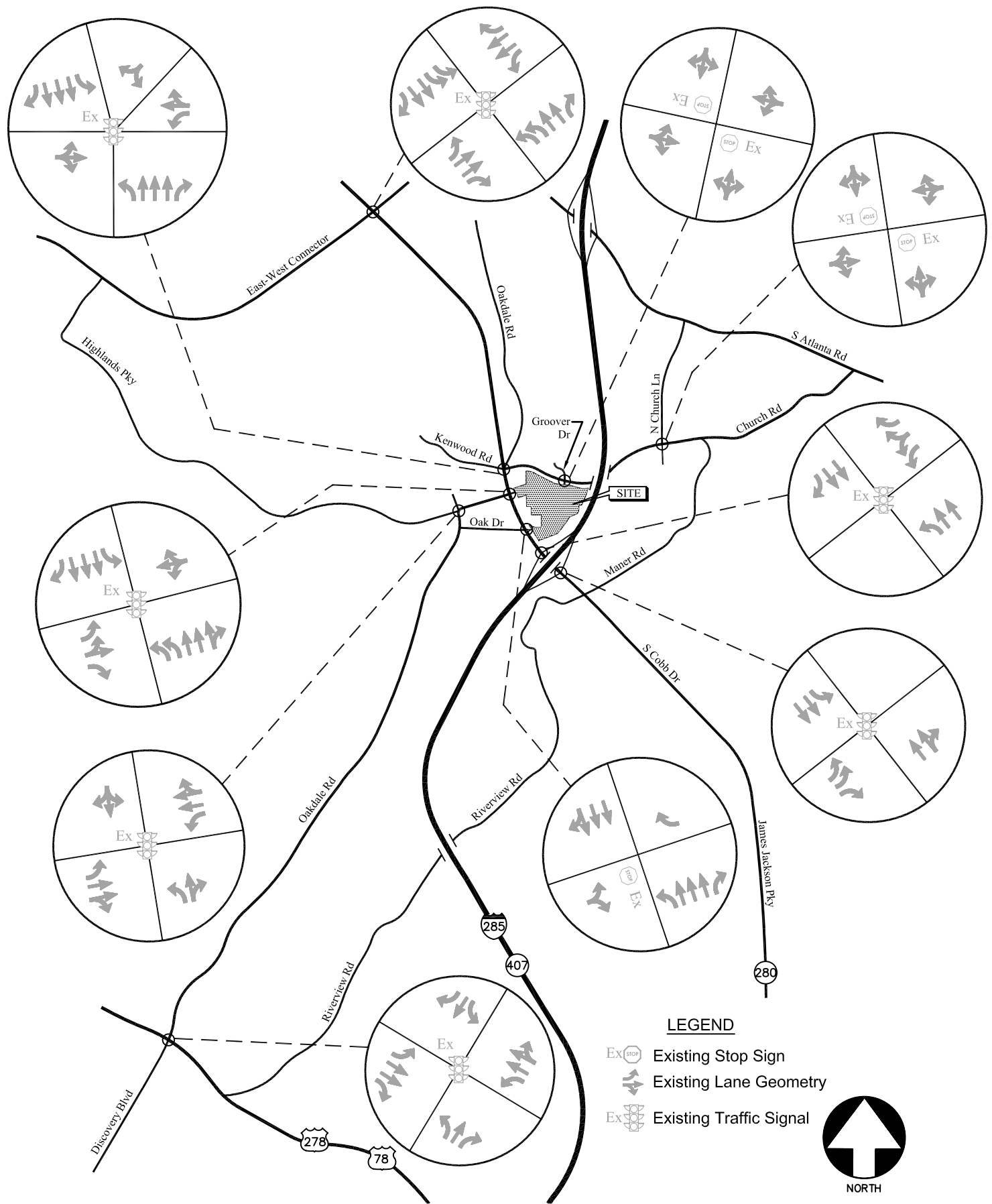
The site-generated volumes shown in Table 2 were distributed to the surrounding roadway network in accordance with the trip distribution. The site-generated volumes for the study intersections are shown in Figure 7. Figure 7 includes all the traffic that will be generated by the proposed development. It does not show the net effect of deducting the existing traffic that is currently being generated by the apartment complex and adjacent gas station both of which will be closed and demolished as part of the redevelopment. Existing traffic operations were analyzed at all the existing intersections in accordance with the HCM methodology using Synchro software. The results of the analysis are shown in Table 3.



EXISTING WEEKDAY PEAK-HOUR VOLUMES

FIGURE 5

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EXISTING TRAFFIC CONTROL AND LANE GEOMETRY

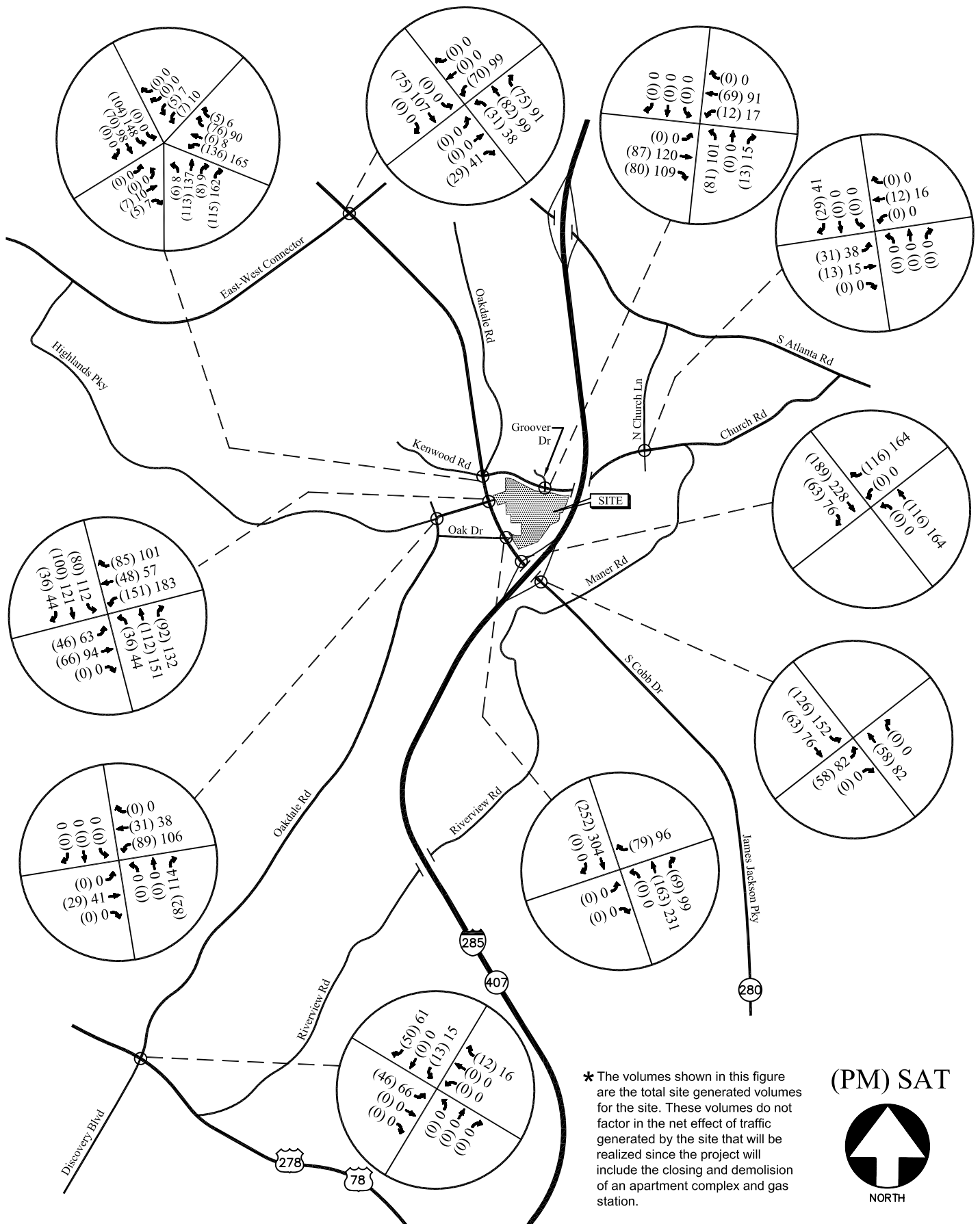
FIGURE 6

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TABLE 3						
EXISTING INTERSECTION OPERATIONS						
Intersection	PM/SAT LOS Standard	Traffic Control	P.M. Peak Hour		SAT Peak Hour	
			LOS (Delay)	v/c*	LOS (Delay)	v/c*
South Cobb Dr / Cumberland Pkwy	E/E	Signalized	F (134.4)	1.25	E (66.4)	0.81
South Cobb Dr / Church Rd / Oakdale Rd / Kenwood Rd	D/D	Signalized	D (44.7)	0.86	B (15.1)	0.56
South Cobb Dr / Highland Pkwy / Gas Station Drwy	D/D	Signalized	D (41.2)	0.78	C (23.9)	0.58
South Cobb Dr / I-285 Southbound Ramps	E/E	Signalized	E (69.7)	0.69	E (72.9)	0.57
South Cobb Dr / I-285 Northbound Ramps	E/E	Signalized	E (65.1)	1.04	F (88.3)	1.24
Highlands Pkwy / Oakdale Rd / Integrity Heights Condos Private Drwy	D/D	Signalized	C (25.9)	0.86	A (8.8)	0.29
US 278 (US 78) / Oakdale Rd / Discovery Boulevard	D/D	Signalized	C (28.4)	0.75	B (16.1)	0.38
Church Rd / North Church Ln / Valley Crest Co Private Drwy		Stop Controlled on North Church Lane / Valley Crest Co Private Driveway				
-Eastbound Left	D/D		A (3.7)	-	A (3.0)	-
-Northbound Approach	D/D		C (17.5)	-	A (9.7)	-
-Southbound Approach	D/D		C (23.4)	-	B (10.2)	-
Church Rd / Groover Dr / Tibarron Pkwy		Stop Controlled on Groover Dr / Tibarron Pkwy				
-Eastbound Left	D/D		A (0.0)	-	A (0.2)	-
-Westbound Left	D/D		A (0.4)	-	A (0.5)	-
-Northbound Approach	D/D		C (15.4)	-	B (10.1)	-
-Southbound Approach	D/D		B (11.7)	-	A (8.9)	-

*v/c ratio is not calculated for unsignalized intersections.

As shown in Table 3, three of the study intersections are not operating at the required LOS standard.



SITE-GENERATED WEEKDAY PEAK-HOUR VOLUMES

FIGURE 7

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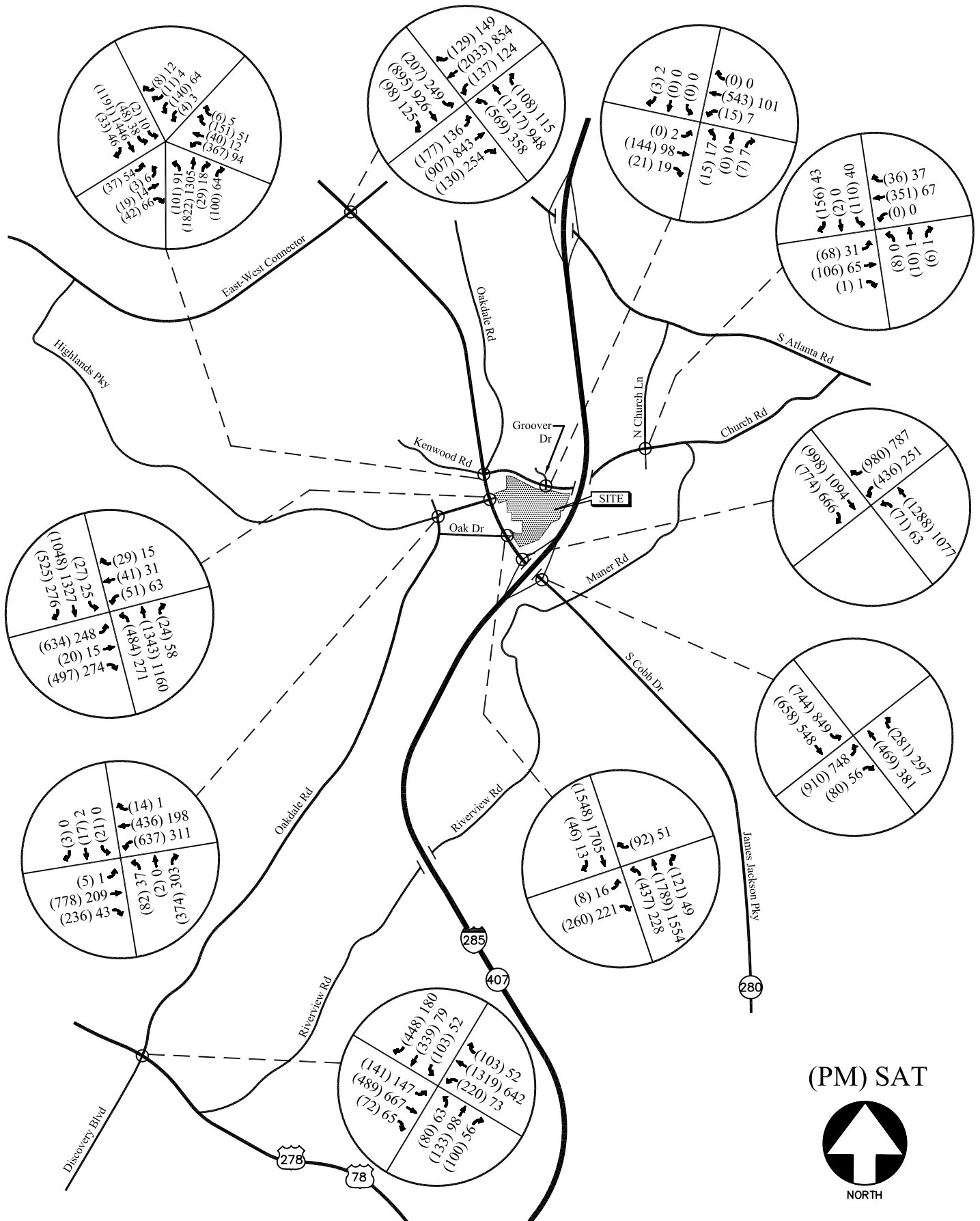
7. FUTURE YEAR BACKGROUND TRAFFIC

In order to evaluate future traffic operations in this area a projection was made of future base year traffic volumes. The Georgia Department of Transportation collected ADT's in the vicinity of the site over the last several years. Using this information, an annual growth factor of 2.7% was calculated. It was agreed upon in the methodology meeting and GRTA letter of understanding to use a growth factor of 3%. This growth factor was applied to the existing traffic volumes on the roadways to estimate the future year 2009 traffic volumes prior to the addition of the site-generated volumes. The future year (base) traffic volumes for 2009 at all the study intersections are shown in Figure 8. The existing signal phasing and timing information obtained from the City of Smyrna and Cobb County for five signalized intersections along South Cobb Drive (I-285 NB Ramps, I-285 SB Ramps, Highland Parkway, Church Road / Kenwood Road, Cumberland Parkway / E-W Connector) was also used to analyze the base traffic operations.

A traffic operation analyses for the following base scenarios was performed:

- Base Year 2009 traffic with existing lane geometry.
- Base Year 2009 traffic with additional recommended improvements to bring all intersections to LOS standard of D.

Results of the analyses for the above scenarios are shown in Tables 4 and 5. Recommendations to bring the intersections back to the LOS standard are discussed after Table 4.



BASE 2009 WEEKDAY PEAK HOUR VOLUMES

FIGURE 8

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TABLE 4						
BASE INTERSECTION OPERATIONS						
Intersection	PM/SAT LOS Standard	Traffic Control	P.M. Peak Hour		SAT Peak Hour	
			LOS (Delay)	v/c*	LOS (Delay)	v/c*
South Cobb Dr / Cumberland Pkwy	E/E	Signalized	F (158.5)	1.33	E (78.1)	0.85
South Cobb Dr / Church Rd / Oakdale Rd / Kenwood Rd	D/D	Signalized	D (48.3)	0.91	B (16.0)	0.64
South Cobb Dr / Highland Pkwy / Gas Station Drwy	D/D	Signalized	D (43.8)	0.83	C (24.7)	0.61
South Cobb Dr / I-285 Southbound Ramps	E/E	Signalized	F (86.4)	0.74	F (95.4)	0.62
South Cobb Dr / I-285 Northbound Ramps	E/E	Signalized	E (79.5)	1.12	F (111.3)	1.35
Highlands Pkwy / Oakdale Rd / Integrity Heights Condos Private Drwy	D/D	Signalized	C (30.7)	0.90	A (8.9)	0.31
US 278 (US 78) / Oakdale Rd / Discovery Boulevard	D/D	Signalized	C (31.5)	0.81	B (16.7)	0.43
Church Rd / North Church Ln / Valley Crest Co Private Drwy	D/D	Stop Controlled on North Church Lane / Valley Crest Co Private Driveway	A (3.7)	-	A (2.5)	-
-Eastbound Left	D/D		C (18.9)	-	A (9.9)	-
-Northbound Approach	D/D		D (28.0)	-	B (10.6)	-
-Southbound Approach	D/D					
Church Rd / Groover Dr / Tibarron Pkwy	D/D	Stop Controlled on Groover Dr / Tibarron Pkwy	A (0.0)	-	A (0.2)	-
-Eastbound Left	D/D		C (16.3)	-	B (10.2)	-
-Northbound Approach	D/D		B (12.0)	-	A (8.9)	-
-Southbound Approach	D/D					

*v/c ratio is not calculated for unsignalized intersections.

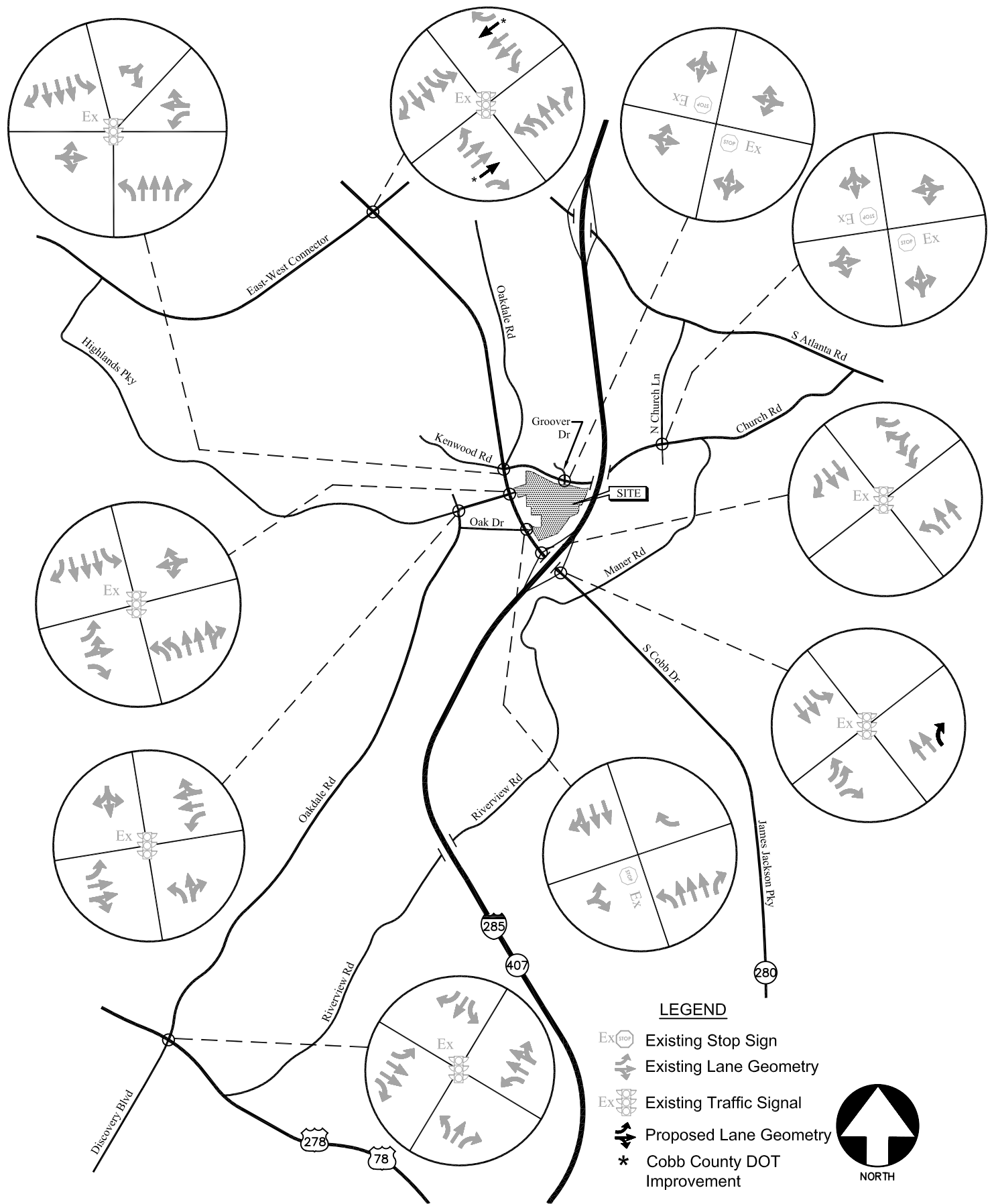
Analysis of the future year (Base 2009) traffic volumes indicates that three of the study intersections will not operate at the LOS standard. The following improvements were identified for those intersections that did not comply with the LOS standard.

- South Cobb Drive / Cumberland Parkway
 - Cobb County DOT is proposing to add additional eastbound and westbound through lanes on Cumberland Parkway creating three through lanes in each direction. This improvement is a planned program projected for completion in 2008. After this improvement is completed, the intersection will continue to operate at LOS F during the PM peak hour. Cobb County is planning to grade separate this intersection in the future.
 - It is also recommended to optimize the intersection signal timing to help improve the overall delay at the intersection.

- South Cobb Drive / I-285 Southbound Ramps
 - Optimize intersection timing splits and offsets. If the signal timing splits and offsets are optimized, the intersection will operate at acceptable LOS without requiring any lane geometry improvements.
- South Cobb Drive / I-285 Northbound Ramps
 - Add a dedicated right turn lane on South Cobb Drive as per Georgia DOT standards.
 - It is also recommended to optimize the intersection signal timing splits and offsets to help the overall delay at the intersection.

The LOS at the above intersections in the year 2009 with just background traffic after the above improvements are implemented is shown in Table 5. The recommended base intersections traffic control and lane geometry are shown in Figure 9.

TABLE 5						
BASE INTERSECTION OPERATIONS – WITH IMPROVEMENTS						
Intersection	PM/SAT LOS Standard	Traffic Control	P.M. Peak Hour		SAT Peak Hour	
			LOS (Delay)	v/c	LOS (Delay)	v/c
South Cobb Dr / Cumberland Pkwy	E/E	Signalized	F (97.0)	1.07	D (46.4)	0.78
South Cobb Dr / I-285 Southbound Ramps	E/E	Signalized	D (37.7)	0.78	C (21.8)	0.66
South Cobb Dr / I-285 Northbound Ramps	E/E	Signalized	D (49.1)	0.91	D (49.2)	0.89



BASE 2009 TRAFFIC CONTROL AND LANE GEOMETRY

FIGURE 9

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8. FUTURE YEAR TOTAL TRAFFIC

The traffic volumes that will be generated by the proposed development were added to the future base year 2009 traffic volumes in order to determine the traffic volumes that will be on the roadway network after completion of the project. The existing trips that are being generated by the existing apartment complex and gas station were deducted from the future traffic volumes since both will be closed and demolished as a part of redevelopment. The future traffic volumes for the year 2009 including the site-generated volumes and deductions for the study intersections are shown in Figure 10.

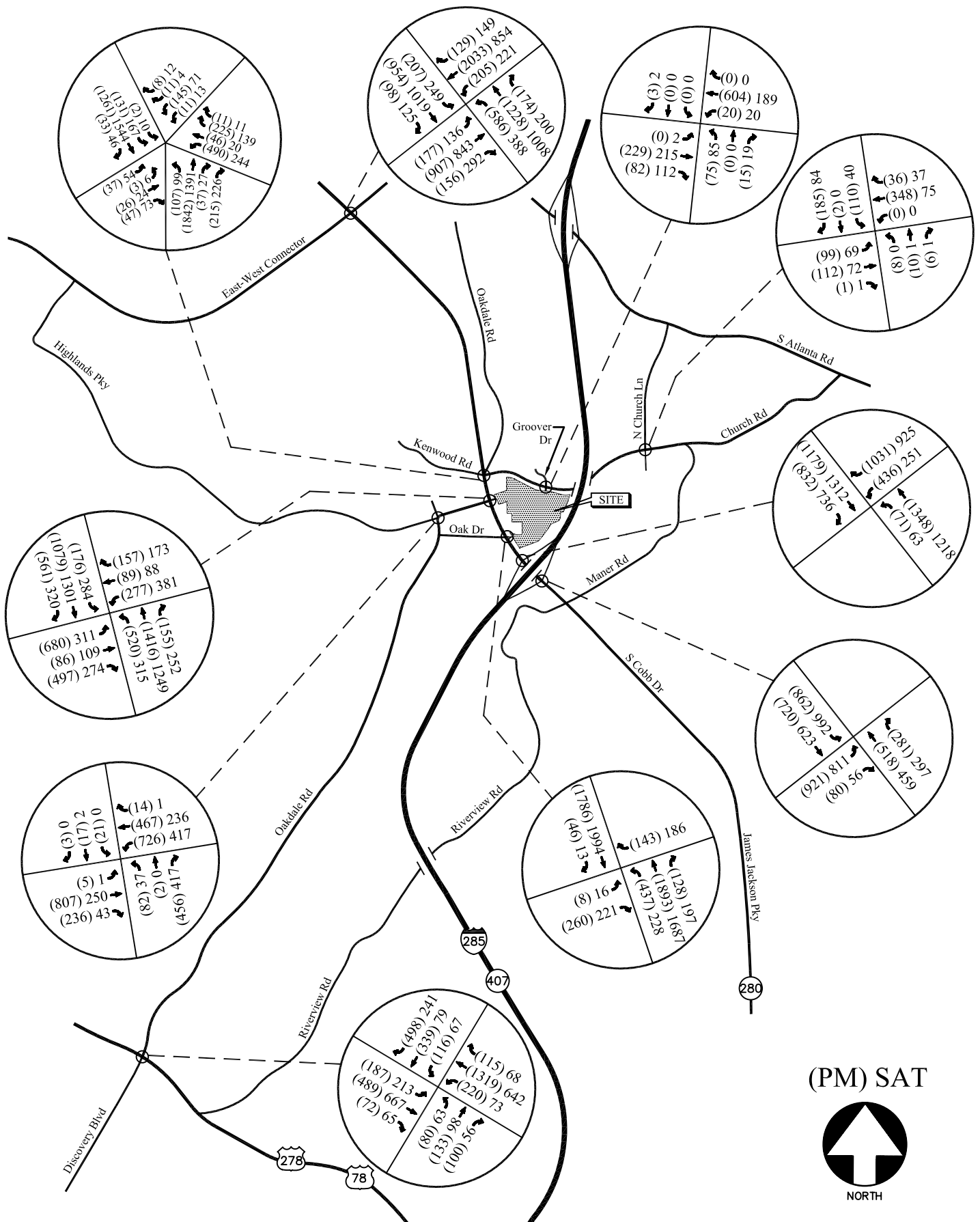
9. FACILITY NEEDS ANALYSIS

9.1 Intersection Analysis

The future year total traffic volumes were used to analyze the study network intersections. Traffic operations analyses for the following scenarios were performed:

- Future Year 2009 Traffic Volumes with site generated traffic and existing lane geometry.
- Future Year 2009 Traffic Volumes with site generated traffic and the recommended improvements to bring all intersections to LOS standard.

The results of the analysis for the above scenarios are shown in Tables 6 and 7. Recommendations to bring the intersections to the LOS standard are discussed after each appropriate section.



FUTURE 2009 WEEKDAY PEAK HOUR VOLUMES

FIGURE 10

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TABLE 6						
FUTURE INTERSECTION OPERATIONS						
Intersection	PM/SAT LOS Standard	Traffic Control	P.M. Peak Hour		SAT Peak Hour	
			LOS (Delay)	v/c*	LOS (Delay)	v/c*
South Cobb Dr / Cumberland Pkwy	E/E	Signalized	F (164.5)	1.30	E (56.1)	0.91
South Cobb Dr / Church Rd / Oakdale Rd / Kenwood Rd	D/D	Signalized	D (42.8)	1.18	C (26.3)	1.02
South Cobb Dr / Highland Pkwy / Main Site Driveway	D/D	Signalized	F (128.6)	1.22	F (122.6)	1.32
South Cobb Dr / I-285 Southbound Ramps	E/E	Signalized	C (25.8)	0.84	C (21.3)	0.84
South Cobb Dr / I-285 Northbound Ramps	E/E	Signalized	F (91.6)	1.09	F (89.6)	1.14
Highlands Pkwy / Oakdale Rd / Integrity Heights Condos Private Drwy	D/D	Signalized	D (36.8)	0.95	B (11.3)	0.47
US 278 (US 78) / Oakdale Rd / Discovery Boulevard	D/D	Signalized	C (34.9)	0.90	B (18.0)	0.48
Church Rd / North Church Ln / Valley Crest Co Private Drwy		Stop Controlled on North Church Lane / Valley Crest Co Private Driveway				
-Eastbound Left	D/D		A (4.5)	-	A (4.5)	-
-Northbound Approach	D/D		C (22.9)	-	B (10.4)	-
-Southbound Approach	D/D		E (38.6)	-	B (11.2)	-

*v/c ratio is not calculated for unsignalized intersections.

Analysis of the future year 2009 traffic volumes indicates that six of the study intersections will not operate within the LOS standard. The following lists the improvements needed to restore that intersections back to the LOS standard for the future year 2009 traffic:

- South Cobb Drive / Cumberland Parkway
 - Cobb County DOT is proposing to add additional eastbound and westbound through lanes on Cumberland Parkway creating three through lanes in each direction. This improvement is a planned program projected for completion in 2008. After this improvement is completed, the intersection will continue to operate at LOS F during the PM peak hour. Cobb County is planning to grade separate this intersection in the future.
 - It is also recommended to optimize the intersection signal timing to help improve the overall delay at the intersection.
- South Cobb Drive / Highland Parkway / Main Site Driveway
 - Optimize intersection signal timing splits and offsets.
 - Add a dedicated northbound right turn lane on South Cobb Drive as per

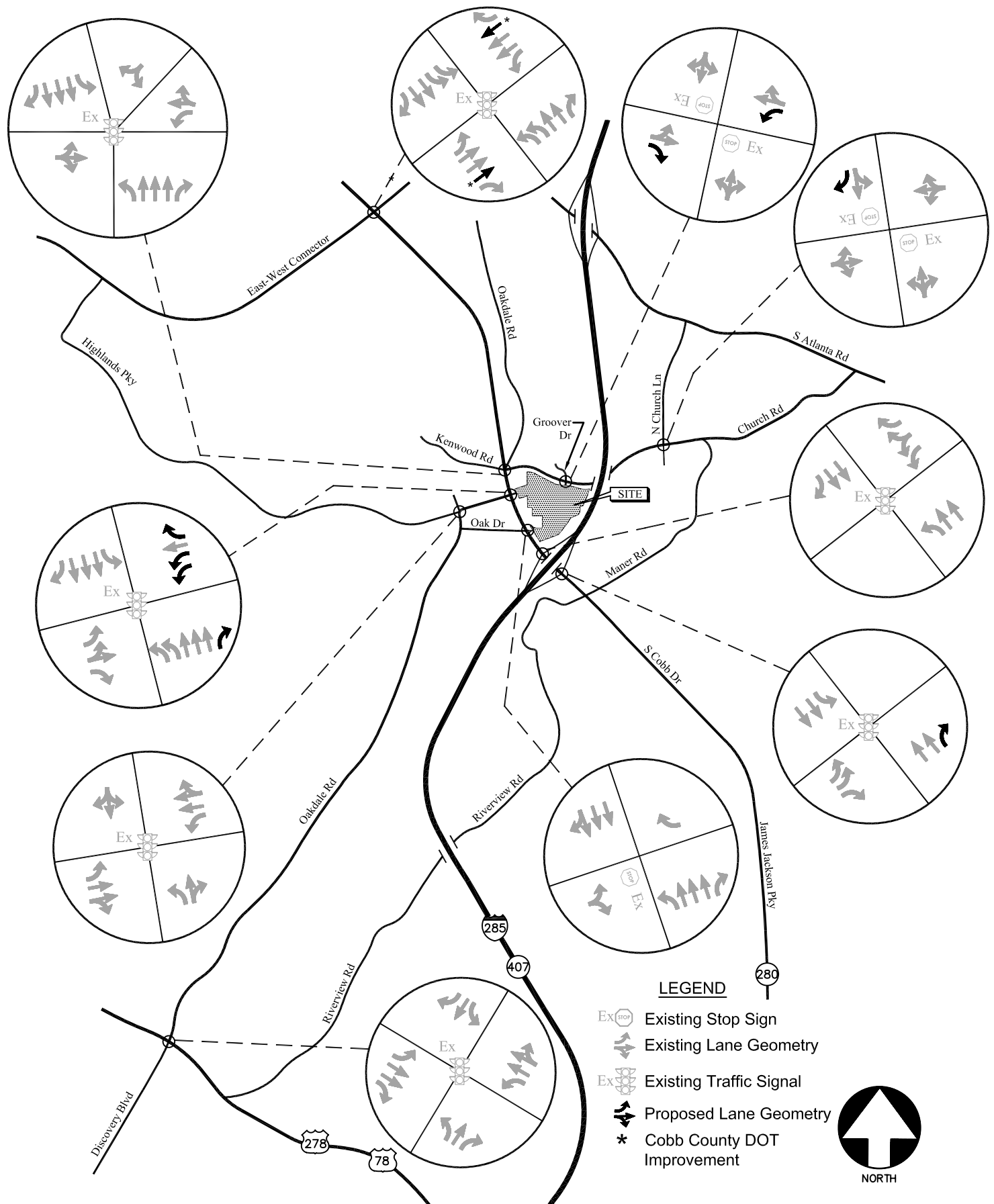
Georgia DOT standards.

- Provide westbound dual left turn lanes, a dedicated through lane and a dedicated westbound right turn lane on the Main Site Driveway.
 - Change the existing southbound right turn phase on South Cobb Drive from permissive to permissive + overlap.
- South Cobb Drive / I-285 Northbound Ramps
 - Add a dedicated right turn lane on South Cobb Drive as per Georgia DOT standards.
 - It is also recommended to optimize the intersection signal timing splits and offsets to help the overall delay at the intersection.
 - Church Road / North Church Lane / Valley Crest Co Private Driveway
 - Add a dedicated southbound right turn lane on North Church Lane.

The LOS for the above intersections in the year 2009 with the addition of site-generated traffic after the implementation of above recommended improvement is shown in Table 7.

TABLE 7						
FUTURE INTERSECTION OPERATIONS WITH IMPROVEMENTS						
Intersection	PM/SA T LOS Standard	Traffic Control	P.M. Peak Hour		SAT Peak Hour	
			LOS	Delay (sec)	LOS	Delay (sec)
South Cobb Dr (SR 280) / Cumberland Pkwy	E/E	Signalized	F (98.7)	1.10	D (51.4)	0.81
South Cobb Dr (SR 280) / Highland Pkwy / Gas Station Drwy	D/D	Signalized	D (49.4)	0.85	D (46.6)	0.84
South Cobb Dr (SR 280) / I-285 Northbound Ramps	E/E	Signalized	E (61.8)	1.00	E (68.1)	1.03
Church Rd / North Church Ln / Valley Crest Co Private Drwy	D/D	Stop Controlled on North Church Lane / Valley Crest Co Private Driveway	A (4.5)	-	A(4.5)	-
-Eastbound Left	D/D		A (0.0)	-	A (0.0)	-
-Westbound Left	D/D		C (20.7)	-	B (10.4)	-
-Northbound Approach	D/D		C (18.8)	-	B (10.5)	-
-Southbound Approach						

The future intersection traffic control and lane geometry for the study area network needed to bring all intersections to the LOS standard is shown in Figure 11.



FUTURE 2009 TRAFFIC CONTROL AND LANE GEOMETRY

FIGURE 11

A&R Engineering Inc.

9.2 Site Access Analysis

The site proposes to have one full access (Main Site Driveway) and one right in right out driveways along South Cobb Drive and three full access site driveways along Church Road. The future traffic volumes at the site driveways are shown in Figure 12. The recommended traffic control and lane geometry at these driveways are presented in the following sections.

The site access analysis was performed for the Future Year 2009 traffic volumes with recommended lane geometry. Results of the analysis are shown in Table 8. Traffic control and lane geometry adopted to operate the intersections at the LOS standard are discussed in the following pages.

TABLE 8 FUTURE SITE DRIVEWAY OPERATIONS						
Intersection	PM/SAT LOS Standard	Traffic Control	P.M. Peak Hour		SAT Peak Hour	
			LOS (Delay)	v/c*	LOS (Delay)	v/c*
South Cobb Dr / Highland Pkwy / Main Site Drwy	D/D	Signalized	D (49.4)	0.85	D (46.6)	0.84
Church Rd / North Site Drwy 1						
-Westbound Left	D/D	Stop Controlled on North Site	A (8.2)	-	A (8.3)	-
-Northbound Approach	D/D	Drwy 1	D (33.6)	-	C (17.3)	-
Church Rd / Groover Dr / North Site Drwy 2						
-Eastbound Left	D/D	Stop Controlled	A (0.0)	-	A (0.1)	-
-Westbound Left	D/D	on Groover Rd /	A (8.3)	-	A (8.2)	-
-Northbound Approach	D/D	North Site Drwy 2	D (34.3)	-	C (15.9)	-
-Southbound Approach	D/D		B (12.7)	-	A (9.5)	-
Church Rd / North Site Drwy 3						
-Westbound Left	D/D	Stop Controlled	A (0.6)	-	A (1.4)	-
-Northbound Approach	D/D	on North Site	C (17.5)	-	B (11.2)	-
		Drwy 3				

*v/c ratio is not calculated for unsignalized intersections.

The following lists the recommended lane geometry for the site driveways.

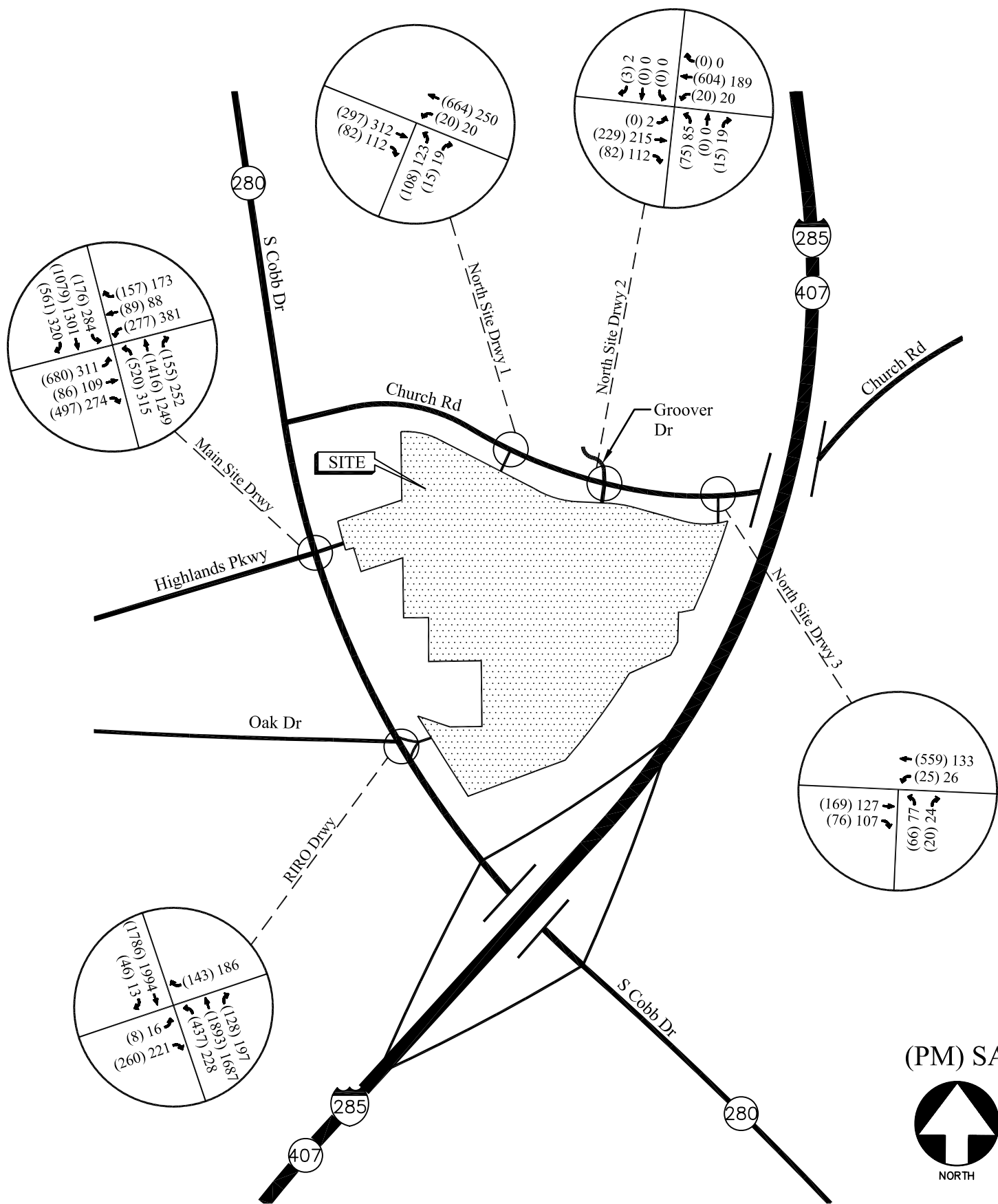
- South Cobb Drive / Highland Parkway / Main Site Driveway
 - Optimize intersection signal timing splits and offsets.
 - Add a dedicated northbound right turn lane on South Cobb Drive as per Georgia DOT standards.
 - Provide westbound dual left turn lanes, a dedicated through lane and a dedicated westbound right turn lane on the Main Site Driveway.
 - Change the existing southbound right turn phase on South Cobb Drive from permissive to permissive + overlap.

- Church Road / North Site Driveway 1
 - It is recommended that the intersection have a stop controlled side street (North Site Driveway 1), with Church Road remaining free flow.
 - Add a dedicated eastbound right turn lane and a dedicated westbound left turn lane on Church Road for traffic entering the proposed development.
 - Provide a shared left / right turn lane on the northbound approach for exiting traffic.

- Church Road / Groover Drive / North Site Driveway 2
 - Add a dedicated eastbound right turn lane and a dedicated westbound left turn lane on Church Road for traffic entering the proposed development.
 - Provide a shared left / through / right turn lane on the northbound approach for exiting traffic.
 - The North Site Driveway 2 should be controlled with a stop sign, with Church Road remaining free flow.

- Church Road / North Site Driveway 3
 - It is recommended that the intersection have a stop controlled side street (North Site Driveway 3), with Church Road remaining free flow.
 - Add a dedicated eastbound right turn lane on Church Road for traffic entering the proposed development.
 - Provide a shared left / right turn lane on the northbound approach for exiting traffic.

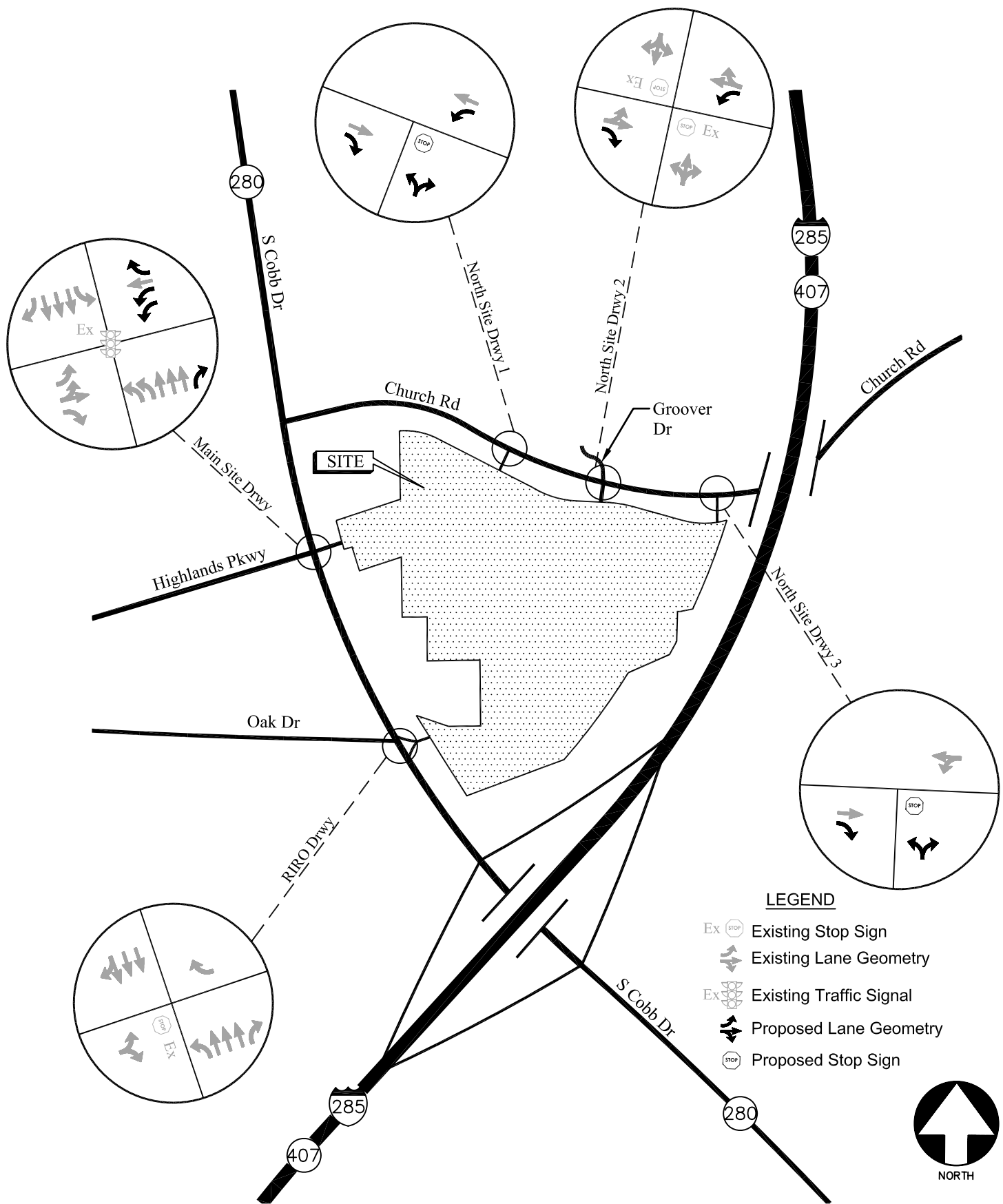
The recommended traffic control and lane geometry for the site driveways is shown in Figure 13.



FUTURE 2009 SITE ACCESS PEAK HOUR VOLUMES

FIGURE 12

A&R Engineering Inc.



FUTURE 2009 SITE ACCESS TRAFFIC CONTROL AND LANE GEOMETRY

FIGURE 13
A&R Engineering Inc.

10. NON-EXPEDITED CRITERIA

1. Quality, Character, Convenience, and Flexibility of Transportation Options

CCT bus route 70 currently serves the proposed site and a bus stop is located on South Cobb Drive between Oak Drive and Highlands Parkway.

2. Vehicle Miles Traveled

The table below displays the reductions in trip generation due to mixed-use and pass-by reductions.

24-hour Trip Generation	17,572
- Pass-by Reductions	-4,110
- Existing Apartment Trip Reduction	-2,406
- Existing Gas Station Trip Reduction	-2,202
Net Trips:	8,585

3. Relationship Between Location of Proposed DRI and Regional Mobility

The proposed DRI is currently served by one CCT bus route.

4. Relationship Between Proposed DRI and Existing or Planned Transit Facilities

CCT bus route 70 currently serves the proposed site and a bus stop on South Cobb Drive is located across from the site. Details for CCT bus 70 is included in the Appendix.

5. Transportation Management Area Designation

The area around the proposed project is not designated as a transportation management area.

6. Offsite Trip Reduction and Trip Reduction Techniques

Due to the nature of the development, there will be significant mixed-use and pass by trip reductions. These reductions have been applied for the AM peak hour, PM peak hour and 24-hour trips projected to be generated by the site. In addition, the site is currently generating traffic. These existing uses will be closed and demolished as part of redevelopment. Accordingly, the trips generated by these uses will cease to occur.

7. Balance of Land Uses – Jobs/Housing Balance

Please refer to the AOI study submitted along with this report.

8. Relationships Between Proposed DRI and Existing Development and Infrastructure

The proposed DRI is located in an area where adequate public facilities will be available to serve the proposed development. Cobb County Water and Sewer authority will provide water and wastewater services for the development. Regarding transportation, the traffic study has identified transportation improvements relating to the site access, along with improvements to the surrounding roadway network, which will allow traffic in the area to operate at the LOS standard.

10.2 Pedestrian and Internal Circulation

The proposed project will provide pedestrian walkways in all directions to connect the site with adjacent pedestrian facilities. The network of sidewalks will provide adequate pedestrian access to the various land uses within the development.

Internal roadways will provide adequate circulation of vehicular traffic as designed in the site plan. Vehicles exiting the site will be able to move internally to the site exit without experiencing excessive delays.

11. OTHER PERTINENT INFORMATION

At this time no other pertinent information is available with regards to this development. All significant characteristics of the proposed development are fully discussed within this report. Additionally, an Area of Influence study has been prepared and submitted along with the traffic analysis.

12. SIGNIFICANT IMPACT ANALYSIS

Due to the annual growth in the county in addition to the site generated traffic a number of intersection improvements will be required to bring the intersections in the vicinity of the site to the LOS standard. Even with the annual growth and the addition of the site generated traffic, the implementation of the recommended improvements will allow all intersections in the study area network to operate at acceptable levels of service.

Appendix

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Existing Intersection Analysis

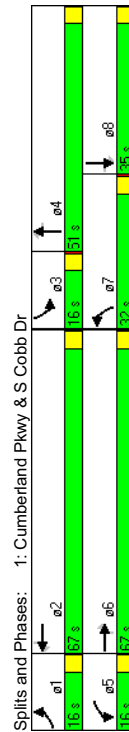
Existing PM Intersection Analysis

Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	167	855	123	129	1916	122	536	1147	102	195	844	92
Lane Group Flow (vph)	192	891	145	143	2017	136	583	1274	124	219	870	116
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases												
Total Split (s)	16.0	67.0	35.0	16.0	67.0	51.0	32.0	51.0	67.0	16.0	35.0	67.0
Act Eff Green (s)	12.0	63.0	31.4	12.0	63.0	47.0	27.4	47.0	63.0	11.8	31.4	63.0
Actuated g/C Ratio	0.08	0.42	0.21	0.08	0.42	0.31	0.18	0.31	0.42	0.08	0.21	0.42
Control Delay	246.4	35.7	21.6	144.0	199.8	31.0	82.1	122.9	15.8	89.7	141.8	7.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	246.4	35.7	21.6	144.0	199.8	31.0	82.1	122.9	15.8	89.7	141.8	7.9
LOS	F	D	C	F	F	C	F	F	B	F	F	A
Approach Delay												
Approach LOS												

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 149.8												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 1.36												
Intersection Signal Delay: 128.2												
Intersection Capacity Utilization 114.2%												
Analysis Period (min) 15												



HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	167	855	123	129	1916	122	536	1147	102	195	844	92
Peak-hour factor, PHF	0.87	0.96	0.85	0.90	0.95	0.90	0.92	0.90	0.82	0.89	0.97	0.79
Adj. Flow (vph)	192	891	145	143	2017	136	583	1274	124	219	870	116
RTOR Reduction (vph)	0	0	75	0	0	21	0	0	31	0	0	54
Lane Group Flow (vph)	192	891	70	143	2017	115	583	1274	93	219	870	62
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases												
Actuated Green, G (s)	12.0	63.0	31.4	12.0	63.0	47.0	27.4	47.0	63.0	11.8	31.4	63.0
Effective Green, g (s)	12.0	63.0	31.4	12.0	63.0	47.0	27.4	47.0	63.0	11.8	31.4	63.0
Actuated g/C Ratio	0.08	0.42	0.21	0.08	0.42	0.31	0.18	0.31	0.42	0.08	0.21	0.42
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	142	1488	332	142	1488	497	628	1110	666	270	742	666
v/s Ratio Prot	c0.11	0.25	0.04	0.08	c0.57	c0.17	c0.36		0.06	0.06	0.25	0.04
v/s Ratio Perm	1.35	0.60	0.21	1.01	1.36	0.23	0.93	1.15	0.14	0.81	1.17	0.09
Uniform Delay, d1	68.9	33.6	49.0	68.9	43.4	38.0	60.2	51.4	26.7	67.9	59.2	26.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	197.5	1.8	0.3	77.4	164.5	0.2	20.0	77.3	0.4	16.7	91.6	0.3
Delay (s)	266.4	35.4	49.3	146.3	207.9	38.3	80.2	128.7	27.1	84.6	150.8	26.4
Level of Service	F	D	D	F	F	D	F	F	C	F	F	C
Approach Delay (s)												
Approach LOS												

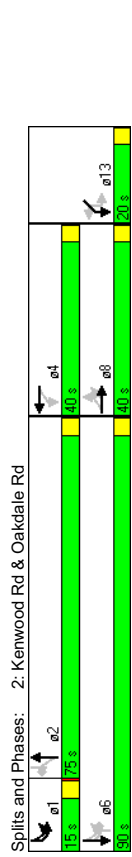
Intersection Summary												
HCM Average Control Delay												
HCM Volume to Capacity ratio												
Actuated Cycle Length (s)												
Intersection Capacity Utilization												
Analysis Period (min)												
c Critical Lane Group												

Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Existing PM
5/18/2007

Lane Group	EBL2	EBL	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	1723	0	1770	1641	0	0	1770	5075	0	1583
Satd. Flow (prot)	0	0	0.718	0	0.615	0.234	0	0	0.234	0	0	0
Fit Permitted	0	0	1263	0	1146	1641	0	0	436	5075	0	1583
Satd. Flow (perm)	35	3	18	40	346	38	142	6	95	1717	27	94
Satd. Flow (RTOR)	0	0	128	0	357	191	0	0	96	1761	0	95
Volume (vph)	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Lane Group Flow (vph)	8	8	8	8	4	4	4	4	2	2	2	6
Turn Type	Protected Phases	Permitted Phases	Total Split (s)	Act Effct Green (s)	Actuated g/C Ratio	v/c Ratio	Control Delay	Queue Delay	Total Delay	LOS	Approach Delay	Approach LOS
Protected Phases	8	8	40.0	0.0	0.0	40.0	0.0	0.0	75.0	75.0	0.0	90.0
Permitted Phases	40.0	40.0	36.0	36.0	0.24	0.24	0.24	0.51	0.51	0.51	0.57	0.57
Total Split (s)	36.0	36.0	0.40	1.30	0.48	0.43	0.68	0.10	0.10	0.10	0.10	0.10
Act Effct Green (s)	0.24	0.24	43.8	202.6	53.6	25.6	25.8	5.9	5.9	5.9	5.9	5.9
Actuated g/C Ratio	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
v/c Ratio	43.8	202.6	53.6	25.6	25.8	5.9	5.9	5.9	5.9	5.9	5.9	5.9
Control Delay	D	F	D	F	D	F	D	F	D	F	D	F
Queue Delay	43.8	202.6	53.6	25.6	25.8	5.9	5.9	5.9	5.9	5.9	5.9	5.9
Total Delay	43.8	202.6	53.6	25.6	25.8	5.9	5.9	5.9	5.9	5.9	5.9	5.9
LOS	D	F	D	F	D	F	D	F	D	F	D	F
Approach Delay	43.8	202.6	53.6	25.6	25.8	5.9	5.9	5.9	5.9	5.9	5.9	5.9
Approach LOS	D	F	D	F	D	F	D	F	D	F	D	F

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 140 (93%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.30												
Intersection Signal Delay: 44.4												
Intersection Capacity Utilization 86.0%												
Analysis Period (min) 15												



Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Existing PM
5/18/2007

Lane Group	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations		↔	↕↕↕	↕		↕		↕
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	0	1770	5085	1583	0	1770	1583	0
Fit Permitted	0.053							
Satd. Flow (perm)	0	99	5085	1583	0	1770	1583	0
Satd. Flow (RTOR)	32							
Volume (vph)	2	45	1123	31	4	132	10	8
Lane Group Flow (vph)	0	48	1146	32	0	155	20	0
Turn Type	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm	Perm
Protected Phases	1	1	6	2	13	13	13	13
Permitted Phases	6	6						
Total Split (s)	15.0	15.0	90.0	75.0	20.0	20.0	20.0	0.0
Act Effct Green (s)	86.0	86.0	86.0	76.7	16.0	16.0	16.0	
Actuated g/C Ratio	0.57	0.57	0.51	0.11	0.11	0.11	0.11	
v/c Ratio	0.35	0.39	0.04	0.82	0.82	0.11	0.11	
Control Delay	21.0	18.1	6.3	6.3	96.7	42.5	42.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	21.0	18.1	6.3	6.3	96.7	42.5	42.5	
LOS	C	B	A	A	F	F	D	
Approach Delay	17.9							
Approach LOS	B							
Intersection Summary								

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 140 (93%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.30												
Intersection Signal Delay: 44.4												
Intersection Capacity Utilization 86.0%												
Analysis Period (min) 15												



HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Existing PM
5/18/2007

Movement	EBL2	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.94	0.98	0.95	1.00	0.88	0.95	1.00	1.00	1.00	0.85	0.85
Flt Protected	0.98	0.98	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1724	1770	1770	1640	1770	1640	1770	5074	5074	1583	1583
Flt Permitted	0.72	0.61	0.22	1.00	0.22	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1263	1145	1640	1640	1145	1640	1640	408	5074	1583	1583
Volume (vph)	35	3	18	40	346	38	142	6	95	1717	27
Peak-hour factor, PHF	0.75	0.75	0.75	0.97	0.97	0.97	0.97	0.99	0.99	0.99	0.99
Adj. Flow (vph)	47	4	24	53	357	39	146	6	96	1734	27
RTOR Reduction (vph)	0	0	17	0	0	1	0	0	0	0	20
Lane Group Flow (vph)	0	0	111	0	357	190	0	0	96	1761	0
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	custom	custom
Protected Phases	8	8	8	4	4	4	4	2	2	6	6
Permitted Phases	8	8	8	4	4	4	4	2	2	75.9	75.9
Actuated Green, G (s)	36.0	36.0	36.0	36.0	36.0	36.0	36.0	75.9	75.9	86.0	86.0
Effective Green, g (s)	36.0	36.0	36.0	36.0	36.0	36.0	36.0	75.9	75.9	86.0	86.0
Actuated g/C Ratio	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.51	0.51	0.57	0.57
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	303	275	394	275	394	275	394	206	2567	908	908
v/s Ratio Prot	0.09	0.37	0.31	0.31	0.31	0.31	0.31	0.24	0.35	0.05	0.05
v/s Ratio Perm	0.37	0.37	0.37	0.37	0.37	0.37	0.37	0.47	0.69	0.08	0.08
Uniform Delay, d1	47.5	47.5	47.5	47.5	47.5	47.5	47.5	24.0	28.0	14.3	14.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.81	0.88	0.76	0.76
Incremental Delay, d2	0.8	0.8	0.8	0.8	0.8	0.8	0.8	5.7	1.2	0.1	0.1
Delay (s)	48.3	48.3	48.3	48.3	48.3	48.3	48.3	25.0	25.7	11.1	11.1
Level of Service	D	D	D	D	D	D	D	C	C	B	B
Approach Delay (s)	48.3	48.3	48.3	48.3	48.3	48.3	48.3	25.0	25.0	11.1	11.1
Approach LOS	D	D	D	D	D	D	D	C	C	B	B

Intersection Summary	
HCM Average Control Delay	44.7
HCM Volume to Capacity ratio	0.86
Actuated Cycle Length (s)	150.0
Intersection Capacity Utilization	86.0%
Analysis Period (min)	15
Critical Lane Group	

HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Existing PM
5/18/2007

Movement	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	0.95	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1770	1770	5085	1583	1770	1583	1770	1583
Flt Permitted	0.06	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	110	5085	1583	1583	1770	1583	1770	1583
Volume (vph)	2	45	1123	31	4	132	10	8
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.88	0.88	0.88	0.88
Adj. Flow (vph)	2	46	1146	32	5	150	11	9
RTOR Reduction (vph)	0	0	0	16	0	0	0	8
Lane Group Flow (vph)	0	48	1146	16	0	155	12	0
Turn Type	pm+pt	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm
Protected Phases	1	1	6	2	13	13	13	13
Permitted Phases	6	6	6	2	13	13	13	13
Actuated Green, G (s)	86.0	86.0	86.0	75.9	16.0	16.0	16.0	16.0
Effective Green, g (s)	86.0	86.0	86.0	75.9	16.0	16.0	16.0	16.0
Actuated g/C Ratio	0.57	0.57	0.57	0.51	0.11	0.11	0.11	0.11
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	131	2915	801	801	189	169	169	169
v/s Ratio Prot	0.01	0.23	0.01	0.01	0.09	0.01	0.01	0.01
v/s Ratio Perm	0.20	0.37	0.39	0.02	0.82	0.07	0.07	0.07
Uniform Delay, d1	21.3	17.6	18.5	18.5	65.6	60.3	60.3	60.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.7	0.4	0.0	0.0	31.3	0.8	0.8	0.8
Delay (s)	23.0	18.0	18.5	18.5	96.9	61.1	61.1	61.1
Level of Service	C	B	B	B	F	F	F	F
Approach Delay (s)	18.2	18.2	18.2	18.2	92.8	92.8	92.8	92.8
Approach LOS	B	B	B	B	F	F	F	F

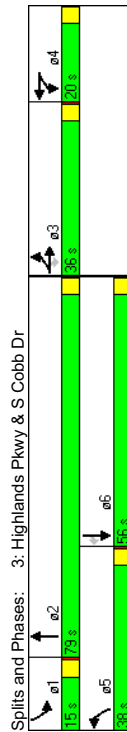
Intersection Summary	
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Lanes, Volumes, Timings
3: Highlands Pkwy & S Cobb Dr

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1681	1692	1583	0	1762	0	3433	5070	0	1770	5085	1583
Fit Permitted	0.950	0.956	0.980	0.980	0.980	0.980	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (perm)	1681	1692	1583	0	1762	0	3433	5070	0	1770	5085	1583
Satd. Flow (RTOR)	435	435	435	0	435	0	435	435	0	435	435	435
Volume (vph)	598	19	468	48	39	27	456	1353	23	25	1000	495
Lane Group Flow (vph)	323	341	498	0	140	0	490	1423	0	32	1031	544
Turn Type	Split	Perm	Split	Split	Split	Split	Prot	Prot	Prot	Prot	Prot	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	6
Permitted Phases	36.0	36.0	36.0	20.0	20.0	20.0	0.0	38.0	79.0	0.0	15.0	56.0
Total Split (s)	31.6	31.6	31.6	14.5	14.5	14.5	25.7	83.8	8.1	62.1	62.1	62.1
Act Effct Green (s)	0.21	0.21	0.21	0.10	0.10	0.10	0.17	0.56	0.05	0.41	0.41	0.41
Actuated g/C Ratio	0.91	0.96	0.74	0.78	0.78	0.78	0.83	0.50	0.33	0.49	0.71	0.71
Control Delay	87.3	95.4	15.4	89.6	89.6	89.6	63.5	22.8	79.9	23.1	18.3	18.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.2	0.5	0.5
Total Delay	87.3	95.4	15.4	89.6	89.6	89.6	63.5	22.9	79.9	23.3	18.9	18.9
LOS	F	F	B	F	F	F	E	C	E	C	B	B
Approach Delay	58.9	58.9	58.9	89.6	89.6	89.6	33.3	33.3	33.3	22.9	22.9	22.9
Approach LOS	E	E	E	F	F	F	C	C	C	C	C	C

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 115 (77%), Referenced to phase 2:NBT and 6:SBT, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.96												
Intersection Signal Delay: 37.6												
Intersection Capacity Utilization 66.1%												
ICU Level of Service C												
Analysis Period (min) 15												



Splits and Phases: 3: Highlands Pkwy & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
3: Highlands Pkwy & S Cobb Dr

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit Protected	0.95	0.96	1.00	0.98	0.98	0.95	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1681	1692	1583	1763	1763	1681	1692	1583	1763	1681	1692	1583
Fit Permitted	0.95	0.96	1.00	0.98	0.98	0.95	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	1681	1692	1583	1763	1763	1681	1692	1583	1763	1681	1692	1583
Volume (vph)	598	19	468	48	39	27	456	1353	23	25	1000	495
Peak-hour factor, PHF	0.94	0.68	0.94	0.86	0.81	0.75	0.93	0.97	0.82	0.78	0.97	0.91
Adj. Flow (vph)	636	28	498	56	48	36	490	1395	28	32	1031	544
RTOR Reduction (vph)	0	0	343	0	8	0	0	1	0	0	0	105
Lane Group Flow (vph)	323	341	498	0	132	0	490	1422	0	32	1031	439
Turn Type	Split	Perm	Split	Split	Split	Split	Prot	Prot	Prot	Prot	Prot	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	6
Permitted Phases	31.6	31.6	31.6	14.5	14.5	14.5	25.7	82.2	5.7	62.2	62.2	62.2
Actuated Green, g (s)	31.6	31.6	31.6	14.5	14.5	14.5	25.7	82.2	5.7	62.2	62.2	62.2
Effective Green, g (s)	0.21	0.21	0.21	0.10	0.10	0.10	0.17	0.55	0.04	0.41	0.41	0.41
Actuated g/C Ratio	0.91	0.96	0.74	0.78	0.78	0.78	0.83	0.50	0.33	0.49	0.71	0.71
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	354	356	333	170	170	170	588	2778	67	2109	656	656
v/s Ratio Prot	0.19	c0.20	0.10	c0.07	c0.07	c0.07	c0.14	0.28	0.02	0.20	0.20	0.20
v/s Ratio Perm	0.91	0.96	0.46	0.78	0.78	0.78	0.83	0.51	0.48	0.49	0.67	0.67
Uniform Delay, d1	57.8	58.5	51.8	66.2	66.2	66.2	60.1	21.3	70.7	32.2	35.6	35.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.04	1.07	0.67	0.53	0.53
Incremental Delay, d2	26.9	36.3	1.0	19.6	19.6	19.6	6.7	0.4	4.2	0.6	4.2	4.2
Delay (s)	84.8	94.8	52.8	85.7	85.7	85.7	61.8	22.5	80.1	22.2	23.2	23.2
Level of Service	F	F	D	F	F	F	E	C	F	C	C	C
Approach Delay (s)	74.0	74.0	74.0	85.7	85.7	85.7	32.6	32.6	32.6	23.7	23.7	23.7
Approach LOS	E	E	E	F	F	F	C	C	C	C	C	C

Intersection Summary												
HCM Average Control Delay	41.2											
HCM Volume to Capacity ratio	0.78											
Actuated Cycle Length (s)	150.0											
Intersection Capacity Utilization	66.1%											
Analysis Period (min)	15											
c Critical Lane Group												

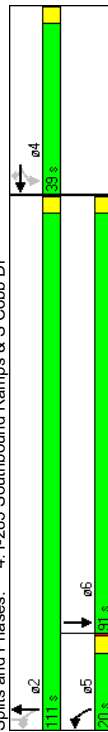
Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	0	1681	1443	1504	1770	3539	0	0	3539	1583
Satd. Flow (prot)	0	0	0	0.950	0.999	0.207						
Flt Permitted	0	0	0	1681	1443	1504	386	3539	0	0	3539	1583
Satd. Flow (RTOR)	0	0	0	107	107	107						
Volume (vph)	0	0	0	411	0	924	67	1214	0	0	941	730
Satd. Flow (prot)	0	0	0	426	520	514	80	1252	0	0	991	785
Lane Group Flow (vph)	0	0	0	426	520	514	80	1252	0	0	991	785
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	pm+pt	pm+pt	Perm	Perm	pm+pt	custom
Protected Phases	4	4	4	4	4	4	5	2			6	
Permitted Phases	0.0	0.0	0.0	39.0	39.0	39.0	20.0	111.0	0.0	0.0	91.0	111.0
Total Split (s)	0.0	0.0	0.0	35.0	35.0	35.0	107.0	107.0	0.0	0.0	97.5	107.0
Act Effct Green (s)	0.23	0.23	0.23	0.71	0.71	0.71	0.71	0.71	0.65	0.71	0.65	0.71
Actuated g/C Ratio	1.09	1.24	1.19	0.25	0.50	0.43	0.58					
v/c Ratio	123.3	164.4	143.7	0.9	1.2	6.0	4.8					
Queue Delay	7.6	7.9	0.0	0.0	0.4	0.0	0.0					
Queue Delay	131.0	172.2	143.7	0.9	1.6	6.0	4.8					
LOS	F	F	F	A	A	A	A					
Approach Delay	150.2											
Approach LOS	F											

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 122 (81%), Referenced to phase 2:NBT and 6:SBT, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.24												
Intersection Signal Delay: 50.6												
Intersection Capacity Utilization 129.0%												
Analysis Period (min) 15												

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
4: I-285 Southbound Ramps & S Cobb Dr

Existing PM
5/18/2007

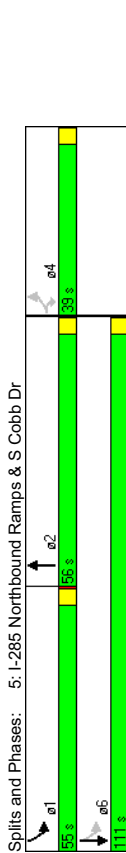
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	0	0	0	1681	1443	1504	1770	3539	0	0	3539	1583
Lane Util. Factor	0.95	0.91	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Protected	0.95	0.91	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1681	1443	1504	1770	3539	3539	1583					
Flt Permitted	0.95	1.00	1.00	0.23	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1681	1443	1504	425	3539	3539	1583					
Volume (vph)	0	0	0	411	0	924	67	1214	0	0	941	730
Peak-hour factor, PHF	0.92	0.92	0.92	0.95	0.92	0.90	0.84	0.97	0.92	0.92	0.95	0.93
Adj. Flow (vph)	0	0	0	433	0	1027	80	1252	0	0	991	785
RTOR Reduction (vph)	0	0	0	0	82	82	0	0	0	0	0	225
Lane Group Flow (vph)	0	0	0	426	438	432	80	1252	0	0	991	560
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	pm+pt	pm+pt	Perm	Perm	pm+pt	custom
Protected Phases	4	4	4	4	4	4	5	2			6	
Permitted Phases	35.0	35.0	35.0	107.0	107.0	107.0	107.0	107.0	35.0	35.0	97.5	107.0
Effective Green, G (s)	35.0	35.0	35.0	107.0	107.0	107.0	107.0	107.0	35.0	35.0	97.5	107.0
Actuated g/C Ratio	0.23	0.23	0.23	0.71	0.71	0.71	0.71	0.71	0.65	0.71	0.65	0.71
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	392	337	351	352	2524	2524					2300	1129
v/s Ratio Prot	0.25	0.30	0.29	0.15							0.28	
v/s Ratio Perm	1.09	1.30	1.23	0.23	0.50						0.43	0.50
Uniform Delay, d1	57.5	57.5	57.5	8.3	9.5						12.8	9.5
Progression Factor	1.00	1.00	1.00	0.05	0.10						0.42	6.73
Incremental Delay, d2	70.8	155.0	126.3	0.1	0.2						0.5	1.4
Delay (s)	128.3	212.5	183.8	0.5	1.2						5.9	65.6
Level of Service	F	F	F	A	A						A	E
Approach Delay (s)	0.0	0.0	0.0	177.8							32.3	
Approach LOS	A	A	A	F							C	
Intersection Summary												
HCM Average Control Delay	69.7											
HCM Volume to Capacity ratio	0.69											
Actuated Cycle Length (s)	150.0											
Intersection Capacity Utilization	129.0%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3330	0	1770	3539	0	0
Flt Permitted	0.950									0.169		
Satd. Flow (perm)	3433	0	1583	0	0	0	3330	0	315	3539	0	0
Satd. Flow (RTOR)			87				112					
Volume (vph)	858	0	75	0	0	0	442	265	701	620	0	0
Lane Group Flow (vph)	876	0	88	0	0	0	787	0	746	667	0	0
Turn Type	custom	custom						pm+pt				
Protected Phases	4		4				2			1		6
Permitted Phases												
Total Split (s)	39.0	0.0	39.0	0.0	0.0	0.0	56.0	0.0	55.0	111.0	0.0	0.0
Act Effct Green (s)	35.0		35.0				52.0		107.0	107.0		
Actuated g/C Ratio	0.23		0.23				0.35		0.71	0.71		
Control Delay	112.9		9.9				37.7		75.4	8.3		
Queue Delay	0.0		0.0				0.0		91.4	0.4		
Total Delay	112.9		9.9				37.7		166.8	8.7		
LOS	F		A				D		F	A		
Approach Delay							37.7					F
Approach LOS							D					

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 101 (67%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.09												
Intersection Signal Delay: 82.1												
Intersection Capacity Utilization 129.0%												
Analysis Period (min) 15												



HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.85	1.00	0.95	1.00	0.94	1.00	1.00	1.00	1.00	0.95
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3433	1583		3433	1583		3329	1770	3539		1770	3539
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.17	1.00		0.17	1.00
Satd. Flow (perm)	3433	1583		3433	1583		3329	1770	3539		1770	3539
Volume (vph)	858	0	75	0	0	0	442	265	701	620	0	0
Peak-hour factor, PHF	0.98	0.92	0.85	0.92	0.92	0.92	0.93	0.85	0.94	0.93	0.92	0.92
Adj. Flow (vph)	876	0	88	0	0	0	475	312	746	667	0	0
RTOR Reduction (vph)	0	0	67	0	0	0	0	73	0	0	0	0
Lane Group Flow (vph)	876	0	21	0	0	0	714	0	746	667	0	0
Turn Type	custom	custom						pm+pt				
Protected Phases	4		4				2			1		6
Permitted Phases												
Actuated Green, G (s)	35.0		35.0				52.0		107.0	107.0		
Effective Green, g (s)	35.0		35.0				52.0		107.0	107.0		
Actuated g/C Ratio	0.23		0.23				0.35		0.71	0.71		
Clearance Time (s)	4.0		4.0				4.0		4.0	4.0		
Vehicle Extension (s)	3.0		3.0				3.0		3.0	3.0		
Lane Grp Cap (vph)	801		369				1154		719	2524		
v/s Ratio Prot							0.21		0.35	0.19		
v/s Ratio Perm	0.26		0.01						0.39			
v/c Ratio	1.09		0.06				0.62		1.04	0.26		
Uniform Delay, d1	57.5		44.7				40.8		36.1	7.6		
Progression Factor	1.00		1.00				1.00		1.09	1.06		
Incremental Delay, d2	60.4		0.1				2.5		39.7	0.2		
Delay (s)	117.9		44.8				43.2		79.2	8.2		
Level of Service	F		D				D		E	A		
Approach Delay (s)		111.3			0.0		43.2			45.7		
Approach LOS		F			A		D			D		
Intersection Summary												
HCM Average Control Delay	65.1											
HCM Volume to Capacity ratio	1.04											
Actuated Cycle Length (s)	150.0											
Intersection Capacity Utilization	129.0%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
6: Church Rd & N Church Lane

Existing PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1820	0	0	1837	0	0	1762	0	0	1681	0
Flt Permitted	0	0.980	0	0	0.981	0	0	0.982	0	0	0.981	0
Satd. Flow (perm)	0	1820	0	0	1837	0	0	1762	0	0	1681	0
Volume (vph)	64	100	1	0	331	34	8	9	6	104	2	147
Lane Group Flow (vph)	0	188	0	0	396	0	0	44	0	0	297	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 59.2%												
Analysis Period (min) 15												
ICU Level of Service B												

HCM Unsignalized Intersection Capacity Analysis
6: Church Rd & N Church Lane

Existing PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	64	100	1	0	331	34	8	9	6	104	2	147
Peak Hour Factor	0.84	0.93	0.25	0.92	0.93	0.85	0.50	0.56	0.50	0.90	0.50	0.83
Hourly flow rate (vph)	76	108	4	0	356	40	16	16	12	116	4	177
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				None
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	356			112			817	618	110	658	640	376
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	356			112			817	618	110	658	640	376
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	94			100			92	96	99	66	99	74
cM capacity (veh/h)	1203			1478			205	379	944	343	369	671
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	188	396	44	297								
Volume Left	76	0	16	116								
Volume Right	4	40	12	177								
cSH	1203	1478	331	485								
Volume to Capacity	0.06	0.00	0.13	0.61								
Queue Length 95th (ft)	5	0	11	101								
Control Delay (s)	3.7	0.0	17.5	23.4								
Lane LOS	A	C	C	C								
Approach Delay (s)	3.7	0.0	17.5	23.4								
Approach LOS	C	C	C	C								
Intersection Summary												
Average Delay												
Intersection Capacity Utilization												
Analysis Period (min)												
ICU Level of Service B												

Lanes, Volumes, Timings
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3412	0	1770	3511	0	1770	1863	1583	0	1787	0
Flt Permitted	0.950	0.083		0.083			0.721				0.878	
Satd. Flow (perm)	1770	3412	0	155	3511	0	1343	1863	1583	0	1606	0
Satd. Flow (RTOR)	40			11							60	
Volume (vph)	5	733	222	600	411	13	77	2	353	20	16	3
Lane Group Flow (vph)	8	1052	0	632	457	0	104	4	364	0	60	0
Turn Type	Prot	pm+pt		Perm	pm+ov	Perm						
Protected Phases	1	6	5	2			4	5		8		
Permitted Phases												
Total Split (s)	12.0	48.0	0.0	52.0	88.0	0.0	20.0	20.0	52.0	20.0	20.0	0.0
Act Effct Green (s)	6.2	48.5		89.4	88.3		12.4	12.4	50.6		12.1	
Actuated g/C Ratio	0.05	0.45		0.83	0.82		0.11	0.11	0.47		0.11	
v/c Ratio	0.08	0.67		0.92	0.16		0.68	0.02	0.47		0.33	
Control Delay	57.0	28.9		46.6	3.2		70.2	45.0	16.0		47.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0		0.0	
Total Delay	57.0	28.9		46.6	3.2		70.2	45.0	16.0		47.5	
LOS	E	C		D	A		E	D	B		D	
Approach Delay		29.1		28.4				28.2			47.5	
Approach LOS		C		C				C			D	

Intersection Summary

Cycle Length: 120
Actuated Cycle Length: 107.2
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.92
Intersection Signal Delay: 29.1
Intersection Capacity Utilization 79.4%
Analysis Period (min) 15

Splits and Phases: 7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy



HCM Signalized Intersection Capacity Analysis
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.99	1.00	1.00	1.00	0.85	0.98	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.98		
Satd. Flow (prot)	1770	3411		1770	3511		1770	1863	1583	1788		
Flt Permitted	0.95	1.00		0.14	1.00		0.73	1.00	1.00	0.86		
Satd. Flow (perm)	1770	3411		267	3511		1353	1863	1583	1576		
Volume (vph)	5	733	222	600	411	13	77	2	353	20	16	3
Peak-hour factor, PHF	0.62	0.92	0.87	0.95	0.95	0.54	0.74	0.50	0.97	0.71	0.67	0.38
Adj. Flow (vph)	8	797	255	632	433	24	104	4	364	28	24	8
RTOR Reduction (vph)	0	21	0	0	2	0	0	0	34	0	5	0
Lane Group Flow (vph)	8	1031	0	632	455	0	104	4	330	0	55	0
Turn Type	Prot	pm+pt		Perm	pm+ov	Perm						
Protected Phases	1	6	5	2			4	5		8		
Permitted Phases												
Actuated Green, G (s)	1.3	51.9		92.7	87.4		10.7	10.7	47.5		10.7	
Effective Green, g (s)	1.3	51.9		92.7	87.4		10.7	10.7	47.5		10.7	
Actuated g/C Ratio	0.01	0.47		0.83	0.78		0.10	0.10	0.43		0.10	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0	4.0		4.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	21	1589		719	2755		130	179	732		151	
v/s Ratio Prot	0.00	0.30		c0.29	0.13		c0.08	0.00	0.15		0.06	
v/c Ratio Perm	0.38	0.65		0.88	0.17		0.80	0.02	0.45		0.37	
Uniform Delay, d1	54.7	22.8		24.4	3.0		49.3	45.6	22.7		47.2	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00		1.00	
Incremental Delay, d2	11.2	0.9		11.8	0.1		28.6	0.1	0.4		1.5	
Delay (s)	65.8	23.7		36.2	3.1		77.9	45.7	23.1		48.7	
Level of Service	E	C		D	A		E	D	C		D	
Approach Delay (s)		24.0		22.3				35.4			48.7	
Approach LOS		C		C				D			D	

Intersection Summary

HCM Average Control Delay	25.9	HCM Level of Service	C
HCM Volume to Capacity ratio	0.86		
Actuated Cycle Length (s)	111.4	Sum of lost time (s)	8.0
Intersection Capacity Utilization	79.4%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

Lanes, Volumes, Timings
8: US 278 (US 78) & Oakdale Rd

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3500	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.063			0.446			0.457			0.482		
Satd. Flow (perm)	117	3539	1583	831	3500	0	851	1863	1583	898	1863	1583
Satd. Flow (RTOR)				76			10			112		
Volume (vph)	133	461	68	207	1243	97	75	125	94	97	320	422
Lane Group Flow (vph)	145	501	76	225	1459	0	88	136	112	115	348	449
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	Perm
Protected Phases	1	6	6	5	2			4		3	8	
Permitted Phases	6			2			4			4	8	
Total Split (s)	16.0	67.0	13.0	64.0	0.0	28.0	28.0	12.0	40.0	40.0	40.0	40.0
Act Eff Green (s)	72.8	63.2	63.2	71.1	62.4	16.7	16.7	16.7	28.5	28.5	28.5	28.5
Actuated g/C Ratio	0.65	0.56	0.56	0.63	0.55	0.15	0.15	0.15	0.25	0.25	0.25	0.25
v/c Ratio	0.67	0.25	0.08	0.38	0.75	0.70	0.49	0.34	0.40	0.74	0.85	0.85
Control Delay	36.7	13.9	3.4	10.0	23.7	73.2	49.8	10.4	37.2	48.4	40.9	40.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.7	13.9	3.4	10.0	23.7	73.2	49.8	10.4	37.2	48.4	40.9	40.9
LOS	D	B	A	A	C	E	D	B	D	D	D	D
Approach Delay	17.4			21.8			42.8			43.3		
Approach LOS	B			C			D			D		

A-11

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 112.5	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.85	
Intersection Signal Delay: 28.2	Intersection LOS: C
Intersection Capacity Utilization 79.1%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 8: US 278 (US 78) & Oakdale Rd



HCM Signalized Intersection Capacity Analysis
8: US 278 (US 78) & Oakdale Rd

Existing PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3500	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.08	1.00	1.00	0.44	1.00	0.41	1.00	1.00	0.42	1.00	1.00	1.00
Satd. Flow (perm)	155	3539	1583	823	3500	762	1863	1583	776	1863	1583	1583
Volume (vph)	133	461	68	207	1243	97	75	125	94	97	320	422
Peak-hour factor, PHF	0.92	0.92	0.90	0.92	0.92	0.90	0.85	0.92	0.84	0.84	0.92	0.94
Adj. Flow (vph)	145	501	76	225	1351	108	88	136	112	115	348	449
RTOR Reduction (vph)	0	0	33	0	4	0	0	0	0	95	0	125
Lane Group Flow (vph)	145	501	43	225	1455	0	88	136	17	115	348	324
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	Perm
Protected Phases	1	6	6	5	2		4		4	3	8	
Permitted Phases	6			2			4			4	8	
Actuated Green, G (s)	72.9	63.3	63.3	71.1	62.4	16.7	16.7	16.7	28.5	28.5	28.5	28.5
Effective Green, g (s)	72.9	63.3	63.3	71.1	62.4	16.7	16.7	16.7	28.5	28.5	28.5	28.5
Actuated g/C Ratio	0.65	0.56	0.56	0.63	0.55	0.15	0.15	0.15	0.25	0.25	0.25	0.25
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	238	1991	891	593	1941	113	277	235	266	472	401	401
v/s Ratio Prot	0.05	0.14		0.03	0.03	0.03	0.07		0.03	0.03	0.19	
v/s Ratio Perm	0.34		0.03	0.21		0.12			0.01	0.08		0.20
v/c Ratio	0.61	0.25	0.05	0.38	0.75	0.78	0.49	0.07	0.43	0.74	0.81	0.81
Uniform Delay, d1	16.7	12.5	11.1	8.8	19.1	46.1	44.0	41.2	33.8	38.6	39.4	39.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	4.4	0.3	0.1	0.4	2.7	27.9	1.4	0.1	1.1	5.9	11.4	11.4
Delay (s)	21.1	12.8	11.2	9.2	21.8	74.1	45.4	41.4	35.0	44.5	50.8	50.8
Level of Service	C	B	B	A	C	E	D	D	C	D	D	D
Approach Delay (s)	14.3			20.1			51.5			46.4		
Approach LOS	B			C			D			D		

Intersection Summary	
HCM Average Control Delay	28.4
HCM Volume to Capacity ratio	0.75
Actuated Cycle Length (s)	112.5
Intersection Capacity Utilization	79.1%
Analysis Period (min)	15
c Critical Lane Group	

Lanes, Volumes, Timings
9: Oak Dr & S Cobb Dr

Existing PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔				↔	↔	↔	↔		↔	↔
Satd. Flow (prot)	0	1619	0	0	0	1611	1770	5085	1583	0	5060	0
Flt Permitted		0.998					0.950					
Satd. Flow (perm)	0	1619	0	0	0	1611	1770	5085	1583	0	5060	0
Volume (vph)	8	0	245	0	0	87	412	1686	114	0	1459	43
Lane Group Flow (vph)	0	281	0	0	0	104	438	1720	128	0	1588	0
Sign Control	Yield											
Intersection Summary	Stop											
Control Type: Unsignalized	Free											
Intersection Capacity Utilization	77.6%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
9: Oak Dr & S Cobb Dr

Existing PM
5/18/2007

Intersection has too many lanes per leg.
HCM All-Way analysis is limited to two lanes per leg.
Channelized right turn lanes are not counted.

Lanes, Volumes, Timings
10: Church Rd & Groover Dr

Existing PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1814	0	0	1859	0	0	1729	0	0	1611	0
Flt Permitted					0.998			0.966				
Satd. Flow (perm)	0	1814	0	0	1859	0	0	1729	0	0	1611	0
Volume (vph)	0	136	20	14	512	0	14	0	7	0	0	3
Lane Group Flow (vph)	0	208	0	0	559	0	0	28	0	0	8	0
Sign Control	Free	Free			Free			Stop			Stop	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 52.8%												
Analysis Period (min) 15												

HCM Unsignalized Intersection Capacity Analysis
10: Church Rd & Groover Dr

Existing PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free		Free	Free			Stop			Stop	
Grade	0%	0%		0%	0%			0%			0%	
Volume (veh/h)	0	136	20	14	512	0	14	0	7	0	0	3
Peak Hour Factor	0.92	0.81	0.50	0.70	0.95	0.92	0.70	0.92	0.88	0.92	0.92	0.38
Hourly flow rate (vph)	0	168	40	20	539	0	20	0	8	0	0	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)							1244					
pX, platoon unblocked												
VC, conflicting volume	539			208			775	767	188	775	787	539
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	539			208			775	767	188	775	787	539
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			93	100	99	100	100	99
cM capacity (veh/h)	1029			1363			307	328	854	309	319	543
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	208	559	28	8								
Volume Left	0	20	20	0								
Volume Right	40	0	8	8								
cSH	1029	1363	376	543								
Volume to Capacity	0.00	0.01	0.07	0.01								
Queue Length 95th (ft)	0	1	6	1								
Control Delay (s)	0.0	0.4	15.4	11.7								
Lane LOS	A	C	C	B								
Approach Delay (s)	0.0	0.4	15.4	11.7								
Approach LOS	C	C	C	B								
Intersection Summary												
Average Delay												
Intersection Capacity Utilization 52.8%												
ICU Level of Service A												
Analysis Period (min) 15												

Existing SAT Intersection Analysis

Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

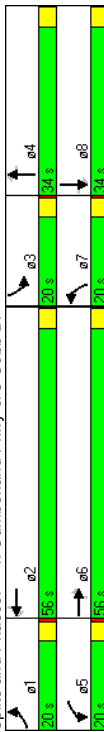
Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Fit Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	232			73			77			77		
Volume (vph)	128	795	239	117	805	140	337	894	108	235	873	118
Lane Group Flow (vph)	152	828	275	136	958	147	362	951	120	261	919	140
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	7	4	3	8			
Permitted Phases												
Total Split (s)	20.0	56.0	34.0	20.0	56.0	34.0	20.0	56.0	34.0	20.0	56.0	34.0
Act Effct Green (s)	14.4	52.7	30.1	13.8	52.0	31.5	15.6	31.5	52.7	14.2	30.1	52.0
Actuated g/C Ratio	0.11	0.41	0.23	0.11	0.41	0.25	0.12	0.25	0.41	0.11	0.23	0.41
v/c Ratio	0.76	0.57	0.50	0.72	0.67	0.33	0.86	1.09	0.17	0.69	1.11	0.20
Control Delay	79.2	31.3	12.1	76.0	34.1	23.2	76.0	104.3	10.7	64.8	110.5	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	79.2	31.3	12.1	76.0	34.1	23.2	76.0	104.3	10.7	64.8	110.5	6.7
LOS	E	C	B	E	C	C	E	F	B	E	F	A
Approach Delay		32.9			37.4			89.3				90.4
Approach LOS		C			D			F				F

A-15

Intersection Summary												
Cycle Length: 130												
Actuated Cycle Length: 128.1												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 1.11												
Intersection Signal Delay: 63.8												
Intersection Capacity Utilization 76.4%												
Analysis Period (min) 15												

Splits and Phases: 1: Cumberland Pkwy & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00
Fit Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Fit Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	128	795	239	117	805	140	337	894	108	235	873	118
Peak-hour factor, PHF	0.84	0.96	0.87	0.86	0.84	0.95	0.93	0.94	0.90	0.90	0.95	0.84
Adj. Flow (vph)	152	828	275	136	958	147	362	951	120	261	919	140
RTOR Reduction (vph)	0	0	178	0	0	55	0	0	45	0	0	71
Lane Group Flow (vph)	152	828	97	136	958	92	362	951	75	261	919	69
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	7	4	3	8			
Permitted Phases												
Actuated Green, G (s)	14.4	52.7	30.1	13.8	52.1	31.5	15.6	31.5	52.7	14.2	30.1	52.1
Effective Green, g (s)	14.4	52.7	30.1	13.8	52.1	31.5	15.6	31.5	52.7	14.2	30.1	52.1
Actuated g/C Ratio	0.11	0.41	0.23	0.11	0.41	0.25	0.12	0.25	0.41	0.11	0.23	0.41
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	199	1455	372	191	1438	389	418	870	651	380	831	643
v/s Ratio Prot	c0.09	0.23	0.06	0.08	c0.27	c0.11	c0.27	0.08	0.08	0.26	0.04	0.04
v/s Ratio Perm	0.76	0.57	0.26	0.71	0.67	0.24	0.87	1.09	0.11	0.69	1.11	0.11
Uniform Delay, d1	55.2	29.0	40.0	55.3	31.0	38.7	55.3	48.3	23.3	54.9	49.0	23.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	15.9	1.6	0.4	11.8	2.5	0.3	16.9	59.1	0.4	5.1	64.4	0.3
Delay (s)	71.1	30.6	40.4	67.1	33.4	39.0	72.2	107.5	23.7	60.0	113.4	23.9
Level of Service	E	C	D	E	C	D	E	F	C	E	F	C
Approach Delay (s)		37.7			37.8			91.5				93.4
Approach LOS		D			D			F				F

Intersection Summary												
HCM Average Control Delay	66.4											
HCM Volume to Capacity ratio	0.81											
Actuated Cycle Length (s)	128.2											
Intersection Capacity Utilization	76.4%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

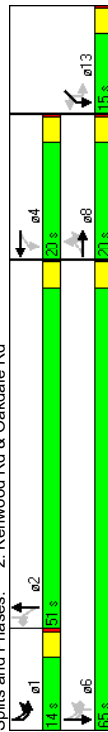
Existing SAT
5/18/2007

Lane Group	EBL2	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	0	0	1709	0	1770	1630	0	0	1770	5075	0
Flt Permitted	0	0	0.832	0	0.566	0.171	0	0	0.171	0	0
Satd. Flow (perm)	0	0	1452	0	1054	1630	0	0	319	5075	0
Satd. Flow (RTOR)	38	13	62	89	11	48	5	86	1230	17	60
Volume (vph)	51	6	13	62	89	11	48	5	86	1230	17
Lane Group Flow (vph)	0	0	152	0	100	72	0	0	89	1286	0
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	custom	custom
Protected Phases	8	8	8	4	4	4	4	2	2	2	6
Permitted Phases	20.0	20.0	20.0	20.0	20.0	20.0	0.0	0.0	51.0	51.0	65.0
Total Split (s)	13.3	13.3	13.3	13.3	13.3	13.3	0.0	0.0	54.3	54.3	61.0
Act Effct Green (s)	0.13	0.13	0.13	0.13	0.13	0.13	0.0	0.0	0.54	0.54	0.61
Actuated g/C Ratio	0.68	0.71	0.33	0.71	0.33	0.51	0.47	0.06	0.51	0.47	0.06
v/c Ratio	45.4	68.2	40.1	24.1	9.1	1.3	0.0	0.0	0.0	0.0	0.0
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	45.4	68.2	40.1	24.1	9.1	1.3	0.0	0.0	0.0	0.0	0.0
Total Delay	D	E	D	E	D	C	A	A	A	A	A
LOS	D	E	D	E	D	C	A	A	A	A	A
Approach Delay	45.4	56.4	45.4	56.4	45.4	56.4	45.4	56.4	45.4	56.4	45.4
Approach LOS	D	E	D	E	D	C	A	A	A	A	A

Intersection Summary

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 52 (52%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.71
Intersection Signal Delay: 15.0
Intersection Capacity Utilization 62.2%
Analysis Period (min) 15

Splits and Phases: 2: Kenwood Rd & Oakdale Rd



Baseline
A & R Engineering Inc.

Synchro 6 Report
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Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Existing SAT
5/18/2007

Lane Group	SBL2	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	0	1770	5085	1583	0	1770	1583
Flt Permitted	0	0.128	0.950	0.950	0	0.128	0
Satd. Flow (perm)	0	238	5085	1583	0	1770	1583
Satd. Flow (RTOR)	9	36	1363	43	3	60	4
Volume (vph)	9	36	1363	43	3	60	4
Lane Group Flow (vph)	0	48	1450	46	0	68	16
Turn Type	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm
Protected Phases	1	1	6	2	13	13	13
Permitted Phases	6	6	2	13	13	13	13
Total Split (s)	14.0	14.0	65.0	51.0	15.0	15.0	0.0
Act Effct Green (s)	61.0	61.0	54.3	13.7	13.7	13.7	0.0
Actuated g/C Ratio	0.61	0.61	0.54	0.14	0.14	0.14	0.0
v/c Ratio	0.20	0.47	0.05	0.28	0.07	0.07	0.0
Control Delay	9.7	11.3	4.2	43.9	24.3	24.3	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.7	11.3	4.2	43.9	24.3	24.3	0.0
LOS	A	B	A	D	C	C	D
Approach Delay	11.0	11.0	40.2	40.2	40.2	40.2	40.2
Approach LOS	B	B	D	D	D	D	D

Intersection Summary

Baseline
A & R Engineering Inc.

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HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Existing SAT
5/18/2007

Movement	EBL2	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations		↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.85	1.00
Flt Protected	0.98	0.98	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1708	1770	1770	1630	1770	1630	1770	5075	5075	1583	1583
Flt Permitted	0.83	0.54	1.00	0.16	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1442	1002	1630	294	5075	1583	1583	1583	1583	1583	1583
Volume (vph)	51	6	13	62	89	11	48	5	86	1230	17
Peak-hour factor, PHF	0.87	0.87	0.87	0.89	0.89	0.89	0.89	0.89	0.97	0.97	0.97
Adj. Flow (vph)	59	7	15	71	100	12	54	6	89	1268	18
RTOR Reduction (vph)	0	0	33	0	0	3	0	0	0	0	24
Lane Group Flow (vph)	0	0	119	0	100	69	0	0	89	1286	0
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	custom	custom
Protected Phases	8	8	8	4	4	4	4	2	2	6	6
Permitted Phases	8	8	8	4	4	4	4	2	2	6	6
Actuated Green, G (s)	13.3	13.3	13.3	13.3	13.3	13.3	13.3	52.7	52.7	61.0	61.0
Effective Green, g (s)	13.3	13.3	13.3	13.3	13.3	13.3	13.3	52.7	52.7	61.0	61.0
Actuated g/C Ratio	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.53	0.53	0.61	0.61
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	192	133	217	155	2675	966	966	966	966	966	966
v/s Ratio Prot	0.08	0.08	0.08	0.04	0.04	0.04	0.04	0.25	0.25	0.02	0.02
v/s Ratio Perm	0.62	0.75	0.32	0.32	0.32	0.32	0.32	0.57	0.48	0.04	0.04
Uniform Delay, d1	41.0	41.8	39.2	16.0	15.0	15.0	15.0	16.0	15.0	7.8	7.8
Progression Factor	1.00	1.00	1.00	0.73	0.58	0.58	0.58	0.73	0.58	0.55	0.55
Incremental Delay, d2	6.1	21.0	0.8	13.1	0.6	0.1	0.1	13.1	0.6	0.1	0.1
Delay (s)	47.0	62.8	40.1	24.8	9.2	4.4	4.4	24.8	9.2	4.4	4.4
Level of Service	D	E	D	C	C	A	A	C	A	A	A
Approach Delay (s)	47.0	53.3	40.1	24.8	9.2	4.4	4.4	24.8	9.2	4.4	4.4
Approach LOS	D	D	D	C	C	A	A	C	A	A	A
Intersection Summary											
HCM Average Control Delay	15.1	HCM Level of Service									
HCM Volume to Capacity ratio	0.56	B									
Actuated Cycle Length (s)	100.0	Sum of lost time (s)									
Intersection Capacity Utilization	62.2%	B									
Analysis Period (min)	15	B									
c Critical Lane Group											

HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Existing SAT
5/18/2007

Movement	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations		↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	0.85	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	1770	1583
Flt Permitted	0.14	1.00	1.00	0.95	1.00	1.00	1.00	1.00
Satd. Flow (perm)	267	5085	1583	1770	5085	1583	1770	1583
Volume (vph)	9	36	1363	43	3	60	4	11
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.93	0.93	0.93	0.93
Adj. Flow (vph)	10	38	1450	46	3	65	4	12
RTOR Reduction (vph)	0	0	0	22	0	0	0	0
Lane Group Flow (vph)	0	48	1450	24	0	68	6	0
Turn Type	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm	Perm
Protected Phases	1	1	6	2	13	13	13	13
Permitted Phases	6	6	6	2	13	13	13	13
Actuated Green, G (s)	61.0	61.0	52.7	13.7	13.7	13.7	13.7	13.7
Effective Green, g (s)	61.0	61.0	52.7	13.7	13.7	13.7	13.7	13.7
Actuated g/C Ratio	0.61	0.61	0.53	0.14	0.14	0.14	0.14	0.14
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	227	3102	834	242	217	217	217	217
v/s Ratio Prot	0.01	0.29	0.02	0.04	0.04	0.04	0.04	0.04
v/s Ratio Perm	0.12	0.47	0.03	0.28	0.28	0.28	0.28	0.28
Uniform Delay, d1	9.4	10.6	11.4	38.7	37.4	37.4	37.4	37.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	0.5	0.1	2.9	0.2	0.2	0.2	0.2
Delay (s)	9.9	11.1	11.4	41.6	37.6	37.6	37.6	37.6
Level of Service	A	B	B	D	D	D	D	D
Approach Delay (s)	11.1	11.1	11.1	40.8	40.8	40.8	40.8	40.8
Approach LOS	B	B	B	D	D	D	D	D
Intersection Summary								

Lanes, Volumes, Timings
3: Highlands Pkwy & S Cobb Dr

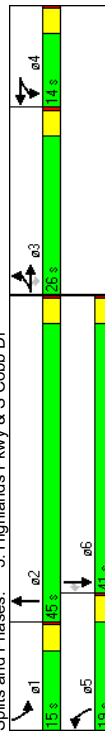
Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1681	1697	1583	0	1780	0	3433	5045	0	1770	5085	1583
Flt Permitted	0.950	0.959			0.972		0.950			0.950		
Satd. Flow (perm)	1681	1697	1583	0	1780	0	3433	5045	0	1770	5085	1583
Satd. Flow (RTOR)	269	289			6		10					
Volume (vph)	234	14	258	59	29	14	255	1141	55	24	1265	260
Lane Group Flow (vph)	135	142	269	0	124	0	277	1240	0	32	1291	277
Turn Type	Split	Perm	Split	Split	Split	Split	Prot	Prot	Prot	Prot	Perm	Perm
Protected Phases	3	3	3	4	4	4	5	2		1	6	
Permitted Phases												
Total Split (s)	26.0	26.0	26.0	14.0	14.0	0.0	19.0	45.0	0.0	15.0	41.0	41.0
Act Effct Green (s)	13.6	13.6	13.6	11.5	11.5	12.6	55.7	7.1	46.3	46.3		
Actuated g/C Ratio	0.14	0.14	0.14	0.12	0.12	0.13	0.56	0.07	0.46	0.46		
v/c Ratio	0.59	0.61	0.60	0.59	0.59	0.64	0.44	0.25	0.55	0.35		
Control Delay	50.3	51.5	10.6	51.3	39.0	23.9	52.0	16.7	8.6	0.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.3	51.5	10.6	51.3	39.0	23.9	52.0	16.7	8.6			
LOS	D	D	B	D	D	C	D	C	D	B	B	A
Approach Delay	31.1			51.3			26.6			16.0		
Approach LOS	C			D			C			B		

Intersection Summary

Cycle Length: 100
Actuated Cycle Length: 100
Offset: 69 (69%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.64
Intersection Signal Delay: 23.6
Intersection Capacity Utilization 56.1%
Analysis Period (min) 15

Splits and Phases: 3: Highlands Pkwy & S Cobb Dr



Baseline
A & R Engineering Inc.

Synchro 6 Report
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HCM Signalized Intersection Capacity Analysis
3: Highlands Pkwy & S Cobb Dr

Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	1.00	1.00	0.98	1.00	0.99	1.00	0.99	1.00	1.00	0.85
Flt Protected	0.95	0.96	1.00	0.97	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.85
Satd. Flow (prot)	1681	1697	1583	1779	3433	5046	1770	5085	1583			
Flt Permitted	0.95	0.96	1.00	0.97	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.85
Satd. Flow (perm)	1681	1697	1583	1779	3433	5046	1770	5085	1583			
Volume (vph)	234	14	258	59	29	14	255	1141	55	24	1265	260
Peak-hour factor, PHF	0.91	0.70	0.96	0.82	0.81	0.88	0.92	0.97	0.86	0.75	0.98	0.94
Adj. Flow (vph)	257	20	269	72	36	16	277	1176	64	32	1291	277
RTOR Reduction (vph)	0	0	232	0	5	0	0	5	0	0	0	61
Lane Group Flow (vph)	135	142	37	0	119	0	277	1235	0	32	1291	216
Turn Type	Split	Perm	Split	Split	Split	Split	Prot	Prot	Prot	Prot	Perm	Perm
Protected Phases	3	3	3	4	4	4	5	2		1	6	
Permitted Phases												
Actuated Green, G (s)	13.6	13.6	13.6	11.5	11.5	12.6	54.1	4.8	46.3	46.3		
Effective Green, g (s)	13.6	13.6	13.6	11.5	11.5	12.6	54.1	4.8	46.3	46.3		
Actuated g/C Ratio	0.14	0.14	0.14	0.12	0.12	0.13	0.54	0.05	0.46	0.46		
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	229	231	215	205	433	2730	85	2354	733			
v/s Ratio Prot	0.08	c0.08	0.02	c0.07	c0.08	0.24				0.02	c0.25	
v/s Ratio Perm	0.59	0.61	0.17	0.58	0.64	0.45				0.38	0.55	0.29
Uniform Delay, d1	40.6	40.7	38.2	42.0	41.5	13.9	46.1	19.3	16.7			
Progression Factor	1.00	1.00	1.00	1.00	0.82	1.55				1.10	0.75	0.63
Incremental Delay, d2	3.8	4.8	0.4	3.9	2.2	0.4				2.5	0.8	0.9
Delay (s)	44.4	45.5	38.6	45.9	36.4	22.0	53.1	15.4	11.4			
Level of Service	D	D	D	D	D	C				D	B	B
Approach Delay (s)	41.8			45.9			24.6			15.5		
Approach LOS	D			D			C			B		

Intersection Summary	
HCM Average Control Delay	23.9
HCM Volume to Capacity ratio	0.58
Actuated Cycle Length	100.0
Intersection Capacity Utilization	56.1%
Analysis Period (min)	15
c Critical Lane Group	

Baseline
A & R Engineering Inc.

Synchro 6 Report
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Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

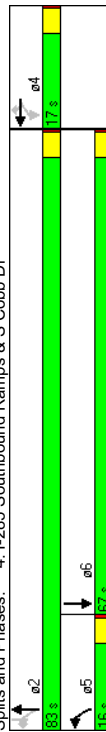
Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	0	1681	1459	1504	1770	3539	0	0	3539
Satd. Flow (prot)	0	0	0	0.950	0.995	0.202					
Flt Permitted	0	0	0	1681	1459	1504	376	3539	0	0	3539
Satd. Flow (RTOR)	0	0	0	185	185						
Volume (vph)	0	0	0	237	0	742	59	1015	0	0	1031
Satd. Flow (prot)	0	0	0	250	444	414	72	1068	0	0	1063
Lane Group Flow (vph)	0	0	0	250	444	414	72	1068	0	0	1063
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	5	2	2	6				
Permitted Phases	0.0	0.0	0.0	17.0	17.0	17.0	16.0	83.0	0.0	0.0	67.0
Total Split (s)	0.0	0.0	0.0	13.0	13.0	13.0	79.0	79.0	0.0	0.0	83.0
Act Effct Green (s)	0.13	0.13	0.13	0.13	0.13	0.13	0.79	0.79	0.71	0.79	0.79
Actuated g/C Ratio	1.14	1.26	1.16	0.19	0.38	0.42	0.48				
Control Delay	145.1	163.2	123.1	1.8	2.2	4.4	4.2				
Queue Delay	32.1	23.1	0.0	0.0	0.2	0.0	0.0				
Total Delay	177.2	186.3	123.1	1.8	2.5	4.4	4.2				
LOS	F	F	F	A	A	A	A				
Approach Delay	160.6										
Approach LOS	F										

A-19

Intersection Summary											
Cycle Length: 100											
Actuated Cycle Length: 100											
Offset: 52 (52%), Referenced to phase 2:NBTL and 6:SBT, Start of Green											
Control Type: Actuated-Coordinated											
Maximum v/c Ratio: 1.26											
Intersection Signal Delay: 47.4											
Intersection Capacity Utilization 93.3%											
ICU Level of Service F											
Analysis Period (min) 15											

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
4: I-285 Southbound Ramps & S Cobb Dr

Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.91	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.85
Flt Protected	1.00	0.86	0.85	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1681	1458	1504	1770	3539	3539	1583				
Flt Permitted	0.95	1.00	1.00	0.22	1.00	1.00	1.00				
Satd. Flow (perm)	1681	1458	1504	411	3539	3539	1583				
Volume (vph)	0	0	0	237	0	742	59	1015	0	0	1031
Peak-hour factor, PHF	0.92	0.92	0.92	0.81	0.92	0.81	0.82	0.95	0.92	0.92	0.95
Adj. Flow (vph)	0	0	0	293	0	815	72	1068	0	0	1063
RTOR Reduction (vph)	0	0	0	0	161	161	0	0	0	0	139
Lane Group Flow (vph)	0	0	0	250	283	253	72	1068	0	0	1063
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	5	2	2	6				
Permitted Phases	4	4	4	2							
Actuated Green, G (s)	13.0	13.0	13.0	79.0	79.0	79.0	70.2	79.0			
Effective Green, g (s)	13.0	13.0	13.0	79.0	79.0	79.0	70.2	79.0			
Actuated g/C Ratio	0.13	0.13	0.13	0.79	0.79	0.79	0.70	0.79			
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0			
Lane Grp Cap (vph)	219	190	196	390	2796		2484	1251			
v/s Ratio Prot	0.15	0.19	0.17	0.14			0.30				
v/s Ratio Perm	1.14	1.49	1.29	0.18			0.38				
Uniform Delay, d1	43.5	43.5	43.5	3.4	3.2		6.3				
Progression Factor	1.00	1.00	1.00	0.63	0.66		0.60				
Incremental Delay, d2	104.2	246.2	163.6	0.1	0.1		0.5				
Delay (s)	147.7	289.7	207.1	2.2	2.2		4.3				
Level of Service	F	F	F	A	A		A				
Approach Delay (s)	0.0	0.0	0.0	226.8			20.6				
Approach LOS	A	A	A	F			C				
Intersection Summary											
HCM Average Control Delay	72.9										
HCM Volume to Capacity ratio	0.57										
Actuated Cycle Length (s)	100.0										
Intersection Capacity Utilization	93.3%										
Analysis Period (min)	15										
c Critical Lane Group											

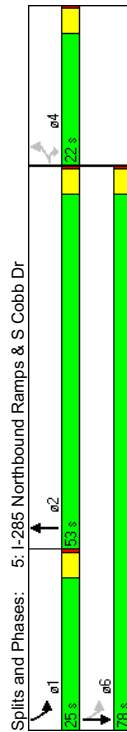
Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	0	3302	0	1770	3539	0
Flt Permitted	0.950							0.293				
Satd. Flow (perm)	3433	0	1583	0	0	0	0	3302	0	546	3539	0
Satd. Flow (RTOR)												
Volume (vph)	705	0	53	0	0	0	0	359	280	800	517	0
Lane Group Flow (vph)	766	0	60	0	0	0	0	694	0	833	550	0
Turn Type	custom	custom	custom							pm+pt		
Protected Phases	4		4					2		1	6	
Permitted Phases	22.0	0.0	22.0	0.0	0.0	0.0	0.0	53.0	0.0	25.0	78.0	0.0
Act Effct Green (s)	18.0		18.0					49.0		74.0	74.0	
Actuated g/C Ratio	0.18		0.18					0.49		0.74	0.74	
Control Delay	157.4		10.9					9.8		144.1	2.8	
Queue Delay	0.0		0.0					0.0		69.6	0.0	
Total Delay	157.4		10.9					9.8		213.7	2.8	
LOS	F		B					A		F	A	
Approach Delay								9.8			129.8	
Approach LOS								A			F	

A-20

Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 22 (22%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.26												
Intersection Signal Delay: 105.9												
Intersection Capacity Utilization 93.3%												
Analysis Period (min) 15												



Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	1.00	0.85	1.00	0.95	1.00	0.93	1.00	1.00	1.00	0.95
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)	3433	1583	1583	1583	1583	1583	3304	3304	1770	3539	3539	1770
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.29	1.00	1.00	0.95
Satd. Flow (perm)	3433	1583	1583	1583	1583	1583	3304	3304	545	3539	3539	545
Volume (vph)	705	0	53	0	0	0	0	359	280	800	517	0
Peak-hour factor, PHF	0.92	0.92	0.88	0.92	0.92	0.92	0.93	0.91	0.96	0.94	0.92	0.92
Adj. Flow (vph)	766	0	60	0	0	0	0	386	308	833	550	0
RTOR Reduction (vph)	0	0	49	0	0	0	0	144	0	0	0	0
Lane Group Flow (vph)	766	0	11	0	0	0	0	550	0	833	550	0
Turn Type	custom	custom	custom							pm+pt		
Protected Phases	4		4					2		1	6	
Permitted Phases	18.0		18.0					49.0		74.0	74.0	
Actuated Green, G (s)	18.0		18.0					49.0		74.0	74.0	
Effective Green, g (s)	18.0		18.0					49.0		74.0	74.0	
Actuated g/C Ratio	0.18		0.18					0.49		0.74	0.74	
Clearance Time (s)	4.0		4.0					4.0		4.0	4.0	
Vehicle Extension (s)	3.0		3.0					3.0		3.0	3.0	
Lane Grp Cap (vph)	618		285					1619		661	2619	
v/s Ratio Prot	c0.22		0.01					0.17		c0.26	0.16	
v/s Ratio Perm	1.24		0.04					0.34		1.26	0.21	
Uniform Delay, d1	41.0		33.9					15.6		14.8	4.0	
Progression Factor	1.00		1.00					1.00		0.93	0.85	
Incremental Delay, d2	121.2		0.1					0.6		127.1	0.2	
Delay (s)	162.2		33.9					16.2		140.9	2.8	
Level of Service	F		C					B		F	A	
Approach Delay (s)	152.9		F					16.2		85.9	F	
Approach LOS	F		F					B		F	F	
Intersection Summary												
HCM Average Control Delay	88.3											
HCM Volume to Capacity ratio	1.24											
Actuated Cycle Length (s)	100.0											
Intersection Capacity Utilization	93.3%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
6: Church Rd & N Church Lane

Existing SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1818	0	0	1781	0	0	1736	0	0	1693	0
Flt Permitted	0	1818	0	0	1781	0	0	1736	0	0	1693	0
Satd. Flow (perm)	0	1818	0	0	1781	0	0	1736	0	0	1693	0
Volume (vph)	29	61	1	0	63	35	0	1	1	38	0	41
Lane Group Flow (vph)	0	113	0	0	124	0	0	8	0	0	100	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 29.5%												
Analysis Period (min) 15												

HCM Unsignalized Intersection Capacity Analysis
6: Church Rd & N Church Lane

Existing SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	29	61	1	0	63	35	0	1	1	38	0	41
Peak Hour Factor	0.66	0.94	0.25	0.92	0.75	0.88	0.92	0.25	0.25	0.79	0.92	0.79
Hourly flow rate (vph)	44	65	4	0	84	40	0	4	4	48	0	52
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	84			69			311	239	67	265	261	104
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
vCu, unblocked vol	84			69			311	239	67	265	261	104
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	97			100			100	99	100	93	100	95
cM capacity (veh/h)	1513			1532			593	643	997	667	625	951
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	113	124	8	100								
Volume Left	44	0	0	48								
Volume Right	4	40	4	52								
cSH	1513	1532	782	789								
Volume to Capacity	0.03	0.00	0.01	0.13								
Queue Length 95th (ft)	2	0	1	11								
Control Delay (s)	3.0	0.0	9.7	10.2								
Lane LOS	A	A	A	B								
Approach Delay (s)	3.0	0.0	9.7	10.2								
Approach LOS	A	B	A	B								
Intersection Summary												
Average Delay				4.2								
Intersection Capacity Utilization				29.5%						A		
Analysis Period (min)				15								

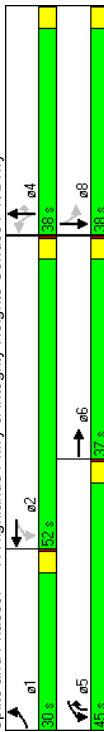
Lanes, Volumes, Timings
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3433	0	1770	3529	0	1770	1863	1583	0	1863	0
Fit Permitted	0.950			0.478			0.755					
Satd. Flow (perm)	1770	3433	0	890	3529	0	1406	1863	1583	0	1863	0
Satd. Flow (RTOR)	26			2								
Volume (vph)	1	197	41	293	187	1	35	1	286	0	2	0
Lane Group Flow (vph)	4	259	0	305	212	0	44	1	304	0	4	0
Turn Type	Prot	pm+pt	Perm	pm+pt	Perm	pm+ov	Perm	pm+ov	Perm	pm+ov	Perm	pm+ov
Protected Phases	1	6	5	2	2	4	4	5	8			
Permitted Phases	30.0	37.0	0.0	45.0	52.0	0.0	38.0	38.0	45.0	38.0	38.0	0.0
Total Split (s)	5.8	30.9		51.7	51.0		7.5	7.5	26.7			7.4
Act Effct Green (s)	0.08	0.48		0.84	0.83		0.11	0.11	0.43			0.11
Actuated g/C Ratio	0.03	0.15		0.30	0.07		0.27	0.00	0.35			0.02
Control Delay	33.0	8.1		2.8	3.0		31.9	27.0	3.5			27.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0			0.0
Total Delay	33.0	8.1		2.8	3.0		31.9	27.0	3.5			27.0
LOS	C	A		A	A		C	C	A			C
Approach Delay	8.5			2.9			7.2					27.0
Approach LOS	A			A			A					C

Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 61.8												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.35												
Intersection Signal Delay: 5.6												
Intersection Capacity Utilization 41.6%												
Analysis Period (min) 15												

Splits and Phases: 7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy



HCM Signalized Intersection Capacity Analysis
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.85	1.00	1.00
Fit Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1770	3433	1770	3529	1770	3529	1770	1863	1583	1863	1863	1863
Fit Permitted	0.95	1.00	0.52	1.00	0.83	1.00	0.83	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1770	3433	977	3529	1552	1863	1552	1863	1583	1583	1863	1863
Volume (vph)	1	197	41	293	187	1	35	1	286	0	2	0
Peak-hour factor, PHF	0.25	0.95	0.79	0.96	0.90	0.25	0.80	0.92	0.94	0.92	0.50	0.92
Adj. Flow (vph)	4	207	52	305	208	4	44	1	304	0	4	0
RTOR Reduction (vph)	0	14	0	0	1	0	0	0	197	0	0	0
Lane Group Flow (vph)	4	245	0	305	211	0	44	1	107	0	4	0
Turn Type	Prot	pm+pt	Perm	pm+pt	Perm	pm+ov	Perm	pm+ov	Perm	pm+ov	Perm	pm+ov
Protected Phases	1	6	5	2	2	4	4	5	8			
Permitted Phases	1.0	31.3		54.1	49.1		4.8	4.8	23.6			4.8
Actuated Green, G (s)	1.0	31.3		54.1	49.1		4.8	4.8	23.6			4.8
Effective Green, g (s)	0.01	0.47		0.81	0.73		0.07	0.07	0.35			0.07
Actuated g/C Ratio	4.0	4.0		4.0	4.0		4.0	4.0	4.0			4.0
Clearance Time (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			3.0
Vehicle Extension (s)	26	1606		1013	2590		111	134	653			134
Lane Grp Cap (vph)	0.00	0.07		c0.08	0.06		0.00	c0.05	0.00			0.00
v/s Ratio Prot	0.15	0.15		0.30	0.08		0.40	0.01	0.16			0.03
v/c Ratio	32.5	10.2		1.6	2.5		29.7	28.8	14.9			28.9
Uniform Delay, d1	1.00	1.00		1.00	1.00		1.00	1.00	1.00			1.00
Progression Factor	2.7	0.0		0.2	0.1		2.3	0.0	0.1			0.1
Incremental Delay, d2	35.3	10.2		1.8	2.6		32.0	28.9	15.0			29.0
Delay (s)	D	B		A	A		C	C	B			C
Level of Service	D	B		A	A		C	C	B			C
Approach Delay (s)	10.6			2.1			17.2					29.0
Approach LOS	B			A			B					C
Intersection Summary												
HCM Average Control Delay	8.8	HCM Level of Service										
HCM Volume to Capacity ratio	0.29	A										
Actuated Cycle Length (s)	66.9	Sum of lost time (s)										
Intersection Capacity Utilization	41.6%	ICU Level of Service										
Analysis Period (min)	15	A										
c Critical Lane Group												

Lanes, Volumes, Timings
8: US 278 (US 78) & Oakdale Rd

Existing SAT
5/18/2007

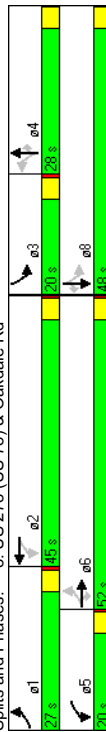
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3497	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.201	0.315	0.315	0.695	0.695	0	0.695	0.537	0.695	0.537	0.695	0.537
Satd. Flow (perm)	374	3539	1583	587	3497	0	1295	1863	1583	1000	1863	1583
Satd. Flow (RTOR)	68	68	68	68	68	0	68	68	68	68	68	68
Volume (vph)	139	629	61	69	605	49	59	92	53	49	74	170
Lane Group Flow (vph)	156	699	72	80	721	0	76	108	56	60	96	189
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt
Protected Phases	1	6	5	2	4	4	4	4	4	3	8	8
Permitted Phases	6	6	2	2	4	4	4	4	4	3	8	8
Total Split (s)	27.0	52.0	20.0	45.0	0.0	28.0	28.0	20.0	20.0	48.0	48.0	20.2
Act Effct Green (s)	58.3	51.5	51.5	55.8	50.4	10.6	10.6	10.6	10.6	20.4	20.2	20.2
Actuated g/C Ratio	0.64	0.58	0.58	0.62	0.57	0.12	0.12	0.12	0.12	0.22	0.23	0.23
v/c Ratio	0.42	0.34	0.08	0.18	0.36	0.49	0.49	0.24	0.21	0.23	0.37	0.37
Control Delay	10.2	12.8	4.1	7.7	13.8	49.0	45.0	12.9	27.5	27.7	6.3	6.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.2	12.8	4.1	7.7	13.8	49.0	45.0	12.9	27.5	27.7	6.3	6.3
LOS	B	B	A	A	B	D	D	B	B	C	C	A
Approach Delay	11.7	11.7	13.2	13.2	13.2	13.2	13.2	38.7	38.7	16.0	16.0	16.0
Approach LOS	B	B	B	B	B	B	B	D	D	B	B	B

A-23

Intersection Summary

Cycle Length: 120
Actuated Cycle Length: 88.8
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.49
Intersection Signal Delay: 15.7
Intersection Capacity Utilization 45.9%
Analysis Period (min) 15

Splits and Phases: 8: US 278 (US 78) & Oakdale Rd



Baseline
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HCM Signalized Intersection Capacity Analysis
8: US 278 (US 78) & Oakdale Rd

Existing SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.99	1.00	1.00	1.00	0.85	1.00	0.85
Satd. Flow (prot)	1770	3539	1583	1770	3498	1770	1863	1583	1770	1863	1583	1583
Flt Permitted	0.32	1.00	1.00	0.35	1.00	0.69	1.00	1.00	1.00	0.45	1.00	1.00
Satd. Flow (perm)	595	3539	1583	650	3498	1294	1863	1583	833	1863	1583	1583
Volume (vph)	139	629	61	69	605	49	59	92	53	49	74	170
Peak-hour factor, PHF	0.89	0.90	0.85	0.86	0.91	0.88	0.78	0.85	0.95	0.82	0.77	0.90
Adj. Flow (vph)	156	699	72	80	665	56	76	108	56	60	96	189
RTOR Reduction (vph)	0	0	29	0	4	0	0	0	49	0	0	145
Lane Group Flow (vph)	156	699	43	80	717	0	76	108	7	60	96	44
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt
Protected Phases	1	6	5	2	4	4	4	4	4	3	8	8
Permitted Phases	6	6	2	2	4	4	4	4	4	3	8	8
Actuated Green, G (s)	58.7	51.6	51.6	56.3	50.4	10.6	10.6	10.6	10.6	21.1	21.1	21.1
Effective Green, g (s)	58.7	51.6	51.6	56.3	50.4	10.6	10.6	10.6	10.6	21.1	21.1	21.1
Actuated g/C Ratio	0.65	0.57	0.57	0.62	0.56	0.12	0.12	0.12	0.12	0.23	0.23	0.23
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	478	2016	902	477	1946	151	218	185	261	434	369	369
v/s Ratio Prot	c0.03	0.20	0.01	c0.21	c0.21	c0.06	0.06	0.06	0.02	c0.05	0.03	0.03
v/s Ratio Perm	0.19	0.03	0.05	0.17	0.37	0.50	0.50	0.50	0.04	0.23	0.22	0.12
Uniform Delay, d1	6.6	10.5	8.6	6.9	11.2	37.5	37.5	37.5	35.5	27.7	28.1	27.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.5	0.1	0.2	0.5	2.6	1.8	0.1	0.5	0.3	0.1	0.1
Delay (s)	7.0	10.9	8.7	7.1	11.8	40.2	39.3	35.5	35.5	28.2	28.4	27.6
Level of Service	A	B	A	A	B	D	D	D	D	C	C	C
Approach Delay (s)	10.1	11.3	11.3	11.3	11.3	38.7	38.7	38.7	38.7	27.9	27.9	27.9
Approach LOS	B	B	B	B	B	D	D	D	D	C	C	C

Intersection Summary
HCM Average Control Delay 16.1 HCM Level of Service B
HCM Volume to Capacity ratio 0.38
Actuated Cycle Length (s) 90.6 Sum of lost time (s) 16.0
Intersection Capacity Utilization 45.9% ICU Level of Service A
Analysis Period (min) 15
c Critical Lane Group

Baseline
A & R Engineering Inc.

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Lanes, Volumes, Timings
9: Oak Dr & S Cobb Dr

Existing SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔				↔	↔	↔	↔	↔	↔	↔
Satd. Flow (prot)	0	1625	0	0	0	1611	1770	5085	1583	0	5080	0
Flt Permitted		0.996					0.950					
Satd. Flow (perm)	0	1625	0	0	0	1611	1770	5085	1583	0	5080	0
Volume (vph)	15	0	208	0	0	48	215	1465	46	0	1607	12
Lane Group Flow (vph)	0	244	0	0	0	60	269	1542	56	0	1690	0
Sign Control		Stop			Yield		Free				Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 66.9%	ICU Level of Service C											
Analysis Period (min) 15												

HCM Unsignalized Intersection Capacity Analysis
9: Oak Dr & S Cobb Dr

Existing SAT
5/18/2007

Intersection has too many lanes per leg.
HCM All-Way analysis is limited to two lanes per leg.
Channelized right turn lanes are not counted.

Lanes, Volumes, Timings
10: Church Rd & Groover Rd

Existing SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1820	0	0	1857	0	0	1715	0	0	1611	0
Flt Permitted	0	0.999	0	0	0.997	0	0	0.970	0	0	0	0
Satd. Flow (perm)	0	1820	0	0	1857	0	0	1715	0	0	1611	0
Volume (vph)	2	92	18	7	95	0	16	0	7	0	0	2
Lane Group Flow (vph)	0	147	0	0	128	0	0	32	0	0	4	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	23.5%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
10: Church Rd & Groover Rd

Existing SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	2	92	18	7	95	0	16	0	7	0	0	2
Peak Hour Factor	0.50	0.77	0.75	0.88	0.79	0.92	0.80	0.92	0.58	0.92	0.92	0.50
Hourly flow rate (vph)	4	119	24	8	120	0	20	0	12	0	0	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				None
Median storage (veh)												
Upstream signal (ft)								1244				
pX, platoon unblocked												
VC, conflicting volume	120			143			280	276	131	288	288	120
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	120			143			280	276	131	288	288	120
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			97	100	99	100	100	100
cM capacity (veh/h)	1467			1439			666	627	918	652	617	931
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	147	128	32	4								
Volume Left	4	8	20	0								
Volume Right	24	0	12	4								
cSH	1467	1439	742	931								
Volume to Capacity	0.00	0.01	0.04	0.00								
Queue Length 95th (ft)	0	0	3	0								
Control Delay (s)	0.2	0.5	10.1	8.9								
Lane LOS	A	A	B	A								
Approach Delay (s)	0.2	0.5	10.1	8.9								
Approach LOS	B	B	A	A								
Intersection Summary												
Average Delay	1.5											
Intersection Capacity Utilization	23.5%											
Analysis Period (min)	15											

Base Intersection Analysis

Base 2009 PM

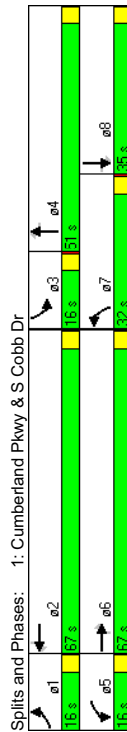
Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	95			31			54			54		
Volume (vph)	177	907	130	137	2033	129	569	1217	108	207	895	98
Lane Group Flow (vph)	203	945	153	152	2140	143	618	1352	132	233	923	124
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	7	4	3	8			
Permitted Phases												
Total Split (s)	16.0	67.0	35.0	16.0	67.0	51.0	32.0	51.0	67.0	16.0	35.0	67.0
Act Eff Green (s)	12.0	63.0	31.0	12.0	63.0	47.0	28.0	47.0	63.0	12.0	31.0	63.0
Actuated g/C Ratio	0.08	0.42	0.21	0.08	0.42	0.31	0.19	0.31	0.42	0.08	0.21	0.42
v/c Ratio	1.43	0.64	0.38	1.07	1.44	0.28	0.96	1.22	0.19	0.85	1.26	0.17
Control Delay	275.0	36.8	23.2	158.1	235.6	31.7	87.7	150.4	16.7	94.0	176.2	8.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	275.0	36.8	23.2	158.1	235.6	31.7	87.7	150.4	16.7	94.0	176.2	8.5
LOS	F	D	C	F	F	C	F	F	B	F	F	A
Approach Delay		72.4			218.8			123.6			145.0	
Approach LOS		E			F			F			F	

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Intersection Summary	
Cycle Length: 150	
Actuated Cycle Length: 150	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.44	
Intersection Signal Delay: 150.7	Intersection LOS: F
Intersection Capacity Utilization 120.3%	ICU Level of Service H
Analysis Period (min) 15	



HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	177	907	130	137	2033	129	569	1217	108	207	895	98
Peak-hour factor, PHF	0.87	0.96	0.85	0.90	0.95	0.90	0.92	0.90	0.82	0.89	0.97	0.79
Adj. Flow (vph)	203	945	153	152	2140	143	618	1352	132	233	923	124
RTOR Reduction (vph)	0	0	75	0	0	21	0	0	31	0	0	55
Lane Group Flow (vph)	203	945	78	152	2140	122	618	1352	101	233	923	69
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	7	4	3	8			
Permitted Phases												
Actuated Green, G (s)	12.0	63.0	31.0	12.0	63.0	47.0	28.0	47.0	63.0	12.0	31.0	63.0
Effective Green, g (s)	12.0	63.0	31.0	12.0	63.0	47.0	28.0	47.0	63.0	12.0	31.0	63.0
Actuated g/C Ratio	0.08	0.42	0.21	0.08	0.42	0.31	0.19	0.31	0.42	0.08	0.21	0.42
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	142	1486	327	142	1486	496	641	1109	665	275	731	665
v/s Ratio Prot	c0.11	0.27	0.05	0.09	c0.60	0.08	c0.18	c0.38	0.06	0.07	0.26	0.04
v/s Ratio Perm	1.43	0.64	0.24	1.07	1.44	0.25	0.96	1.22	0.15	0.85	1.26	0.10
Uniform Delay, d1	69.0	34.4	49.6	69.0	43.5	38.3	60.5	51.5	26.9	68.1	59.5	26.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	228.9	2.1	0.4	95.6	201.9	0.3	26.6	106.9	0.5	20.8	129.0	0.3
Delay (s)	297.9	36.5	50.0	164.6	245.4	38.6	87.1	158.4	27.4	88.8	188.5	26.7
Level of Service	F	D	D	F	F	D	F	F	C	F	F	C
Approach Delay (s)		78.9			228.2			129.2			154.7	
Approach LOS		E			F			F			F	

Intersection Summary	
HCM Average Control Delay	158.5
HCM Volume to Capacity ratio	1.33
Actuated Cycle Length (s)	150.0
Intersection Capacity Utilization	120.3%
Analysis Period (min)	15
Critical Lane Group	

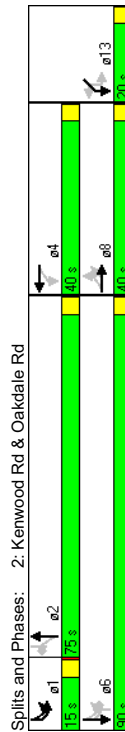
Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Base PM
5/18/2007

Lane Group	EBL2	EBL	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations												
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	0	0	1725	0	1770	1639	0	0	1770	5075	0	1583
Flt Permitted			0.686		0.607				0.213			
Satd. Flow (perm)	0	0	1206	0	1131	1639	0	0	397	5075	0	1583
Satd. Flow (RTOR)			23		1							48
Volume (vph)	37	3	19	42	367	40	151	6	101	1822	29	100
Lane Group Flow (vph)	0	0	134	0	378	203	0	0	102	1869	0	101
Turn Type	Perm	Perm			Perm				Perm		custom	
Protected Phases	8	8		8		4				2		6
Permitted Phases												
Total Split (s)	40.0	40.0	40.0	0.0	40.0	40.0	0.0	0.0	75.0	75.0	0.0	90.0
Act Effct Green (s)			36.0		36.0	36.0			76.6	76.6		86.0
Actuated g/C Ratio			0.24		0.24	0.24			0.51	0.51		0.57
v/c Ratio			0.44		1.39	0.52			0.50	0.72		0.11
Control Delay			45.0		240.0	54.6			27.2	25.0		5.2
Queue Delay			0.0		0.0	0.0			0.0	6.2		0.0
Total Delay			45.0		240.0	54.6			27.2	31.2		5.2
LOS			D		F	D			C	C		A
Approach Delay			45.0		175.3				29.7			C
Approach LOS			D		F				C			C

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Intersection Summary	
Cycle Length: 150	
Actuated Cycle Length: 150	
Offset: 140 (93%), Referenced to phase 2:NBT and 6:SBTL, Start of Green	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 1.39	
Intersection Signal Delay: 49.4	Intersection LOS: D
Intersection Capacity Utilization 89.9%	ICU Level of Service E
Analysis Period (min) 15	



Baseline
A & R Engineering Inc.

Synchro 6 Report
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Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Base PM
5/18/2007

Lane Group	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations								
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	0	1770	5085	1583	0	1770	1583	0
Flt Permitted		0.053			0.950			
Satd. Flow (perm)	0	99	5085	1583	0	1770	1583	0
Satd. Flow (RTOR)								9
Volume (vph)	2	48	1191	33	4	140	11	8
Lane Group Flow (vph)	0	51	1215	34	0	164	21	0
Turn Type	pm+pt	pm+pt	custom	Perm	Perm			
Protected Phases	1	1	6		13			
Permitted Phases	6	6		2	13			
Total Split (s)	15.0	15.0	90.0	75.0	20.0	20.0	0.0	0.0
Act Effct Green (s)		86.0	86.0	76.6		16.0		16.0
Actuated g/C Ratio		0.57	0.57	0.51		0.11		0.11
v/c Ratio		0.37	0.42	0.04		0.87		0.12
Control Delay		21.6	18.5	6.1		103.5		42.9
Queue Delay		0.0	0.0	0.0		0.0		0.0
Total Delay		21.6	18.5	6.1		103.5		42.9
LOS		C	B	A		F		D
Approach Delay			18.3			96.7		F
Approach LOS			B			F		F

Intersection Summary

Baseline
A & R Engineering Inc.

Synchro 6 Report
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HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Base PM
5/18/2007

Movement	EBL2	EBL	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations		↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.88	1.00	1.00	1.00	1.00	0.85	1.00
Flt Protected	0.98	0.98	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1724	1770	1640	1770	1640	1770	1583	1770	1583	1770	1583	1583
Flt Permitted	0.69	0.61	1.00	0.20	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1206	1131	1640	371	5073	1583	1583	371	5073	1583	1583	1583
Volume (vph)	37	3	19	42	367	40	151	6	101	1822	29	100
Peak-hour factor, PHF	0.75	0.75	0.75	0.97	0.97	0.97	0.97	0.97	0.99	0.99	0.99	0.99
Adj. Flow (vph)	49	4	25	56	378	41	156	6	102	1840	29	101
RTOR Reduction (vph)	0	0	17	0	0	1	0	0	0	0	0	20
Lane Group Flow (vph)	0	0	117	0	378	202	0	0	102	1869	0	81
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	custom	custom
Protected Phases	8	8	8	4	4	4	4	2	2	2	6	6
Permitted Phases	8	8	8	4	4	4	4	2	2	2	6	6
Actuated Green, G (s)	36.0	36.0	36.0	36.0	36.0	36.0	36.0	75.8	75.8	75.8	86.0	86.0
Effective Green, g (s)	36.0	36.0	36.0	36.0	36.0	36.0	36.0	75.8	75.8	75.8	86.0	86.0
Actuated g/C Ratio	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.51	0.51	0.51	0.57	0.57
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	289	271	394	0.12	187	2564	908	0.37	0.27	0.27	0.05	0.05
v/s Ratio Prot	0.10	0.33	0.51	0.51	0.51	0.51	0.51	0.73	0.73	0.73	0.09	0.09
v/s Ratio Perm	0.40	1.39	0.51	0.51	0.51	0.51	0.51	0.73	0.73	0.73	0.09	0.09
Uniform Delay, d1	48.0	57.0	49.4	25.3	29.1	25.3	29.1	14.4	14.4	14.4	14.4	14.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.75	0.81	0.81	0.64	0.64
Incremental Delay, d2	0.9	198.7	1.1	8.0	1.3	8.0	1.3	0.1	0.1	0.1	0.1	0.1
Delay (s)	48.9	255.7	50.5	27.1	24.9	24.9	24.9	9.3	9.3	9.3	9.3	9.3
Level of Service	D	F	D	C	C	C	C	A	A	A	A	A
Approach Delay (s)	48.9	184.0	50.5	27.1	24.9	24.9	24.9	9.3	9.3	9.3	9.3	9.3
Approach LOS	D	F	D	C	C	C	C	A	A	A	A	A

Intersection Summary	
HCM Average Control Delay	48.3
HCM Volume to Capacity ratio	0.91
Actuated Cycle Length (s)	150.0
Intersection Capacity Utilization	89.9%
Analysis Period (min)	15
Critical Lane Group	

Baseline
A & R Engineering Inc.

Synchro 6 Report
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HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Base PM
5/18/2007

Movement	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations		↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.85	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	1583	1770	1583	1583
Flt Permitted	0.05	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	93	5085	1583	1770	1583	1770	1583	1583
Volume (vph)	2	48	1191	33	4	140	11	8
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.88	0.88	0.88	0.88
Adj. Flow (vph)	2	49	1215	34	5	159	12	9
RTOR Reduction (vph)	0	0	0	17	0	0	0	0
Lane Group Flow (vph)	0	51	1215	17	0	164	13	0
Turn Type	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm	Perm
Protected Phases	1	1	6	2	13	13	13	13
Permitted Phases	6	6	6	2	13	13	13	13
Actuated Green, G (s)	86.0	86.0	86.0	75.8	16.0	16.0	16.0	16.0
Effective Green, g (s)	86.0	86.0	86.0	75.8	16.0	16.0	16.0	16.0
Actuated g/C Ratio	0.57	0.57	0.57	0.51	0.11	0.11	0.11	0.11
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	123	2915	800	189	169	169	169	169
v/s Ratio Prot	0.02	0.24	0.01	0.01	0.09	0.01	0.01	0.01
v/s Ratio Perm	0.22	0.42	0.02	0.02	0.87	0.08	0.08	0.08
Uniform Delay, d1	23.1	17.9	18.6	66.0	60.3	60.3	60.3	60.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	2.3	0.4	0.0	38.1	0.9	0.9	0.9	0.9
Delay (s)	25.4	18.4	18.6	104.1	61.2	61.2	61.2	61.2
Level of Service	C	B	B	F	F	F	F	F
Approach Delay (s)	18.7	18.7	18.7	99.2	99.2	99.2	99.2	99.2
Approach LOS	B	B	B	F	F	F	F	F

Intersection Summary	
HCM Average Control Delay	48.3
HCM Volume to Capacity ratio	0.91
Actuated Cycle Length (s)	150.0
Intersection Capacity Utilization	89.9%
Analysis Period (min)	15
Critical Lane Group	

Baseline
A & R Engineering Inc.

Synchro 6 Report
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Lanes, Volumes, Timings
3: Highlands Pkwy & S Cobb Dr

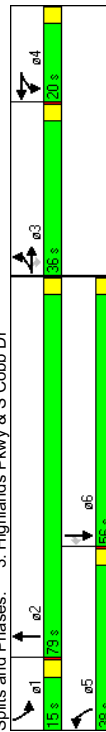
Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Ideal Flow (vphpl)	1681	1692	1583	0	1763	0	3433	5070	0	1770	5085	1583
Total Lost time (s)	0.950	0.956	0.981	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	1681	1692	1583	0	1763	0	3433	5070	0	1770	5085	1583
Satd. Flow (perm)	1681	1692	1583	0	1763	0	3433	5070	0	1770	5085	1583
Volume (vph)	634	20	497	51	41	29	484	1435	24	27	1061	525
Lane Group Flow (vph)	342	361	529	0	149	0	520	1508	0	35	1094	577
Turn Type	Split	Perm	Split	Split	Split	Split	Prot	Prot	Prot	Prot	Prot	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	6
Permitted Phases	36.0	36.0	36.0	20.0	20.0	20.0	0.0	38.0	79.0	0.0	15.0	56.0
Act Effct Green (s)	32.0	32.0	32.0	14.9	14.9	14.9	27.1	80.8	8.3	60.0	60.0	60.0
Actuated g/C Ratio	0.21	0.21	0.21	0.10	0.10	0.10	0.18	0.54	0.06	0.40	0.40	0.40
Control Delay	94.7	105.4	20.2	92.2	92.2	92.2	60.5	24.9	80.1	24.5	21.5	0.7
Queue Delay	0.0	0.0	0.0	0.2	0.2	0.2	0.0	0.4	0.0	0.3	0.7	0.7
Total Delay	94.7	105.4	20.2	92.4	92.4	92.4	60.5	25.3	80.1	24.8	22.2	0.7
LOS	F	F	C	F	F	F	E	C	F	C	C	C
Approach Delay	65.8			92.4				34.3			25.0	
Approach LOS	E			F				C			C	

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Cycle Length: 150
Actuated Cycle Length: 150
Offset: 115 (77%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.00
Intersection Signal Delay: 40.5
Intersection Capacity Utilization 69.1%
Analysis Period (min) 15

Splits and Phases: 3: Highlands Pkwy & S Cobb Dr



Baseline
A & R Engineering Inc.

Synchro 6 Report
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HCM Signalized Intersection Capacity Analysis
3: Highlands Pkwy & S Cobb Dr

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Ideal Flow (vphpl)	1681	1692	1583	0	1763	0	3433	5070	0	1770	5085	1583
Total Lost time (s)	0.950	0.956	0.981	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	1681	1692	1583	0	1763	0	3433	5070	0	1770	5085	1583
Satd. Flow (perm)	1681	1692	1583	0	1763	0	3433	5070	0	1770	5085	1583
Volume (vph)	634	20	497	51	41	29	484	1435	24	27	1061	525
Peak-hour factor, PHF	0.94	0.68	0.94	0.86	0.81	0.75	0.93	0.97	0.82	0.78	0.97	0.91
Adj. Flow (vph)	674	29	529	59	51	39	520	1479	29	35	1094	577
RTOR Reduction (vph)	0	0	337	0	9	0	0	1	0	0	0	108
Lane Group Flow (vph)	342	361	192	0	140	0	520	1507	0	35	1094	469
Turn Type	Split	Perm	Split	Split	Split	Split	Prot	Prot	Prot	Prot	Prot	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	6
Permitted Phases	32.0	32.0	32.0	14.9	14.9	14.9	27.1	79.9	7.2	60.0	60.0	60.0
Effective Green, g (s)	32.0	32.0	32.0	14.9	14.9	14.9	27.1	79.9	7.2	60.0	60.0	60.0
Actuated g/C Ratio	0.21	0.21	0.21	0.10	0.10	0.10	0.18	0.53	0.05	0.40	0.40	0.40
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	359	361	338	175	175	175	620	2701	85	2034	633	
v/s Ratio Prot	0.20	c0.21	0.12	c0.08	c0.08	c0.08	c0.15	0.30	0.02	0.22		
v/s Ratio Perm	0.95	1.00	0.57	0.80	0.80	0.80	0.84	0.56	0.41	0.54	0.74	
Uniform Delay, d1	58.3	59.0	52.8	66.1	66.1	66.1	59.3	23.3	69.3	34.4	38.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.89	1.03	1.07	0.67	0.54	
Incremental Delay, d2	35.1	47.4	2.2	22.0	22.0	22.0	5.9	0.5	2.4	0.8	5.8	
Delay (s)	93.3	106.4	55.0	88.1	88.1	88.1	58.9	24.5	76.8	23.8	26.4	
Level of Service	F	F	D	F	F	F	E	C	E	C	C	
Approach Delay (s)	80.7			88.1				33.3		25.7		
Approach LOS	F			F				C		C		

c Critical Lane Group

Baseline
A & R Engineering Inc.

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Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

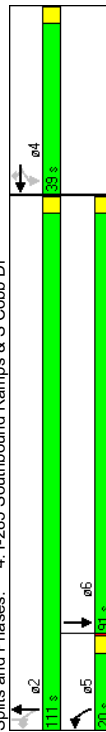
Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	0	0	0	1681	1441	1504	1770	3539	0	0	3539	1583
Flt Permitted				0.950			0.189					
Satd. Flow (perm)	0	0	0	1681	1441	1504	352	3539	0	0	3539	1583
Satd. Flow (RTOR)	0	0	0	436	0	92	92	1288	0	0	998	774
Volume (vph)	0	0	0	436	0	92	92	1288	0	0	998	774
Satd. Flow (prot)	0	0	0	458	545	545	85	1328	0	0	1051	832
Lane Group Flow (vph)	0	0	0	458	545	545	85	1328	0	0	1051	832
Turn Type				Perm	Perm	pm+pt					custom	
Protected Phases				4	4	4	5	2			6	
Permitted Phases				4	4	4	2				6	
Total Split (s)	0.0	0.0	0.0	39.0	39.0	20.0	111.0	0.0	0.0	91.0	111.0	
Act Effct Green (s)				35.0	35.0	35.0	107.0	107.0		97.5	107.0	
Actuated g/C Ratio				0.23	0.23	0.23	0.71	0.71		0.65	0.71	
v/c Ratio				1.17	1.34	1.29	0.28	0.53		0.46	0.61	
Control Delay				149.1	204.5	186.5	1.4	1.6		6.1	5.7	
Queue Delay				14.4	15.0	0.0	0.0	0.5		0.0	0.0	
Total Delay				163.5	219.5	186.5	1.4	2.1		6.1	5.7	
LOS				F	F	F	A	A		A	A	
Approach Delay				191.3			2.1			5.9		
Approach LOS				F			A			A		

A-32

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 122 (81%), Referenced to phase 2:NBT and 6:SBT, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.34												
Intersection Signal Delay: 64.0												
Intersection Capacity Utilization 136.8%												
Analysis Period (min) 15												

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



Baseline
A & R Engineering Inc.

Synchro 6 Report
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HCM Signalized Intersection Capacity Analysis
4: I-285 Southbound Ramps & S Cobb Dr

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.91	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.85	1.00
Flt Protected				0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1681	1441	1504	1770	3539					3539	1583	
Flt Permitted	0.95	1.00	1.00	0.21	1.00					1.00	1.00	
Satd. Flow (perm)	1681	1441	1504	391	3539					3539	1583	
Volume (vph)	0	0	0	436	0	980	71	1288	0	0	998	774
Peak-hour factor, PHF	0.92	0.92	0.92	0.95	0.92	0.90	0.84	0.97	0.92	0.92	0.95	0.93
Adj. Flow (vph)	0	0	0	459	0	1089	85	1328	0	0	1051	832
RTOR Reduction (vph)	0	0	0	0	71	71	0	0	0	0	0	239
Lane Group Flow (vph)	0	0	0	458	474	474	85	1328	0	0	1051	593
Turn Type				Perm	Perm	pm+pt					custom	
Protected Phases				4	4	5	2				6	
Permitted Phases				4	4	4	2				6	
Actuated Green, G (s)				35.0	35.0	35.0	107.0	107.0		97.5	107.0	
Effective Green, g (s)				35.0	35.0	35.0	107.0	107.0		97.5	107.0	
Actuated g/C Ratio				0.23	0.23	0.23	0.71	0.71		0.65	0.71	
Clearance Time (s)				4.0	4.0	4.0	4.0	4.0		4.0	4.0	
Vehicle Extension (s)				3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)				392	336	351	329	2524		2300	1129	
v/s Ratio Prot				0.27	0.33	0.32	0.17			0.30		
v/s Ratio Perm				1.17	1.41	1.35	0.26	0.53		0.46	0.53	
Uniform Delay, d1				57.5	57.5	57.5	8.6	9.9		13.1	9.9	
Progression Factor				1.00	1.00	1.00	0.13	0.14		0.42	8.27	
Incremental Delay, d2				99.9	202.3	176.0	0.1	0.2		0.6	1.5	
Delay (s)				157.4	259.8	233.5	1.2	1.6		6.0	83.0	
Level of Service				F	F	F	A	A		A	F	
Approach Delay (s)				0.0		220.2		1.6		40.1		
Approach LOS				A		F		A		D		
Intersection Summary												
HCM Average Control Delay				86.4						F		
HCM Volume to Capacity ratio				0.74								
Actuated Cycle Length (s)				150.0						8.0		
Intersection Capacity Utilization				136.8%						H		
Analysis Period (min)				15								
c Critical Lane Group												

Baseline
A & R Engineering Inc.

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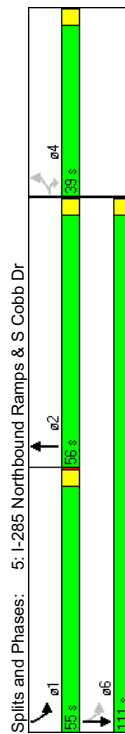
Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3330	0	1770	3539	0	0
Flt Permitted	0.950						0.147					
Satd. Flow (perm)	3433	0	1583	0	0	0	3330	0	274	3539	0	0
Satd. Flow (RTOR)			87				113					
Volume (vph)	910	0	80	0	0	0	0	469	281	744	658	0
Lane Group Flow (vph)	929	0	94	0	0	0	0	835	0	791	708	0
Turn Type	custom	custom	custom							pm+pt		
Protected Phases	4		4				2			1		6
Permitted Phases												
Total Split (s)	39.0	0.0	39.0	0.0	0.0	0.0	56.0	0.0	55.0	111.0	0.0	0.0
Act Effct Green (s)	35.0		35.0				52.0		107.0	107.0		
Actuated g/C Ratio	0.23		0.23				0.35		0.71	0.71		
v/c Ratio	1.16		0.22				0.68		1.12	0.28		
Control Delay	135.3		11.5				39.1		104.6	8.6		
Queue Delay	0.0		0.0				0.0		104.2	0.4		
Total Delay	135.3		11.5				39.1		208.8	9.1		
LOS	F		B				D		F	A		
Approach Delay							39.1			114.5		F
Approach LOS							D					

A-33

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 101 (67%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.16												
Intersection Signal Delay: 98.6												
Intersection Capacity Utilization 136.8%												
Analysis Period (min) 15												



Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.85	1.00	0.95	1.00	0.94	1.00	0.95	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	3433	1583	1583	1583	1583	1583	3329	1770	3539	1770	3539	1770
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.15	1.00	1.00	1.00
Satd. Flow (perm)	3433	1583	1583	1583	1583	1583	3329	273	3539	273	3539	273
Volume (vph)	910	0	80	0	0	0	0	469	281	744	658	0
Peak-hour factor, PHF	0.98	0.92	0.85	0.92	0.92	0.92	0.93	0.85	0.94	0.93	0.92	0.92
Adj. Flow (vph)	929	0	94	0	0	0	0	504	331	791	708	0
RTOR Reduction (vph)	0	0	67	0	0	0	0	74	0	0	0	0
Lane Group Flow (vph)	929	0	27	0	0	0	0	761	0	791	708	0
Turn Type	custom	custom	custom							pm+pt		
Protected Phases	4		4				2			1		6
Permitted Phases												
Actuated Green, G (s)	35.0		35.0				52.0		107.0	107.0		
Effective Green, g (s)	35.0		35.0				52.0		107.0	107.0		
Actuated g/C Ratio	0.23		0.23				0.35		0.71	0.71		
Clearance Time (s)	4.0		4.0				4.0		4.0	4.0		
Vehicle Extension (s)	3.0		3.0				3.0		3.0	3.0		
Lane Grp Cap (vph)	801		369				1154		704	2524		
v/s Ratio Prot							0.23		0.38	0.20		
v/s Ratio Perm	0.27		0.02				0.66		1.12	0.28		
v/c Ratio	1.16		0.07				0.66		1.12	0.28		
Uniform Delay, d1	57.5		44.9				41.5		37.8	7.7		
Progression Factor	1.00		1.00				1.00		1.06	1.09		
Incremental Delay, d2	85.6		0.1				3.0		69.3	0.2		
Delay (s)	143.1		44.9				44.5		109.3	8.6		
Level of Service	F		D				D		F	A		
Approach Delay (s)		134.1			0.0		44.5			61.8		
Approach LOS		F			A		D			E		
Intersection Summary												
HCM Average Control Delay												
HCM Volume to Capacity ratio												
Actuated Cycle Length (s)												
Intersection Capacity Utilization												
Analysis Period (min)												
Critical Lane Group												

Baseline
A & R Engineering Inc.

Synchro 6 Report
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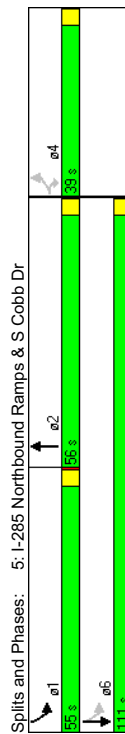
Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3330	0	1770	3539	0	0
Flt Permitted	0.950						0.147					
Satd. Flow (perm)	3433	0	1583	0	0	0	3330	0	274	3539	0	0
Satd. Flow (RTOR)			87				113					
Volume (vph)	910	0	80	0	0	0	0	469	281	744	658	0
Lane Group Flow (vph)	929	0	94	0	0	0	0	835	0	791	708	0
Turn Type	custom	custom	custom							pm+pt		
Protected Phases	4		4				2			1		6
Permitted Phases												
Total Split (s)	39.0	0.0	39.0	0.0	0.0	0.0	56.0	0.0	55.0	111.0	0.0	0.0
Act Effct Green (s)	35.0		35.0				52.0		107.0	107.0		
Actuated g/C Ratio	0.23		0.23				0.35		0.71	0.71		
v/c Ratio	1.16		0.22				0.68		1.12	0.28		
Control Delay	135.3		11.5				39.1		104.6	8.6		
Queue Delay	0.0		0.0				0.0		104.2	0.4		
Total Delay	135.3		11.5				39.1		208.8	9.1		
LOS	F		B				D		F	A		
Approach Delay							39.1			114.5		F
Approach LOS							D					

A-33

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 101 (67%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.16												
Intersection Signal Delay: 98.6												
Intersection Capacity Utilization 136.8%												
Analysis Period (min) 15												



Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr

Baseline
A & R Engineering Inc.

Synchro 6 Report
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Lanes, Volumes, Timings
6: Church Rd & N Church Lane

Base PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1820	0	0	1837	0	0	1767	0	0	1679	0
Flt Permitted	0	0.980	0	0	0.983	0	0	0.981	0	0	0.981	0
Satd. Flow (perm)	0	1820	0	0	1837	0	0	1767	0	0	1679	0
Volume (vph)	68	106	1	0	351	36	8	10	6	110	2	156
Lane Group Flow (vph)	0	199	0	0	419	0	0	46	0	0	314	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	62.5%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
6: Church Rd & N Church Lane

Base PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	68	106	1	0	351	36	8	10	6	110	2	156
Peak Hour Factor	0.84	0.93	0.25	0.92	0.93	0.85	0.50	0.56	0.50	0.90	0.50	0.83
Hourly flow rate (vph)	81	114	4	0	377	42	16	18	12	122	4	188
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	377			118			866	655	116	697	678	399
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	377			118			866	655	116	697	678	399
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	93			100			91	95	99	62	99	71
cM capacity (veh/h)	1181			1470			183	359	936	320	348	651
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	199	420	46	314								
Volume Left	81	0	16	122								
Volume Right	4	42	12	188								
cSH	1181	1470	305	461								
Volume to Capacity	0.07	0.00	0.15	0.68								
Queue Length 95th (ft)	6	0	13	126								
Control Delay (s)	3.7	0.0	18.9	28.0								
Lane LOS	A	C	C	D								
Approach Delay (s)	3.7	0.0	18.9	28.0								
Approach LOS	C	D	C	D								
Intersection Summary												
Average Delay	10.6											
Intersection Capacity Utilization	62.5%											
Analysis Period (min)	15											

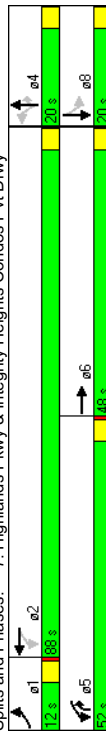
Lanes, Volumes, Timings
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3412	0	1770	3511	0	1770	1863	1583	0	1789	0
Flt Permitted	0.950	0.083		0.083			0.714				0.873	
Satd. Flow (perm)	1770	3412	0	155	3511	0	1330	1863	1583	0	1599	0
Satd. Flow (RTOR)	40	11		11			49				5	
Volume (vph)	5	778	236	637	436	14	82	2	374	21	17	3
Lane Group Flow (vph)	8	1117	0	671	485	0	111	4	386	0	63	0
Turn Type	Prot	pm+pt		Perm	pm+ov	Perm						
Protected Phases	1	6	5	2			4	5	8			
Permitted Phases												
Total Split (s)	12.0	48.0	0.0	52.0	88.0	0.0	20.0	20.0	52.0	20.0	20.0	0.0
Act Effct Green (s)	6.2	45.8	91.3	89.2			13.1	13.1	58.6	13.1		
Actuated g/C Ratio	0.05	0.41	0.81	0.79			0.12	0.12	0.52	0.12		
v/c Ratio	0.09	0.79	0.93	0.17			0.72	0.02	0.46	0.33		
Control Delay	58.2	35.0	47.9	3.5			73.8	45.5	15.7	48.3		
Queue Delay	0.0	0.0	0.0	0.0			0.0	0.0	0.0	0.0		
Total Delay	58.2	35.0	47.9	3.5			73.8	45.5	15.7	48.3		
LOS	E	D	D	A			E	D	B	D		
Approach Delay												
Approach LOS												

Cycle Length: 120
Actuated Cycle Length: 112.5
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.93
Intersection Signal Delay: 31.9
Intersection Capacity Utilization 83.2%
Analysis Period (min) 15

Splits and Phases: 7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy



HCM Signalized Intersection Capacity Analysis
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.96	1.00	0.95	1.00	0.99	1.00	1.00	1.00	0.85	0.98	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.98		
Satd. Flow (prot)	1770	3410	1770	3511	1770	3511	1770	1863	1583	1788		
Flt Permitted	0.95	1.00	0.10	1.00	0.71	1.00	0.71	1.00	1.00	0.87		
Satd. Flow (perm)	1770	3410	186	3511	1330	1863	1583					
Volume (vph)	5	778	236	637	436	14	82	2	374	21	17	3
Peak-hour factor, PHF	0.62	0.92	0.87	0.95	0.95	0.54	0.74	0.50	0.97	0.71	0.67	0.38
Adj. Flow (vph)	8	846	271	671	459	26	111	4	386	30	25	8
RTOR Reduction (vph)	0	23	0	0	3	0	0	0	26	0	4	0
Lane Group Flow (vph)	8	1094	0	671	482	0	111	4	386	0	59	0
Turn Type	Prot	pm+pt		Perm	pm+ov	Perm						
Protected Phases	1	6	5	2			4	5	8			
Permitted Phases												
Actuated Green, G (s)	1.4	49.1	94.6	89.2			13.1	13.1	54.6	13.1		
Effective Green, g (s)	1.4	49.1	94.6	89.2			13.1	13.1	54.6	13.1		
Actuated g/C Ratio	0.01	0.42	0.82	0.77			0.11	0.11	0.47	0.11		
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	21	1447	720	2707	151	211	802					
v/s Ratio Prot	0.00	0.32	c0.33	0.14			c0.08	0.07	0.04			
v/s Ratio Perm			c0.43									
v/c Ratio	0.38	0.76	0.93	0.18			0.74	0.02	0.45	0.33		
Uniform Delay, d1	56.7	28.2	29.1	3.5			49.6	45.6	20.5	47.2		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	11.2	2.3	18.8	0.1			16.9	0.0	0.4	1.1		
Delay (s)	67.9	30.5	47.9	3.7			66.5	45.6	20.9	48.3		
Level of Service	E	C	D	A			E	D	C	D		
Approach Delay (s)												
Approach LOS												

Intersection Summary
HCM Average Control Delay
HCM Volume to Capacity ratio
Actuated Cycle Length (s)
Intersection Capacity Utilization
Analysis Period (min)
c Critical Lane Group

Lanes, Volumes, Timings
8: US 278 (US 78) & Oakdale Rd

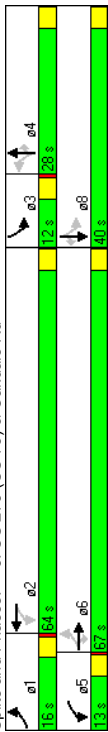
Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3500	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.063			0.428			0.416			0.465		
Satd. Flow (perm)	117	3539	1583	797	3500	0	775	1863	1583	866	1863	1583
Satd. Flow (RTOR)		80		10			119			119		162
Volume (vph)	141	489	72	220	1319	103	80	133	100	103	339	448
Lane Group Flow (vph)	153	532	80	239	1548	0	94	145	119	123	368	477
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	Perm
Protected Phases	1	6	6	5	2		4	4		3	8	
Permitted Phases	6			2			4			8		
Total Split (s)	16.0	67.0	13.0	64.0	0.0	28.0	28.0	28.0	12.0	40.0	40.0	
Act Effct Green (s)	73.1	63.2	63.2	70.9	62.1	18.0	18.0	18.0	29.8	29.8	29.8	
Actuated g/C Ratio	0.64	0.55	0.55	0.62	0.55	0.16	0.16	0.16	0.26	0.26	0.26	
Control Delay	39.9	14.5	3.3	11.0	26.8	82.6	49.2	10.0	37.6	48.9	46.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	39.9	14.5	3.3	11.0	26.8	82.6	49.2	10.0	37.6	48.9	46.4	
LOS	D	B	A	B	C	F	D	B	D	D	D	
Approach Delay		18.4		24.7			44.9			46.3		
Approach LOS		B		C			D			D		

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Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 113.9	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.89	
Intersection Signal Delay: 30.7	Intersection LOS: C
Intersection Capacity Utilization 83.2%	ICU Level of Service E
Analysis Period (min) 15	

Splits and Phases: 8: US 278 (US 78) & Oakdale Rd



Baseline
A & R Engineering Inc.

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HCM Signalized Intersection Capacity Analysis
8: US 278 (US 78) & Oakdale Rd

Base PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.99	1.00	1.00	0.85	1.00	1.00	0.85
Satd. Flow (prot)	1770	3539	1583	1770	3500		1770	1863	1583	1770	1863	1583
Flt Permitted	0.06	1.00	1.00	0.42	1.00		0.37	1.00	1.00	0.41	1.00	1.00
Satd. Flow (perm)	118	3539	1583	788	3500		691	1863	1583	765	1863	1583
Volume (vph)	141	489	72	220	1319	103	80	133	100	103	339	448
Peak-hour factor, PHF	0.92	0.92	0.90	0.92	0.92	0.90	0.85	0.92	0.84	0.84	0.92	0.94
Adj. Flow (vph)	153	532	80	239	1434	114	94	145	119	123	368	477
RTOR Reduction (vph)	0	0	36	0	5	0	0	0	0	100	0	119
Lane Group Flow (vph)	153	532	44	239	1543	0	94	145	19	123	368	358
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	Perm
Protected Phases	1	6	6	5	2		4	4	3	8		
Permitted Phases	6			2			4		4	8		
Actuated Green, G (s)	73.1	63.2	63.2	70.9	62.1	18.0	18.0	18.0	29.9	29.9	29.9	
Effective Green, g (s)	73.1	63.2	63.2	70.9	62.1	18.0	18.0	18.0	29.9	29.9	29.9	
Actuated g/C Ratio	0.64	0.55	0.55	0.62	0.55	0.16	0.16	0.16	0.26	0.26	0.26	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	219	1964	878	566	1908	109	294	250	271	489	416	
v/s Ratio Prot	c0.06	0.15		0.03	c0.44		0.08		0.03	0.20		
v/s Ratio Perm	0.39		0.03	0.23		0.14		0.01	0.09		c0.23	
v/c Ratio	0.70	0.27	0.05	0.42	0.81	0.86	0.49	0.08	0.45	0.75	0.86	
Uniform Delay, d1	26.3	13.3	11.6	9.5	21.1	46.7	43.8	40.9	33.6	38.6	40.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	9.3	0.3	0.1	0.5	3.8	46.1	1.3	0.1	1.2	6.5	16.0	
Delay (s)	35.7	13.6	11.7	10.0	24.9	92.8	45.1	41.0	34.8	45.1	56.0	
Level of Service	D	B	B	B	C	F	D	D	C	D	E	
Approach Delay (s)		17.8		22.9			56.3			49.2		
Approach LOS		B		C			E			D		
Intersection Summary												
HCM Average Control Delay	31.5 HCM Level of Service C											
HCM Volume to Capacity ratio	0.81											
Actuated Cycle Length (s)	113.9 Sum of lost time (s)											
Intersection Capacity Utilization	83.2% ICU Level of Service E											
Analysis Period (min)	15											
Critical Lane Group												

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	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔				↔	↔	↔	↔		↔	↔
Satd. Flow (prot)	0	1617	0	0	0	1611	1770	5085	1583	0	5060	0
Flt Permitted		0.998					0.950					
Satd. Flow (perm)	0	1617	0	0	0	1611	1770	5085	1583	0	5060	0
Volume (vph)	8	0	260	0	0	92	437	1789	121	0	1548	46
Lane Group Flow (vph)	0	298	0	0	0	110	465	1826	136	0	1684	0
Sign Control		Stop			Yield			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 81.7%	ICU Level of Service D											
Analysis Period (min) 15												

Intersection has too many lanes per leg.
HCM All-Way analysis is limited to two lanes per leg.
Channelized right turn lanes are not counted.

Lanes, Volumes, Timings
10: Church Rd & Groover Dr

Base PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1814	0	0	1859	0	0	1731	0	0	1611	0
Flt Permitted					0.998			0.965				
Satd. Flow (perm)	0	1814	0	0	1859	0	0	1731	0	0	1611	0
Volume (vph)	0	144	21	15	543	0	15	0	7	0	0	3
Lane Group Flow (vph)	0	220	0	0	593	0	0	29	0	0	8	0
Sign Control	Free	Free		Free	Free			Stop			Stop	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	55.2%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
10: Church Rd & Groover Dr

Base PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free		Free	Free			Stop			Stop	
Grade					0%			0%				
Volume (veh/h)	0	144	21	15	543	0	15	0	7	0	0	3
Peak Hour Factor	0.92	0.81	0.50	0.70	0.95	0.92	0.70	0.92	0.88	0.92	0.92	0.38
Hourly flow rate (vph)	0	178	42	21	572	0	21	0	8	0	0	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				None
Median storage (veh)												
Upstream signal (ft)								1244				
pX, platoon unblocked												
VC, conflicting volume	572			220			821	813	199	821	834	572
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
vCu, unblocked vol	572			220			821	813	199	821	834	572
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			92	100	99	100	100	98
cM capacity (veh/h)	1001			1350			285	308	842	287	299	520
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	220	593	29	8								
Volume Left	0	21	21	0								
Volume Right	42	0	8	8								
cSH	1001	1350	348	520								
Volume to Capacity	0.00	0.02	0.08	0.02								
Queue Length 95th (ft)	0	1	7	1								
Control Delay (s)	0.0	0.5	16.3	12.0								
Lane LOS	A	C	B	B								
Approach Delay (s)	0.0	0.5	16.3	12.0								
Approach LOS	C	C	B	B								
Intersection Summary												
Average Delay	1.0											
Intersection Capacity Utilization	55.2%											
Analysis Period (min)	15											

Base 2009 PM Improved

Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

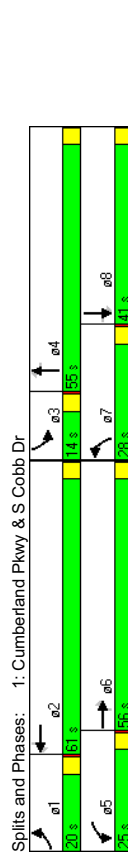
HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Base PM - Improved
5/18/2007

Base PM - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	144			144			46			44		
Volume (vph)	177	907	130	137	2033	129	569	1217	99	207	895	98
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	177	907	130	137	2033	129	569	1217	99	207	895	98
Peak-hour factor, PHF	0.87	0.96	0.85	0.90	0.95	0.90	0.92	0.90	0.82	0.89	0.97	0.79
Adj. Flow (vph)	203	945	153	152	2140	143	618	1352	121	233	923	124
RTOR Reduction (vph)	0	0	108	0	0	30	0	0	28	0	0	55
Lane Group Flow (vph)	203	945	45	152	2140	113	618	1352	93	233	923	69
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases												
Actuated Green, G (s)	16.0	55.8	37.0	17.2	57.0	51.0	24.0	51.0	55.8	10.0	37.0	57.0
Effective Green, g (s)	16.0	55.8	37.0	17.2	57.0	51.0	24.0	51.0	55.8	10.0	37.0	57.0
Actuated g/C Ratio	0.11	0.37	0.25	0.11	0.38	0.34	0.16	0.34	0.37	0.07	0.25	0.38
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	189	1892	390	203	1932	538	549	1203	589	229	873	602
v/s Ratio Prot	c0.11	0.19	0.03	0.09	c0.42	c0.18	c0.38		0.07	0.06	0.26	
v/s Ratio Perm	1.07	0.50	0.11	0.75	1.11	0.21	1.13	1.12	0.16	1.02	1.06	0.11
Uniform Delay, d1	67.0	36.3	43.8	64.3	46.5	35.2	63.0	49.5	31.4	70.0	56.5	30.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	86.5	0.9	0.1	14.0	56.6	0.2	77.9	67.0	0.6	64.0	46.8	0.4
Delay (s)	153.5	37.3	43.9	78.3	103.1	35.4	140.9	116.5	32.0	134.0	103.3	30.5
Level of Service	F	D	D	E	F	D	F	D	F	C	F	C
Approach Delay (s)	56.2			97.6			118.8			101.8		
Approach LOS	E			F			F			F		

Intersection Summary												
Cycle Length: 150												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 1.13												
Intersection Signal Delay: 92.6	Intersection LOS: F											
Intersection Capacity Utilization 103.4%	ICU Level of Service G											
Analysis Period (min) 15												



Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

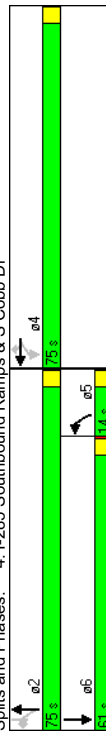
Base PM - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	0	0	0	1681	1441	1504	1770	3539	0	0	3539	1583
Flt Permitted	0	0	0	0.950			0.106					
Satd. Flow (perm)	0	0	0	1681	1441	1504	197	3539	0	0	3539	1583
Satd. Flow (RTOR)	0	0	0	24	24	24			0	0		
Volume (vph)	0	0	0	436	0	980	71	1288	0	0	998	774
Satd. Flow (prot)	0	0	0	459	544	545	85	1328	0	0	1051	832
Lane Group Flow (vph)	0	0	0	459	544	545	85	1328	0	0	1051	832
Turn Type				Perm	Perm	pm+pt					custom	
Protected Phases				4	4	5	2	2			6	
Permitted Phases				4	4	2						
Total Split (s)	0.0	0.0	0.0	75.0	75.0	14.0	75.0	0.0	0.0	0.0	61.0	75.0
Act Effct Green (s)				59.8	59.8	82.2	82.2				68.2	82.2
Actuated g/C Ratio				0.40	0.40	0.40	0.55	0.55			0.45	0.55
Control Delay				0.68	0.92	0.89	0.40	0.69			0.65	0.69
Queue Delay				41.9	62.4	56.3	46.9	37.8			19.6	7.4
LOS				D	E	E	D	D			B	A
Approach Delay				54.3			39.3				14.2	
Approach LOS				D			D				B	

A-41

Intersection Summary	
Cycle Length: 150	
Actuated Cycle Length: 150	
Offset: 125 (83%), Referenced to phase 2:NBTL and 6:SBT, Start of Green	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.92	
Intersection Signal Delay: 34.4	Intersection LOS: C
Intersection Capacity Utilization 132.2%	ICU Level of Service H
Analysis Period (min) 15	

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
4: I-285 Southbound Ramps & S Cobb Dr

Base PM - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)				4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor				0.95	0.91	0.95	1.00	0.95		0.95	1.00	1.00
Flt Protected				0.95	1.00	0.85	1.00	1.00	1.00	1.00	1.00	0.85
Satd. Flow (prot)	1681	1681	1681	1441	1441	1504	1770	3539		3539	1583	1583
Flt Permitted				0.95	1.00	1.00	0.15	1.00		1.00	1.00	1.00
Satd. Flow (perm)	1681	1681	1681	1441	1441	1504	280	3539		3539	1583	1583
Volume (vph)	0	0	0	436	0	980	71	1288	0	0	998	774
Peak-hour factor, PHF	0.92	0.92	0.92	0.95	0.92	0.90	0.84	0.97	0.92	0.92	0.95	0.93
Adj. Flow (vph)	0	0	0	459	0	1089	85	1328	0	0	1051	832
RTOR Reduction (vph)	0	0	0	0	0	14	14	0	0	0	0	341
Lane Group Flow (vph)	0	0	0	459	530	531	85	1328	0	0	1051	491
Turn Type				Perm	Perm	pm+pt					custom	
Protected Phases				4	4	5	2	2			6	
Permitted Phases				4	4	2						2
Actuated Green, G (s)				59.8	59.8	59.8	82.2	82.2			68.2	82.2
Effective Green, g (s)				59.8	59.8	59.8	82.2	82.2			68.2	82.2
Actuated g/C Ratio				0.40	0.40	0.40	0.55	0.55			0.45	0.55
Clearance Time (s)				4.0	4.0	4.0	4.0	4.0			4.0	4.0
Vehicle Extension (s)				3.0	3.0	3.0	3.0	3.0			3.0	3.0
Lane Grp Cap (vph)				670	574	600	253	1939			1609	867
v/s Ratio Prot				0.27	0.37	0.35	0.16	c0.38			0.30	
v/s Ratio Perm				0.69	0.92	0.88	0.34	0.68			0.65	0.57
Uniform Delay, d1				37.3	42.9	41.9	40.1	24.5			31.7	22.2
Progression Factor				1.00	1.00	1.00	1.24	1.39			0.52	1.47
Incremental Delay, d2				2.9	20.5	14.5	0.3	0.8			1.8	2.3
Delay (s)				40.2	63.4	56.4	49.9	34.8			18.4	35.0
Level of Service				D	E	E	D	C			B	D
Approach Delay (s)				0.0		54.1		35.7			25.8	
Approach LOS				A		D		D			C	
Intersection Summary												
HCM Average Control Delay				37.7							D	
HCM Volume to Capacity ratio				0.78								
Actuated Cycle Length (s)				150.0							8.0	
Intersection Capacity Utilization				132.2%							H	
Analysis Period (min)				15								
Critical Lane Group												

Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

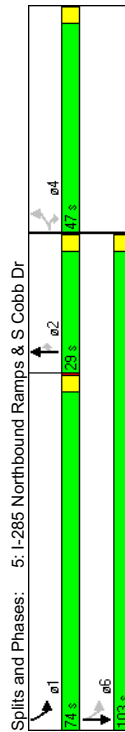
Base PM - Improved

5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3539	1583	1770	3539	0	0
Flt Permitted	0.950								0.138			
Satd. Flow (perm)	3433	0	1583	0	0	0	3539	1583	257	3539	0	0
Satd. Flow (RTOR)			94						252			
Volume (vph)	910	0	80	0	0	0	0	469	281	744	658	0
Lane Group Flow (vph)	929	0	94	0	0	0	0	504	331	791	708	0
Turn Type	custom	custom	custom						Perm	pm+pt		
Protected Phases	4		4					2		1	6	
Permitted Phases									2		6	
Total Split (s)	47.0	0.0	47.0	0.0	0.0	0.0	0.0	29.0	29.0	74.0	103.0	0.0
Act Effct Green (s)	42.3		42.3					25.0	25.0	99.7	99.7	
Actuated g/C Ratio	0.28		0.28					0.17	0.17	0.66	0.66	
Control Delay	73.5		8.0					75.4	23.4	41.3	5.8	
Queue Delay	1.0		0.0					0.0	0.0	15.4	0.3	
Total Delay	74.5		8.0					75.4	23.4	56.7	6.1	
LOS	E		A					E	C	E	A	
Approach Delay								54.8			32.8	
Approach LOS								D			C	

A-42

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 3 (2%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.96												
Intersection Signal Delay: 49.1												
Intersection Capacity Utilization 132.2%												
Analysis Period (min) 15												



Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3539	1583	1770	3539	0	0
Flt Permitted	0.950								0.138			
Satd. Flow (perm)	3433	0	1583	0	0	0	3539	1583	257	3539	0	0
Satd. Flow (RTOR)			94						252			
Volume (vph)	910	0	80	0	0	0	0	469	281	744	658	0
Lane Group Flow (vph)	929	0	94	0	0	0	0	504	331	791	708	0
Turn Type	custom	custom	custom						Perm	pm+pt		
Protected Phases	4		4					2		1	6	
Permitted Phases									2		6	
Actuated Green, G (s)	42.3		42.3					25.0	25.0	99.7	99.7	
Effective Green, g (s)	42.3		42.3					25.0	25.0	99.7	99.7	
Actuated g/C Ratio	0.28		0.28					0.17	0.17	0.66	0.66	
Clearance Time (s)	4.0		4.0					4.0	4.0	4.0	4.0	
Vehicle Extension (s)	3.0		3.0					3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	968		446					590	264	884	2352	
v/s Ratio Prot								0.14		0.42	0.20	
v/s Ratio Perm	c0.27		0.02					0.85	0.46	0.89	0.30	
v/c Ratio	0.96		0.06					0.85	0.46	0.89	0.30	
Uniform Delay, d1	53.0		39.3					60.7	56.4	30.7	10.5	
Progression Factor	1.00		1.00					1.00	1.00	1.00	0.52	
Incremental Delay, d2	19.6		0.1					14.6	5.6	8.9	0.2	
Delay (s)	72.6		39.4					75.4	62.0	39.6	5.7	
Level of Service	E		D					E	E	D	A	
Approach Delay (s)			69.5					70.1			23.6	
Approach LOS			E					E			C	
Intersection Summary												
HCM Average Control Delay	49.1											
HCM Volume to Capacity ratio	0.91											
Actuated Cycle Length (s)	150.0											
Intersection Capacity Utilization	132.2%											
Analysis Period (min)	15											
c Critical Lane Group												

Base 2009 SAT

Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

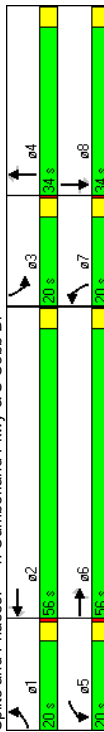
Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	232			75			78			78		
Volume (vph)	136	843	254	124	854	149	358	948	115	249	926	125
Lane Group Flow (vph)	162	878	292	144	1005	157	385	1009	128	277	975	149
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	7	4	3	8			
Permitted Phases												
Total Split (s)	20.0	56.0	34.0	20.0	56.0	34.0	20.0	56.0	20.0	34.0	56.0	20.0
Act Effct Green (s)	14.8	52.6	30.1	14.2	52.0	31.4	15.9	31.4	52.6	14.6	30.1	52.0
Actuated g/C Ratio	0.11	0.41	0.23	0.11	0.40	0.24	0.12	0.24	0.41	0.11	0.23	0.40
v/c Ratio	0.79	0.61	0.53	0.74	0.70	0.36	0.91	1.17	0.18	0.71	1.18	0.21
Control Delay	82.4	32.5	14.1	77.6	35.4	24.1	81.2	131.5	11.3	65.9	137.4	7.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	82.4	32.5	14.1	77.6	35.4	24.1	81.2	131.5	11.3	65.9	137.4	7.4
LOS	F	C	B	E	D	C	F	F	B	E	F	A
Approach Delay		34.5			38.7			108.7			109.4	
Approach LOS		C			D			F			F	

A-44

Intersection Summary												
Cycle Length: 130												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 1.18												
Intersection Signal Delay: 74.7												
Intersection Capacity Utilization 80.3%												
Analysis Period (min) 15												

Splits and Phases: 1: Cumberland Pkwy & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	136	843	254	124	854	149	358	948	115	249	926	125
Peak-hour factor, PHF	0.84	0.96	0.87	0.86	0.85	0.95	0.93	0.94	0.90	0.90	0.95	0.84
Adj. Flow (vph)	162	878	292	144	1005	157	385	1009	128	277	975	149
RTOR Reduction (vph)	0	0	178	0	0	57	0	0	46	0	0	72
Lane Group Flow (vph)	162	878	114	144	1005	100	385	1009	82	277	975	77
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	7	4	3	8			
Permitted Phases												
Actuated Green, G (s)	14.8	52.6	30.1	14.2	52.0	31.4	15.9	31.4	52.6	14.6	30.1	52.0
Effective Green, g (s)	14.8	52.6	30.1	14.2	52.0	31.4	15.9	31.4	52.6	14.6	30.1	52.0
Actuated g/C Ratio	0.11	0.41	0.23	0.11	0.40	0.24	0.12	0.24	0.41	0.11	0.23	0.40
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	203	1445	370	195	1429	386	424	863	646	389	827	639
v/s Ratio Prot	c0.09	0.25	0.07	0.08	c0.28	0.06	c0.11	c0.29	0.08	0.08	0.28	0.05
v/s Ratio Perm	0.80	0.61	0.31	0.74	0.70	0.26	0.91	1.17	0.13	0.71	1.18	0.12
Uniform Delay, d1	55.5	30.0	40.8	55.5	32.0	39.3	55.7	48.7	23.8	55.1	49.4	24.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	19.3	1.9	0.5	13.6	2.9	0.4	22.7	88.5	0.4	6.1	93.0	0.4
Delay (s)	74.8	31.9	41.2	69.1	34.9	39.7	78.5	137.2	24.2	61.1	142.3	24.5
Level of Service	E	C	D	E	C	D	E	F	C	E	F	C
Approach Delay (s)		39.2			39.2			112.9			113.7	
Approach LOS		D			D			F			F	

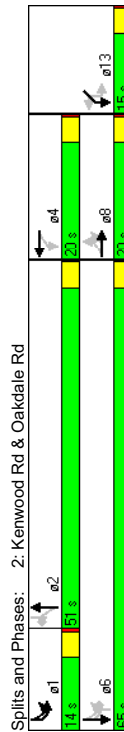
Intersection Summary												
HCM Average Control Delay												
HCM Volume to Capacity ratio												
Actuated Cycle Length (s)												
Intersection Capacity Utilization												
Analysis Period (min)												
Critical Lane Group												

Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Base SAT
5/18/2007

Lane Group	EBL2	EBL	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	1707	0	1770	1632	0	0	1770	5075	0	1583
Satd. Flow (prot)	0	0	0.830	0	0.551	0.156	0	0	0.156	0	0	0
Fit Permitted	0	0	1447	0	1026	1632	0	0	291	5075	0	1583
Satd. Flow (perm)	54	6	14	66	94	12	51	5	91	1305	18	64
Satd. Flow (RTOR)	0	0	161	0	106	76	0	0	94	1364	0	66
Volume (vph)	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Lane Group Flow (vph)	8	8	8	8	4	4	4	4	2	2	2	6
Turn Type	Protected Phases	Permitted Phases	Total Split (s)	Act Effct Green (s)	Actuated g/C Ratio	Control Delay	Queue Delay	Total Delay	LOS	Approach Delay	Approach LOS	Intersection Summary
Protected Phases	8	8	20.0	20.0	0.0	20.0	20.0	0.0	0.0	51.0	51.0	0.0
Permitted Phases	20.0	20.0	13.8	13.8	0.14	0.14	0.14	0.14	0.52	0.52	0.61	0.61
Total Split (s)	13.8	13.8	0.69	0.75	0.33	0.62	0.51	0.07	33.1	10.3	1.2	1.2
Act Effct Green (s)	0.14	0.14	0.0	0.0	0.0	0.0	0.0	0.0	33.1	10.3	1.2	1.2
Actuated g/C Ratio	46.6	71.7	39.9	71.7	39.9	33.1	10.3	1.2	33.1	10.3	1.2	1.2
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	46.6	71.7	39.9	71.7	39.9	33.1	10.3	1.2	33.1	10.3	1.2	1.2
Total Delay	D	E	D	E	D	E	D	E	C	B	A	A
LOS	D	E	D	E	D	E	D	E	C	B	A	A
Approach Delay	46.6	71.7	39.9	71.7	39.9	33.1	10.3	1.2	33.1	10.3	1.2	1.2
Approach LOS	D	E	D	E	D	E	D	E	C	B	A	A

A-45



Splits and Phases: 2: Kenwood Rd & Oakdale Rd

Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Base SAT
5/18/2007

Lane Group	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	1770	5085	1583	0	1770	1583	0
Satd. Flow (prot)	0	0.113	0.950	0	0	0.950	0	0
Fit Permitted	0	210	5085	1583	0	1770	1583	0
Satd. Flow (perm)	10	38	1446	46	3	64	4	12
Satd. Flow (RTOR)	0	51	1538	49	0	72	17	0
Volume (vph)	pm+pt	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm
Lane Group Flow (vph)	1	1	6	2	13	13	13	13
Turn Type	Protected Phases	Permitted Phases	Total Split (s)	Act Effct Green (s)	Actuated g/C Ratio	Control Delay	Queue Delay	Total Delay
Protected Phases	6	6	14.0	14.0	0.52	0.31	0.08	0.31
Permitted Phases	61.0	61.0	52.3	52.3	0.52	0.31	0.08	0.31
Total Split (s)	0.61	0.61	0.50	0.06	0.31	0.08	0.00	0.00
Act Effct Green (s)	10.1	11.6	4.2	44.9	23.6	23.6	23.6	23.6
Actuated g/C Ratio	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	10.1	11.6	4.2	44.9	23.6	23.6	23.6	23.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.1	11.6	4.2	44.9	23.6	23.6	23.6	23.6
LOS	B	B	A	A	D	D	C	C
Approach Delay	11.3	11.3	40.8	40.8	D	D	D	D
Approach LOS	B	B	D	D	D	D	D	D

Intersection Summary

Baseline
A & R Engineering Inc.

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Baseline
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HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Base SAT
5/18/2007

Movement	EBL2	EBL	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations			↔	↔	↔	↔	↔		↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.0			4.0	4.0			4.0	4.0		
Lane Util. Factor		1.00			1.00	1.00			1.00	0.91		1.00
Frt		0.94			1.00	0.88			1.00	1.00		0.85
Flt Protected		0.98			0.95	1.00			0.95	1.00		1.00
Satd. Flow (prot)		1707			1770	1631			1770	5075		1583
Flt Permitted		0.83			0.53	1.00			0.14	1.00		1.00
Satd. Flow (perm)		1441			982	1631			266	5075		1583
Volume (vph)	54	6	14	66	94	12	51	5	91	1305	18	64
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.89	0.89	0.89	0.89	0.97	0.97	0.97	0.97
Adj. Flow (vph)	62	7	16	76	106	13	57	6	94	1345	19	66
RTOR Reduction (vph)	0	0	33	0	0	3	0	0	0	0	0	26
Lane Group Flow (vph)	0	0	128	0	106	73	0	0	94	1364	0	40
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	custom	custom
Protected Phases			8			4				2		
Permitted Phases	8	8			4				2			6
Actuated Green, G (s)	13.8	13.8	13.8	13.8	13.8	13.8	13.8	13.8	51.5	51.5	51.5	61.0
Effective Green, g (s)	13.8	13.8	13.8	13.8	13.8	13.8	13.8	13.8	51.5	51.5	51.5	61.0
Actuated g/C Ratio	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.52	0.52	0.52	0.61
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	199	136	225	136	225	137	2614	966	966	966	966	966
v/s Ratio Prot					0.04					0.27		
v/s Ratio Perm	0.09	0.11			0.04				0.35		0.03	
v/c Ratio	0.64	0.78	0.32	0.32	0.78	0.32	0.32	0.69	0.52	0.04	0.04	
Uniform Delay, d1	40.8	41.6	38.9	38.9	41.6	38.9	38.9	18.2	16.1	7.8	7.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.73	0.59	0.54	0.54	
Incremental Delay, d2	7.0	24.0	0.8	0.8	24.0	0.8	0.8	21.8	0.7	0.1	0.1	
Delay (s)	47.8	65.7	39.7	39.7	65.7	39.7	39.7	35.2	10.2	4.3	4.3	
Level of Service	D	E	D	D	E	D	D	D	D	B	A	
Approach Delay (s)	47.8				54.8					11.5		
Approach LOS	D				D					B		
Intersection Summary												
HCM Average Control Delay	16.0	HCM Level of Service										
HCM Volume to Capacity ratio	0.64	B										
Actuated Cycle Length (s)	100.0	Sum of lost time (s)										
Intersection Capacity Utilization	64.8%	C										
Analysis Period (min)	15											
Critical Lane Group												

Baseline
A & R Engineering Inc.

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HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Base SAT
5/18/2007

Movement	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	1.00	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.85	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5085	1770	1583	1583
Flt Permitted	0.12	1.00	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	232	5085	1583	1770	5085	1770	1583	1583
Volume (vph)	10	38	1446	46	3	64	4	12
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.93	0.93	0.93	0.93
Adj. Flow (vph)	11	40	1538	49	3	69	4	13
RTOR Reduction (vph)	0	0	0	24	0	0	11	0
Lane Group Flow (vph)	0	51	1538	25	0	72	6	0
Turn Type	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm	Perm
Protected Phases	1	1	6	2	13	13	13	13
Permitted Phases	6	6	6	2	13	13	13	13
Actuated Green, G (s)	61.0	61.0	61.0	51.5	13.2	13.2	13.2	13.2
Effective Green, g (s)	61.0	61.0	61.0	51.5	13.2	13.2	13.2	13.2
Actuated g/C Ratio	0.61	0.61	0.61	0.52	0.13	0.13	0.13	0.13
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	226	3102	815	234	209	209	209	209
v/s Ratio Prot	0.01	0.30	0.02	0.02	0.04	0.04	0.04	0.04
v/s Ratio Perm	0.13	0.23	0.50	0.03	0.31	0.31	0.31	0.31
Uniform Delay, d1	9.9	10.9	12.0	39.3	37.8	37.8	37.8	37.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	0.6	0.1	3.4	0.2	0.2	0.2	0.2
Delay (s)	10.4	11.5	12.0	42.6	38.0	38.0	38.0	38.0
Level of Service	B	B	B	B	D	D	D	D
Approach Delay (s)	11.5	11.5	11.5	41.8	41.8	41.8	41.8	41.8
Approach LOS	B	B	B	D	D	D	D	D
Intersection Summary								

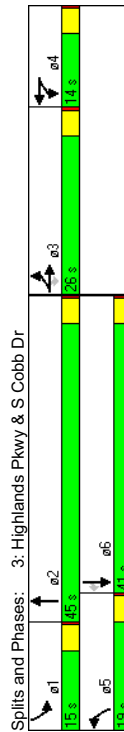
Baseline
A & R Engineering Inc.

Synchro 6 Report
Page 6

Lanes, Volumes, Timings
3: Highlands Pkwy & S Cobb Dr

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Total Flow (vph)	1681	1697	1583	0	1780	0	3433	5045	0	1770	5085	1583
Satd. Flow (prot)	0.950	0.959		0.972			0.950			0.950		
Fit Permitted	1681	1697	1583	0	1780	0	3433	5045	0	1770	5085	1583
Satd. Flow (perm)	0.95	0.96	1.00	0.98	1.00	0.97	0.95	1.00	0.99	1.00	0.95	1.00
Fit Protected	1681	1697	1583	0	1780	0	3433	5045	0	1770	5085	1583
Satd. Flow (RTOR)	248	15	274	63	31	15	271	1210	58	25	1342	276
Volume (vph)	143	151	285	0	132	0	295	1314	0	33	1369	294
Lane Group Flow (vph)	Split	Perm	Split	Perm	Split	Perm	Prot	Prot	Prot	Prot	Perm	Perm
Turn Type	3	3	3	4	4	4	5	2	2	1	6	6
Protected Phases												
Permitted Phases												
Total Split (s)	26.0	26.0	26.0	14.0	14.0	14.0	0.0	19.0	45.0	0.0	15.0	41.0
Act Effct Green (s)	14.1	14.1	14.1	11.8	11.8	11.8	13.0	54.9	7.1	45.1	45.1	45.1
Actuated g/C Ratio	0.14	0.14	0.14	0.12	0.12	0.12	0.13	0.55	0.07	0.45	0.45	0.45
v/c Ratio	0.60	0.63	0.61	0.61	0.61	0.61	0.66	0.47	0.26	0.60	0.38	0.38
Control Delay	50.2	51.5	10.8	52.4	52.4	52.4	37.9	24.8	51.8	18.0	9.3	9.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.2	51.5	10.8	52.4	52.4	52.4	37.9	24.8	51.8	18.0	9.3	9.3
LOS	D	D	B	D	D	D	D	C	D	B	B	A
Approach Delay	31.2			52.4			27.2				17.2	
Approach LOS	C			D			C				B	
Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 69 (69%), Referenced to phase 2:NBT and 6:SBT, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.66												
Intersection Signal Delay: 24.4												
Intersection Capacity Utilization 58.9%												
ICU Level of Service B												
Analysis Period (min) 15												



HCM Signalized Intersection Capacity Analysis
3: Highlands Pkwy & S Cobb Dr

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	1.00	1.00	0.98	1.00	0.99	1.00	0.99	1.00	1.00	0.85
Fit Protected	1.00	1.00	0.85	1.00	0.97	1.00	0.95	1.00	0.95	1.00	1.00	0.85
Satd. Flow (prot)	1681	1697	1583	1778	1778	1778	3433	5046	1770	5085	1583	1583
Fit Permitted	1681	1697	1583	1778	1778	1778	3433	5046	1770	5085	1583	1583
Satd. Flow (perm)	1681	1697	1583	1778	1778	1778	3433	5046	1770	5085	1583	1583
Volume (vph)	248	15	274	63	31	15	271	1210	58	25	1342	276
Peak-hour factor, PHF	0.91	0.70	0.96	0.82	0.81	0.88	0.92	0.97	0.86	0.75	0.98	0.94
Adj. Flow (vph)	273	21	285	77	38	17	295	1247	67	33	1369	294
RTOR Reduction (vph)	0	0	241	0	5	0	0	5	0	0	0	63
Lane Group Flow (vph)	Split	Perm	Split	Perm	Split	Perm	Prot	Prot	Prot	Prot	Perm	Perm
Turn Type	3	3	3	4	4	4	5	2	2	1	6	6
Protected Phases												
Permitted Phases												
Actuated Green, G (s)	14.1	14.1	14.1	11.8	11.8	11.8	13.0	53.3	4.8	45.1	45.1	45.1
Effective Green, g (s)	14.1	14.1	14.1	11.8	11.8	11.8	13.0	53.3	4.8	45.1	45.1	45.1
Actuated g/C Ratio	0.14	0.14	0.14	0.12	0.12	0.12	0.13	0.53	0.05	0.45	0.45	0.45
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	237	239	223	210	210	210	446	2690	85	2293	714	714
v/s Ratio Prot	0.09	c0.09	0.03	c0.07	c0.07	c0.09	0.26			0.02	c0.27	
v/s Ratio Perm	0.60	0.63	0.20	0.60	0.60	0.66	0.49			0.39	0.60	0.32
Uniform Delay, d1	40.3	40.5	37.9	41.9	41.9	41.4	14.7			46.2	20.6	17.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.81	1.53			1.09	0.76	0.63
Incremental Delay, d2	4.3	5.4	0.4	4.8	4.8	2.3	0.4			2.6	1.0	1.0
Delay (s)	44.6	45.9	38.4	46.7	46.7	35.7	23.0			52.9	16.7	12.2
Level of Service	D	D	D	D	D	D	C			D	B	B
Approach Delay (s)	41.9			46.7			25.3			16.6		
Approach LOS	D			D			C			B		
Intersection Summary												
HCM Average Control Delay			24.7				HCM Level of Service			C		
HCM Volume to Capacity ratio			0.61									
Actuated Cycle Length (s)			100.0				Sum of lost time (s)			16.0		
Intersection Capacity Utilization			58.9%				ICU Level of Service			B		
Analysis Period (min)			15									
c Critical Lane Group												

Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

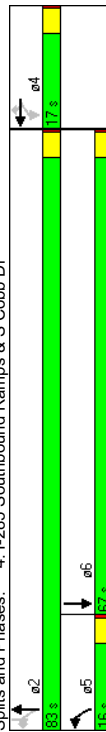
Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	0	1681	1455	1504	1770	3539	0	0	3539
Satd. Flow (prot)	0	0	0	0.950	0.996	0.184	0.184	0.184	0	0	0.184
Flt Permitted	0	0	0	1681	1455	1504	343	3539	0	0	3539
Satd. Flow (RTOR)	0	0	0	166	166	166	166	166	0	0	166
Volume (vph)	0	0	0	251	0	787	63	1077	0	0	1094
Satd. Flow (prot)	0	0	0	274	466	435	77	1134	0	0	1128
Lane Group Flow (vph)	0	0	0	274	466	435	77	1134	0	0	1128
Turn Type	Perm	Perm	Perm	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	Perm	Perm	pm+pt
Protected Phases	4	4	4	5	2	6	6	6	6	6	6
Permitted Phases	0.0	0.0	0.0	17.0	17.0	17.0	16.0	83.0	0.0	0.0	67.0
Total Split (s)	0.0	0.0	0.0	13.0	13.0	13.0	13.0	79.0	0.0	0.0	71.0
Act Effct Green (s)	0.0	0.0	0.0	13.0	13.0	13.0	13.0	79.0	0.0	0.0	71.0
Actuated g/C Ratio	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.79	0.79	0.79	0.79
Control Delay	182.9	218.7	170.7	2.3	2.5	4.8	4.8	4.8	4.8	4.8	4.8
Queue Delay	77.0	52.5	0.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	259.9	271.2	170.7	2.3	2.8	4.8	4.8	4.8	4.8	4.8	4.8
LOS	F	F	F	A	A	A	A	A	A	A	A
Approach Delay	231.4										
Approach LOS	F										

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Intersection Summary											
Cycle Length: 100											
Actuated Cycle Length: 100											
Offset: 52 (52%), Referenced to phase 2:NBTL and 6:SBT, Start of Green											
Control Type: Actuated-Coordinated											
Maximum v/c Ratio: 1.40											
Intersection Signal Delay: 67.4											
Intersection Capacity Utilization 98.4%											
ICU Level of Service F											
Analysis Period (min) 15											

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
4: I-285 Southbound Ramps & S Cobb Dr

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.91	0.95	1.00	0.85	1.00	0.95	1.00	0.95	1.00	0.85
Flt Protected	1.00	0.86	1.00	1.00	0.95	1.00	1.00	1.00	0.95	1.00	0.85
Satd. Flow (prot)	1681	1455	1504	1770	3539	3539	1583	1583	1583	1583	1583
Flt Permitted	0.95	1.00	1.00	0.20	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1681	1455	1504	377	3539	3539	1583	1583	1583	1583	1583
Volume (vph)	0	0	0	251	0	787	63	1077	0	0	1094
Peak-hour factor, PHF	0.92	0.92	0.92	0.81	0.92	0.91	0.82	0.95	0.92	0.92	0.95
Adj. Flow (vph)	0	0	0	310	0	865	77	1134	0	0	1128
RTOR Reduction (vph)	0	0	0	0	144	144	0	0	0	0	147
Lane Group Flow (vph)	0	0	0	274	322	291	77	1134	0	0	1128
Turn Type	Perm	Perm	Perm	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt	Perm	Perm	pm+pt
Protected Phases	4	4	4	5	2	6	6	6	6	6	6
Permitted Phases	13.0	13.0	13.0	13.0	13.0	13.0	79.0	79.0	79.0	79.0	79.0
Actuated Green, G (s)	13.0	13.0	13.0	13.0	13.0	13.0	79.0	79.0	79.0	79.0	79.0
Effective Green, g (s)	0.13	0.13	0.13	0.13	0.13	0.13	0.79	0.79	0.79	0.79	0.79
Actuated g/C Ratio	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	219	189	196	365	2796	2484	1251	1251	1251	1251	1251
Lane Grp Cap (vph)	0.16	0.22	0.19	0.16	0.16	0.16	0.16	0.16	0.16	0.16	0.16
v/s Ratio Prot	1.25	1.70	1.48	0.21	0.41	0.41	0.41	0.41	0.41	0.41	0.41
v/s Ratio Perm	43.5	43.5	43.5	3.6	3.2	3.2	3.2	3.2	3.2	3.2	3.2
Uniform Delay, d1	1.00	1.00	1.00	0.94	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Progression Factor	144.9	337.3	242.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Incremental Delay, d2	188.4	380.8	285.9	3.4	2.5	2.5	2.5	2.5	2.5	2.5	2.5
Delay (s)	F	F	F	F	F	F	F	F	F	F	F
Level of Service	A	A	A	A	A	A	A	A	A	A	A
Approach Delay (s)	0.0	0.0	0.0	300.8	2.5	24.9	24.9	24.9	24.9	24.9	24.9
Approach LOS	A	A	A	F	F	F	F	F	F	F	F
Intersection Summary											
HCM Average Control Delay	95.4										
HCM Volume to Capacity ratio	0.62										
Actuated Cycle Length (s)	100.0										
Intersection Capacity Utilization	98.4%										
Analysis Period (min)	15										
c Critical Lane Group											

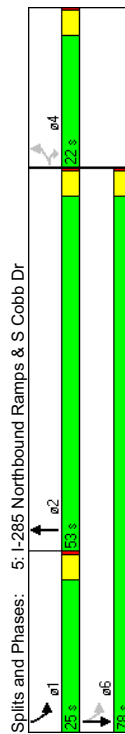
Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	0	3306	0	1770	3539	0
Flt Permitted	0.950							0.274				
Satd. Flow (perm)	3433	0	1583	0	0	0	0	3306	0	510	3539	0
Satd. Flow (RTOR)			64					281				
Volume (vph)	748	0	56	0	0	0	0	381	297	849	548	0
Lane Group Flow (vph)	813	0	64	0	0	0	0	736	0	884	583	0
Turn Type	custom	custom	custom							pm+pt		
Protected Phases	4		4					2		1	6	
Permitted Phases												
Total Split (s)	22.0	0.0	22.0	0.0	0.0	0.0	0.0	53.0	0.0	25.0	78.0	0.0
Act Effct Green (s)	18.0		18.0					49.0		74.0	74.0	
Actuated g/C Ratio	0.18		0.18					0.49		0.74	0.74	
Control Delay	187.9		10.7					10.4		195.3	2.5	
Queue Delay	0.0		0.0					0.0		73.0	0.0	
Total Delay	187.9		10.7					10.4		268.3	2.5	
LOS	F		B					B		F	A	
Approach Delay											162.7	
Approach LOS											F	

A-49

Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 22 (22%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.38												
Intersection Signal Delay: 129.8												
Intersection Capacity Utilization 98.4%												
Analysis Period (min) 15												



Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0		4.0					4.0		4.0	4.0	
Lane Util. Factor	0.97		1.00					0.95		1.00	0.95	
Flt Protected	1.00		0.85					0.93		1.00	1.00	
Satd. Flow (prot)	3433		1583					3304		1770	3539	
Flt Permitted	0.95		1.00					1.00		0.27	1.00	
Satd. Flow (perm)	3433		1583					3304		510	3539	
Volume (vph)	748	0	56	0	0	0	0	381	297	849	548	0
Peak-hour factor, PHF	0.92	0.92	0.88	0.92	0.92	0.92	0.92	0.93	0.91	0.96	0.94	0.92
Adj. Flow (vph)	813	0	64	0	0	0	0	410	326	884	583	0
RTOR Reduction (vph)	0	0	52	0	0	0	0	143	0	0	0	0
Lane Group Flow (vph)	813	0	12	0	0	0	0	593	0	884	583	0
Turn Type	custom	custom	custom							pm+pt		
Protected Phases								2		1	6	
Permitted Phases	4		4									
Actuated Green, G (s)	18.0		18.0					49.0		74.0	74.0	
Effective Green, g (s)	18.0		18.0					49.0		74.0	74.0	
Actuated g/C Ratio	0.18		0.18					0.49		0.74	0.74	
Clearance Time (s)	4.0		4.0					4.0		4.0	4.0	
Vehicle Extension (s)	3.0		3.0					3.0		3.0	3.0	
Lane Grp Cap (vph)	618		285					1619		642	2619	
v/s Ratio Prot								0.18		0.29	0.16	
v/s Ratio Perm	0.24		0.01							0.73		
v/c Ratio	1.32		0.04					0.37		1.38	0.22	
Uniform Delay, d1	41.0		33.9					15.8		15.9	4.0	
Progression Factor	1.00		1.00					1.00		0.88	0.58	
Incremental Delay, d2	153.2		0.1					0.6		177.4	0.2	
Delay (s)	194.2		33.9					16.5		191.3	2.5	
Level of Service	F		C					B		F	A	
Approach Delay (s)			182.5			0.0		16.5			116.3	
Approach LOS			F			A		B			F	
Intersection Summary												
HCM Average Control Delay	111.3											
HCM Volume to Capacity ratio	1.35											
Actuated Cycle Length (s)	100.0											
Intersection Capacity Utilization	98.4%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
6: Church Rd & N Church Lane

Base SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1827	0	0	1783	0	0	1736	0	0	1693	0
Flt Permitted	0	1827	0	0	1783	0	0	1736	0	0	1693	0
Satd. Flow (perm)	0	1827	0	0	1783	0	0	1736	0	0	1693	0
Volume (vph)	31	65	1	0	67	37	0	1	1	40	0	43
Lane Group Flow (vph)	0	153	0	0	131	0	0	8	0	0	105	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 30.0%												
Analysis Period (min) 15												

HCM Unsignalized Intersection Capacity Analysis
6: Church Rd & N Church Lane

Base SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	31	65	1	0	67	37	0	1	1	40	0	43
Peak Hour Factor	0.66	0.64	0.25	0.92	0.75	0.88	0.92	0.25	0.25	0.79	0.92	0.79
Hourly flow rate (vph)	47	102	4	0	89	42	0	4	4	51	0	54
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	89			106			362	287	104	314	310	110
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	89			106			362	287	104	314	310	110
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	97			100			100	99	100	92	100	94
cM capacity (veh/h)	1506			1486			546	603	951	618	586	943
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	153	131	8	105								
Volume Left	47	0	0	51								
Volume Right	4	42	4	54								
cSH	1506	1486	738	752								
Volume to Capacity	0.03	0.00	0.01	0.14								
Queue Length 95th (ft)	2	0	1	12								
Control Delay (s)	2.5	0.0	9.9	10.6								
Lane LOS	A	A	A	B								
Approach Delay (s)	2.5	0.0	9.9	10.6								
Approach LOS	A	A	B	B								
Intersection Summary												
Average Delay				3.9								
Intersection Capacity Utilization			30.0%				ICU Level of Service		A			
Analysis Period (min)			15									

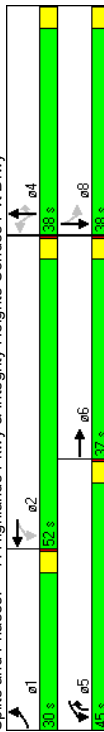
Lanes, Volumes, Timings
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3433	0	1770	3529	0	1770	1863	1583	0	1863	0
Flt Permitted	0.950			0.464			0.755					
Satd. Flow (perm)	1770	3433	0	864	3529	0	1406	1863	1583	0	1863	0
Satd. Flow (RTOR)	25			2								
Volume (vph)	1	209	43	311	198	1	37	1	303	0	2	0
Lane Group Flow (vph)	4	274	0	324	224	0	46	1	322	0	4	0
Turn Type	Prot	pm+pt		Perm	pm+ov	Perm						
Protected Phases	1	6	5	2			4	5		8		
Permitted Phases												
Total Split (s)	30.0	37.0	0.0	45.0	52.0	0.0	38.0	38.0	45.0	38.0	38.0	0.0
Act Effct Green (s)	5.8	30.5		51.7	51.0		7.6	7.6	27.2		7.5	
Actuated g/C Ratio	0.08	0.48		0.84	0.82		0.12	0.12	0.44		0.11	
Control Delay	33.0	8.6		3.0	3.1		32.1	27.0	3.5		27.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0		0.0	
Total Delay	33.0	8.6		3.0	3.1		32.1	27.0	3.5		27.0	
LOS	C	A		A	A		C	C	A		C	
Approach Delay												
Approach LOS												

Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 61.9												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.37												
Intersection Signal Delay: 5.7												
Intersection Capacity Utilization 43.1%												
Analysis Period (min) 15												

Splits and Phases: 7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy



HCM Signalized Intersection Capacity Analysis
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	1.00	1.00	1.00	1.00	
Flt	1.00	0.97		1.00	1.00		1.00	1.00	0.85	1.00	1.00	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	1.00	1.00	
Satd. Flow (prot)	1770	3435	1770	3530	1770	3530	1770	1863	1583	1863	1863	
Flt Permitted	0.95	1.00		0.52	1.00		0.82	1.00	1.00	1.00	1.00	
Satd. Flow (perm)	1770	3435	962	3530			1521	1863	1583	1863		
Volume (vph)	1	209	43	311	198	1	37	1	303	0	2	0
Peak-hour factor, PHF	0.25	0.95	0.79	0.96	0.90	0.25	0.80	0.92	0.94	0.92	0.50	0.92
Adj. Flow (vph)	4	220	54	324	220	4	46	1	322	0	4	0
RTOR Reduction (vph)	0	13	0	0	1	0	0	0	206	0	0	0
Lane Group Flow (vph)	4	261	0	324	223	0	46	1	116	0	4	0
Turn Type	Prot			pm+pt			Perm		pm+ov	Perm		
Protected Phases	1	6	5	2			4	5		8		
Permitted Phases				2				4		8		
Actuated Green, G (s)	1.0	30.9		54.1	49.1		4.9	4.9	24.1		4.9	
Effective Green, g (s)	1.0	30.9		54.1	49.1		4.9	4.9	24.1		4.9	
Actuated g/C Ratio	0.01	0.46		0.81	0.73		0.07	0.07	0.36		0.07	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0	4.0		4.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	26	1584		1008	2587		111	136	664		136	
v/s Ratio Prot	0.00	0.08		c0.09	0.06		0.00	c0.05	0.00		0.00	
v/s Ratio Perm				c0.17			c0.03		0.02			
v/c Ratio	0.15	0.16		0.32	0.09		0.41	0.01	0.17		0.03	
Uniform Delay, d1	32.6	10.5		1.7	2.6		29.7	28.8	14.7		28.8	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00		1.00	
Incremental Delay, d2	2.7	0.0		0.2	0.1		2.5	0.0	0.1		0.1	
Delay (s)	35.3	10.6		1.9	2.6		32.2	28.8	14.8		28.9	
Level of Service	D	B		A	A		C	C	B		C	
Approach Delay (s)												
Approach LOS												
Intersection Summary												
HCM Average Control Delay			8.9				HCM Level of Service			A		
HCM Volume to Capacity ratio			0.31									
Actuated Cycle Length (s)			67.0				Sum of lost time (s)			4.0		
Intersection Capacity Utilization			43.1%				ICU Level of Service			A		
Analysis Period (min)			15									
Critical Lane Group												

Lanes, Volumes, Timings
8: US 278 (US 78) & Oakdale Rd

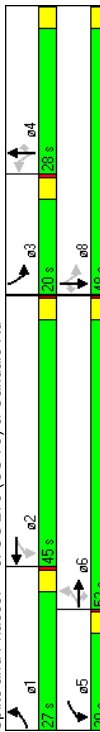
Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3497	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.179			0.290			0.690			0.523		
Satd. Flow (perm)	333	3539	1583	540	3497	0	1285	1863	1583	974	1863	1583
Satd. Flow (RTOR)				68			8			59		
Volume (vph)	147	667	65	73	642	52	63	98	56	52	79	180
Lane Group Flow (vph)	165	741	76	85	764	0	81	115	59	63	103	200
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt
Protected Phases	1	6	5	2			4	4	3	8		
Permitted Phases	6	6	2				4	4	8			
Total Split (s)	27.0	52.0	20.0	45.0	0.0	28.0	28.0	20.0	48.0	48.0		
Act Eff Green (s)	57.5	50.3	53.4	46.5		10.9	10.9	10.9	20.7	20.4	20.4	
Actuated g/C Ratio	0.65	0.57	0.57	0.59	0.53	0.12	0.12	0.12	0.23	0.23	0.23	0.23
v/c Ratio	0.46	0.37	0.08	0.20	0.41	0.51	0.50	0.24	0.21	0.24	0.38	
Control Delay	11.0	13.5	4.5	8.1	15.1	49.3	45.1	12.6	27.5	27.8	6.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	11.0	13.5	4.5	8.1	15.1	49.3	45.1	12.6	27.5	27.8	6.2	
LOS	B	B	A	A	B	D	D	B	C	C	A	
Approach Delay		12.4		14.4				38.9			16.0	
Approach LOS		B		B				D			B	

Intersection Summary

Cycle Length: 120
Actuated Cycle Length: 87.9
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.51
Intersection Signal Delay: 16.4
Intersection Capacity Utilization 47.7%
Analysis Period (min) 15

Splits and Phases: 8: US 278 (US 78) & Oakdale Rd



HCM Signalized Intersection Capacity Analysis
8: US 278 (US 78) & Oakdale Rd

Base SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3498		1770	1863	1583	1770	1863	1583
Flt Permitted	0.28	1.00	1.00	0.34	1.00		0.69	1.00	1.00	0.44	1.00	1.00
Satd. Flow (perm)	529	3539	1583	629	3498		1286	1863	1583	813	1863	1583
Volume (vph)	147	667	65	73	642	52	63	98	56	52	79	180
Peak-hour factor, PHF	0.89	0.90	0.85	0.86	0.91	0.88	0.78	0.85	0.95	0.82	0.77	0.90
Adj. Flow (vph)	165	741	76	85	705	59	81	115	59	63	103	200
RTOR Reduction (vph)	0	0	30	0	4	0	0	0	52	0	0	152
Lane Group Flow (vph)	165	741	46	85	760	0	81	115	7	63	103	48
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt
Protected Phases	1	6	5	2			4	4	3	8		
Permitted Phases	6	6	2				4	4	8			
Actuated Green, G (s)	59.0	50.3	50.3	53.6	47.6		10.9	10.9	10.9	21.4	21.4	21.4
Effective Green, g (s)	59.0	50.3	50.3	53.6	47.6		10.9	10.9	10.9	21.4	21.4	21.4
Actuated g/C Ratio	0.66	0.56	0.56	0.60	0.53		0.12	0.12	0.12	0.24	0.24	0.24
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	468	1985	888	452	1856		156	226	192	263	444	378
v/s Ratio Prot	c0.03	0.21		0.01	c0.22		c0.06	0.06	0.02	c0.06		
v/s Ratio Perm	0.20		0.03	0.10			0.52	0.51	0.04	0.24	0.23	0.13
Uniform Delay, d1	6.7	10.9	8.9	7.7	12.6		36.9	36.9	34.8	27.1	27.5	26.8
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	0.5	0.1	0.2	0.7		2.9	1.8	0.1	0.5	0.3	0.2
Delay (s)	7.1	11.5	9.0	7.9	13.3		39.8	38.7	34.9	27.6	27.8	27.0
Level of Service	A	B	A	A	B		D	D	C	C	C	C
Approach Delay (s)		10.6			12.8			38.2			27.3	
Approach LOS		B			B			D			C	

Intersection Summary

HCM Average Control Delay	16.7	HCM Level of Service	B
HCM Volume to Capacity ratio	0.43		
Actuated Cycle Length (s)	89.7	Sum of lost time (s)	20.0
Intersection Capacity Utilization	47.7%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔				↔	↔	↔	↔	↔	↔	↔
Satd. Flow (prot)	0	1625	0	0	0	1611	1770	5085	1583	0	5080	0
Flt Permitted		0.996					0.950					
Satd. Flow (perm)	0	1625	0	0	0	1611	1770	5085	1583	0	5080	0
Volume (vph)	16	0	221	0	0	51	228	1554	49	0	1705	13
Lane Group Flow (vph)	0	259	0	0	0	64	285	1636	60	0	1793	0
Sign Control		Stop			Yield		Free				Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 70.4%												
Analysis Period (min) 15												

Intersection has too many lanes per leg.
HCM All-Way analysis is limited to two lanes per leg.
Channelized right turn lanes are not counted.

Lanes, Volumes, Timings
10: Church Rd & Groover Rd

Base SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1820	0	0	1857	0	0	1717	0	0	1611	0
Flt Permitted	0	0.999	0	0	0.997	0	0	0.969	0	0	0.969	0
Satd. Flow (perm)	0	1820	0	0	1857	0	0	1717	0	0	1611	0
Volume (vph)	2	98	19	7	101	0	17	0	7	0	0	2
Lane Group Flow (vph)	0	156	0	0	136	0	0	33	0	0	4	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	23.9%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
10: Church Rd & Groover Rd

Base SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	2	98	19	7	101	0	17	0	7	0	0	2
Peak Hour Factor	0.50	0.77	0.75	0.88	0.79	0.92	0.80	0.92	0.58	0.92	0.92	0.50
Hourly flow rate (vph)	4	127	25	8	128	0	21	0	12	0	0	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	128			153			296	292	140	304	304	128
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
vCu, unblocked vol	128			153			296	292	140	304	304	128
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			97	100	99	100	100	100
cM capacity (veh/h)	1458			1428			650	614	908	636	604	922
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	157	136	33	4								
Volume Left	4	8	21	0								
Volume Right	25	0	12	4								
cSH	1458	1428	724	922								
Volume to Capacity	0.00	0.01	0.05	0.00								
Queue Length 95th (ft)	0	0	4	0								
Control Delay (s)	0.2	0.5	10.2	8.9								
Lane LOS	A	A	B	A								
Approach Delay (s)	0.2	0.5	10.2	8.9								
Approach LOS	B	B	A	A								
Intersection Summary												
Average Delay	1.4											
Intersection Capacity Utilization	23.9%											
Analysis Period (min)	15											

Base 2009 SAT Improved

Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

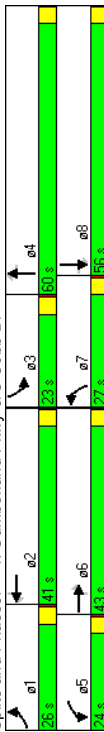
Base SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	292			114			3433	3539	1583	3433	3539	1583
Volume (vph)	136	843	254	124	854	149	358	948	115	249	926	125
Lane Group Flow (vph)	162	878	292	144	1005	157	385	1009	128	277	975	149
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases												
Total Split (s)	26.0	43.0	56.0	24.0	41.0	60.0	27.0	60.0	43.0	23.0	56.0	41.0
Act Effct Green (s)	16.6	39.7	42.4	15.2	38.4	46.0	19.0	46.0	39.7	15.3	42.4	38.4
Actuated g/C Ratio	0.13	0.30	0.32	0.11	0.29	0.35	0.14	0.35	0.30	0.12	0.32	0.29
v/c Ratio	0.73	0.58	0.41	0.71	0.68	0.25	0.78	0.82	0.25	0.70	0.86	0.29
Control Delay	77.4	43.6	5.4	77.9	47.1	11.0	68.4	46.1	25.0	68.1	51.5	21.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.4	43.6	5.4	77.9	47.1	11.0	68.4	46.1	25.0	68.1	51.5	21.4
LOS	E	D	A	E	D	B	E	D	C	E	D	C
Approach Delay												
Approach LOS												

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Intersection Summary
Cycle Length: 150
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.86
Intersection Signal Delay: 46.9
Intersection Capacity Utilization 73.2%
Analysis Period (min) 15

Splits and Phases: 1: Cumberland Pkwy & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Base SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.95	1.00	0.97	0.95	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	136	843	254	124	854	149	358	948	115	249	926	125
Peak-hour factor, PHF	0.84	0.96	0.87	0.86	0.85	0.95	0.93	0.94	0.90	0.90	0.95	0.84
Adj. Flow (vph)	162	878	292	144	1005	157	385	1009	128	277	975	149
RTOR Reduction (vph)	0	0	198	0	0	74	0	0	38	0	0	59
Lane Group Flow (vph)	162	878	94	144	1005	83	385	1009	90	277	975	90
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases												
Actuated Green, G (s)	16.6	39.8	42.4	15.2	38.4	46.1	19.0	46.1	39.8	15.3	42.4	38.4
Effective Green, g (s)	16.6	39.8	42.4	15.2	38.4	46.1	19.0	46.1	39.8	15.3	42.4	38.4
Actuated g/C Ratio	0.13	0.30	0.32	0.11	0.29	0.35	0.14	0.35	0.30	0.12	0.32	0.29
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	222	1529	507	203	1475	551	493	1232	476	397	1133	459
v/s Ratio Prot	c0.09	0.17	0.06	0.08	c0.20	0.05	c0.11	c0.29	0.08	0.08	0.28	0.06
v/s Ratio Perm	0.73	0.57	0.18	0.71	0.68	0.15	0.78	0.82	0.19	0.70	0.86	0.20
Uniform Delay, d1	55.7	39.1	32.5	56.5	41.6	29.7	54.7	39.3	34.3	56.3	42.2	35.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	11.4	1.6	0.2	10.8	2.6	0.1	7.9	4.4	0.9	5.3	6.9	1.0
Delay (s)	67.1	40.7	32.7	67.3	44.2	29.8	62.6	43.7	35.2	61.6	49.1	36.3
Level of Service	E	D	C	E	D	C	E	D	D	E	D	D
Approach Delay (s)												
Approach LOS												

Intersection Summary												
HCM Average Control Delay	46.4											
HCM Volume to Capacity ratio	0.77											
Actuated Cycle Length (s)	132.4											
Intersection Capacity Utilization	73.2%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

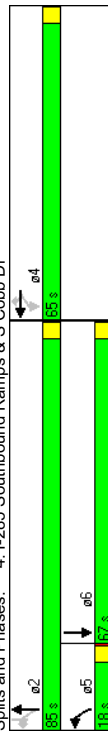
Base SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	0	1681	1441	1504	1770	3539	0	0	3539	1583
Satd. Flow (prot)	0	0	0	0.950	0.950	0.950	0.950	0.950	0	0	0.950	0.950
Fit Permitted	0	0	0	1681	1441	1504	1770	3539	0	0	3539	1583
Satd. Flow (perm)	0	0	0	1681	1441	1504	1770	3539	0	0	3539	1583
Fit Protected	0	0	0	0	0	0	0	0	0	0	0	0
Satd. Flow (RTOR)	0	0	0	0	0	0	0	0	0	0	0	0
Volume (vph)	0	0	0	251	0	787	63	1077	0	0	1094	666
Satd. Flow (prot)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	0	0	310	432	433	77	1134	0	0	1128	701
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	0.0	0.0	0.0	65.0	65.0	65.0	18.0	85.0	0.0	0.0	67.0	85.0
Total Split (s)	0.0	0.0	0.0	45.1	45.1	45.1	96.9	96.9	0.0	0.0	86.8	96.9
Act Effct Green (s)	0.0	0.0	0.0	45.1	45.1	45.1	96.9	96.9	0.0	0.0	86.8	96.9
Actuated g/C Ratio	0.0	0.0	0.0	0.30	0.30	0.30	0.65	0.65	0.0	0.0	0.58	0.65
v/c Ratio	0.0	0.0	0.0	0.61	0.90	0.87	0.44	0.50	0.0	0.0	0.55	0.56
Control Delay	0.0	0.0	0.0	48.8	63.7	58.7	19.0	0.6	0.0	0.0	11.1	2.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.5	0.0	0.0	0.1	0.0
Total Delay	0.0	0.0	0.0	48.8	63.7	58.7	19.0	1.1	0.0	0.0	11.3	2.4
LOS	D	E	E	E	E	E	B	A	A	A	B	A
Approach Delay	0.0	0.0	0.0	57.9	0.0	0.0	2.2	7.9	0.0	0.0	7.9	0.0
Approach LOS	A	A	A	E	E	E	A	A	A	A	B	A

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Cycle Length: 150
Actuated Cycle Length: 150
Offset: 132 (88%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.90
Intersection Signal Delay: 20.2
Intersection Capacity Utilization 96.8%
Analysis Period (min) 15

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



Baseline
A & R Engineering Inc.

Synchro 6 Report
Page 3

HCM Signalized Intersection Capacity Analysis
4: I-285 Southbound Ramps & S Cobb Dr

Base SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	0	0	0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Fit Protected	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (prot)	1681	1441	1504	1770	1770	1770	3539	3539	1583	1583	1583	1583
Fit Permitted	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (perm)	1681	1441	1504	1770	1770	1770	3539	3539	1583	1583	1583	1583
Volume (vph)	0	0	0	251	0	787	63	1077	0	0	1094	666
Peak-hour factor, PHF	0.92	0.92	0.92	0.81	0.92	0.91	0.82	0.95	0.92	0.92	0.97	0.95
Adj. Flow (vph)	0	0	0	310	0	865	77	1134	0	0	1128	701
RTOR Reduction (vph)	0	0	0	0	0	46	46	0	0	0	0	240
Lane Group Flow (vph)	0	0	0	310	386	387	77	1134	0	0	1128	461
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	45.1	45.1	45.1	96.9	96.9	96.9	96.9	96.9	96.9	96.9	96.9	96.9
Effective Green, g (s)	45.1	45.1	45.1	96.9	96.9	96.9	96.9	96.9	96.9	96.9	96.9	96.9
Actuated g/C Ratio	0.30	0.30	0.30	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	505	433	452	259	2286	0.01	c0.32	0.01	c0.32	0.01	c0.32	0.01
v/s Ratio Prot	0.18	0.27	0.26	0.18	0.18	0.18	0.18	0.18	0.18	0.18	0.18	0.18
v/c Ratio Perm	0.61	0.89	0.86	0.30	0.50	0.50	0.50	0.50	0.50	0.50	0.50	0.50
Uniform Delay, d1	45.0	50.1	49.4	13.7	13.8	13.8	13.8	13.8	13.8	13.8	13.8	13.8
Progression Factor	1.00	1.00	1.00	1.24	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
Incremental Delay, d2	2.2	19.9	14.7	0.3	0.3	0.3	0.3	0.3	0.3	0.3	0.3	0.3
Delay (s)	47.2	70.0	64.0	17.3	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Level of Service	D	E	E	B	A	A	A	A	A	A	A	A
Approach Delay (s)	0.0	0.0	0.0	61.8	1.7	1.7	9.4	9.4	9.4	9.4	9.4	9.4
Approach LOS	A	A	A	E	E	E	A	A	A	A	A	A
Intersection Summary												
HCM Average Control Delay	21.8											
HCM Volume to Capacity ratio	0.66											
Actuated Cycle Length (s)	150.0											
Intersection Capacity Utilization	96.8%											
Analysis Period (min)	15											
c Critical Lane Group												

Baseline
A & R Engineering Inc.

Synchro 6 Report
Page 4

Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

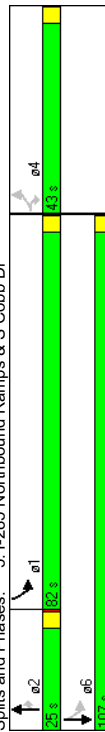
HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

Base SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3539	1583	1770	3539	0	0
Flt Permitted	0.950								0.234			
Satd. Flow (perm)	3433	0	1583	0	0	0	3539	1583	436	3539	0	0
Satd. Flow (RTOR)			64						295			
Volume (vph)	748	0	56	0	0	0	381	297	849	548	0	0
Lane Group Flow (vph)	813	0	64	0	0	0	410	326	884	583	0	0
Turn Type	custom	custom	custom						Perm	pm+pt		
Protected Phases	4		4				2	2	1	6		
Permitted Phases												
Total Split (s)	43.0	0.0	43.0	0.0	0.0	0.0	25.0	25.0	82.0	107.0	0.0	0.0
Act Effct Green (s)	37.9		37.9				22.1	22.1	104.1	104.1		
Actuated g/C Ratio	0.25		0.25				0.15	0.15	0.69	0.69		
Control Delay	73.3		10.0				73.5	16.7	45.8	8.8		
Queue Delay	0.4		0.0				0.0	0.0	0.0	0.0		
Total Delay	73.6		10.0				73.5	16.7	45.8	8.8		
LOS	E		B				E	B	D	A		
Approach Delay												
Approach LOS												

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 147 (98%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.94												
Intersection Signal Delay: 46.0												
Intersection Capacity Utilization 96.8%												
Analysis Period (min) 15												

Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.85	1.00	1.00	1.00	0.95	1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	3433	1583		3539	1583	1770	3539	1583	1770	3539		
Flt Permitted	0.95	1.00		1.00	1.00	1.00	0.25	1.00	1.00	0.25	1.00	
Satd. Flow (perm)	3433	1583		3539	1583	463	3539	1583	463	3539		
Volume (vph)	748	0	56	0	0	0	381	297	849	548	0	0
Peak-hour factor, PHF	0.92	0.92	0.88	0.92	0.92	0.92	0.93	0.91	0.96	0.94	0.92	0.92
Adj. Flow (vph)	813	0	64	0	0	0	410	326	884	583	0	0
RTOR Reduction (vph)	0	0	48	0	0	0	0	0	252	0	0	0
Lane Group Flow (vph)	813	0	16	0	0	0	410	410	74	884	583	0
Turn Type	custom	custom	custom						Perm	pm+pt		
Protected Phases	4		4				2	2	1	6		
Permitted Phases												
Actuated Green, G (s)	37.9		37.9				22.1	22.1	104.1	104.1		
Effective Green, g (s)	37.9		37.9				22.1	22.1	104.1	104.1		
Actuated g/C Ratio	0.25		0.25				0.15	0.15	0.69	0.69		
Clearance Time (s)	4.0		4.0				4.0	4.0	4.0	4.0		
Vehicle Extension (s)	3.0		3.0				3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	867		400				521	233	1001	2456		
v/s Ratio Prot							0.12			0.16		
v/s Ratio Perm	c0.24		0.01						0.05	c0.15		
v/c Ratio	0.94		0.04				0.79	0.32	0.88	0.24		
Uniform Delay, d1	54.9		42.3				61.7	57.2	25.6	8.4		
Progression Factor	1.00		1.00				1.00	1.00	1.26	1.00		
Incremental Delay, d2	17.2		0.0				11.4	3.6	8.0	0.2		
Delay (s)	72.1		42.4				73.1	60.8	40.2	8.6		
Level of Service	E		D				E	E	D	A		
Approach Delay (s)			69.9		0.0		67.6			27.6		
Approach LOS			E		A		E			C		
Intersection Summary												
HCM Average Control Delay			49.2							D		
HCM Volume to Capacity ratio			0.89									
Actuated Cycle Length (s)			150.0							8.0		
Intersection Capacity Utilization			96.8%							F		
Analysis Period (min)			15									
c Critical Lane Group												

Future Intersection Analysis

Future 2009 PM

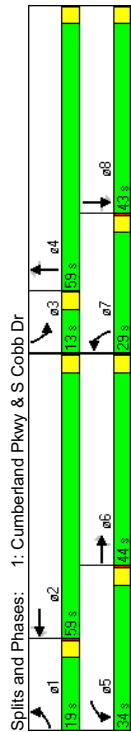
Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	122			34			68			82		
Volume (vph)	177	907	156	205	2033	129	586	1228	174	207	954	98
Lane Group Flow (vph)	203	945	184	228	2140	143	637	1364	212	233	984	124
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	7	4	3	8			
Permitted Phases												
Total Split (s)	19.0	44.0	43.0	34.0	59.0	59.0	29.0	44.0	13.0	43.0	59.0	2
Act Eff Green (s)	15.0	46.7	39.0	23.3	55.0	55.0	25.0	55.0	46.7	9.0	39.0	55.0
Actuated g/C Ratio	0.10	0.31	0.26	0.16	0.37	0.37	0.17	0.37	0.31	0.06	0.26	0.37
v/c Ratio	1.15	0.86	0.37	0.83	1.65	0.24	1.11	1.05	0.39	1.13	1.07	0.20
Control Delay	169.9	57.7	18.1	85.2	327.2	26.0	128.3	85.0	30.7	162.8	101.9	12.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	169.9	57.7	18.1	85.2	327.2	26.0	128.3	85.0	30.7	162.8	101.9	12.9
LOS	F	E	B	F	F	C	F	F	C	F	F	B
Approach Delay		69.3		288.1				92.3			104.3	
Approach LOS		E		F				F			F	

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Intersection Summary	
Cycle Length: 150	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.65	
Intersection Signal Delay: 156.8	Intersection LOS: F
Intersection Capacity Utilization 122.4%	ICU Level of Service H
Analysis Period (min) 15	



HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Future PM
5/18/2007

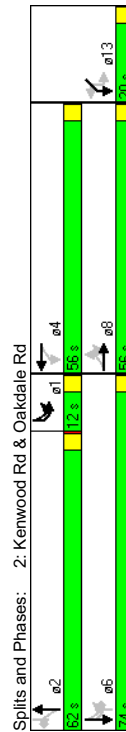
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	177	907	156	205	2033	129	586	1228	174	207	954	98
Peak-hour factor, PHF	0.87	0.96	0.85	0.90	0.95	0.90	0.92	0.90	0.82	0.89	0.97	0.79
Adj. Flow (vph)	203	945	184	228	2140	143	637	1364	212	233	984	124
RTOR Reduction (vph)	0	0	90	0	0	22	0	0	47	0	0	52
Lane Group Flow (vph)	203	945	94	228	2140	121	637	1364	165	233	984	72
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	7	4	3	8			
Permitted Phases												
Actuated Green, G (s)	15.0	46.7	39.0	23.3	55.0	55.0	25.0	55.0	46.7	9.0	39.0	55.0
Effective Green, g (s)	15.0	46.7	39.0	23.3	55.0	55.0	25.0	55.0	46.7	9.0	39.0	55.0
Actuated g/C Ratio	0.10	0.31	0.26	0.16	0.37	0.37	0.17	0.37	0.31	0.06	0.26	0.37
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	177	1102	412	275	1298	580	572	1298	493	206	920	580
v/s Ratio Prot	c0.11	0.27		0.13	c0.60		c0.19	c0.39		0.07	0.28	
v/s Ratio Perm			0.06			0.08			0.10			0.05
v/c Ratio	1.15	0.86	0.23	0.83	1.65	0.21	1.11	1.05	0.34	1.13	1.07	0.12
Uniform Delay, d1	67.5	48.5	43.7	61.4	47.5	32.6	62.5	47.5	39.7	70.5	55.5	31.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	112.7	8.7	0.3	18.3	295.4	0.2	72.8	39.5	1.8	102.4	50.1	0.4
Delay (s)	180.2	57.2	43.9	79.7	342.9	32.8	135.3	87.0	41.5	172.9	105.6	32.0
Level of Service	F	E	D	E	F	C	F	F	D	F	F	C
Approach Delay (s)		74.1		301.3			96.5			110.5		
Approach LOS		E		F			F			F		

Intersection Summary	
HCM Average Control Delay	164.5
HCM Volume to Capacity ratio	1.30
Actuated Cycle Length (s)	150.0
Intersection Capacity Utilization	122.4%
Analysis Period (min)	15
Critical Lane Group	
c	

Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Future PM
5/18/2007

Lane Group	EBL2	EBL	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	1729	0	1770	1628	0	0	1770	5070	0	1583
Satd. Flow (prot)	0	0	0.715	0	0.619	0.118						
Flt Permitted	0	0	1257	0	1153	1628	0	0	220	5070	0	1583
Satd. Flow (perm)	37	3	26	47	490	46	225	11	107	1842	37	215
Satd. Flow (RTOR)	0	0	151	0	505	290	0	0	108	1898	0	217
Volume (vph)	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	custom	custom
Lane Group Flow (vph)	8	8	8	8	4	4	4	4	2	2	6	6
Turn Type	Protected Phases	Permitted Phases	Total Split (s)	Act Effct Green (s)	Actuated g/C Ratio	v/c Ratio	Control Delay	Queue Delay	Total Delay	LOS	Approach Delay	Approach LOS
Protected Phases	8	8	56.0	56.0	0.0	56.0	56.0	0.0	0.0	62.0	62.0	74.0
Permitted Phases	56.0	56.0	52.0	52.0	0.35	0.35	0.35	0.35	0.39	0.39	0.47	0.47
Total Split (s)	52.0	52.0	1.26	1.26	0.51	1.27	0.97	0.28	0.0	0.0	40.2	0.0
Act Effct Green (s)	0.35	0.35	177.3	177.3	42.7	151.2	15.8	2.0	2.0	2.0	2.0	2.0
Actuated g/C Ratio	0.33	0.33	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
v/c Ratio	32.1	32.1	177.3	177.3	42.7	151.2	56.1	2.0	2.0	2.0	2.0	2.0
Control Delay	C	C	F	D	F	D	F	E	A	A	A	A
Queue Delay	32.1	32.1	128.2	128.2	55.4	55.4	55.4	55.4	55.4	55.4	55.4	55.4
LOS	C	C	F	D	F	D	F	E	A	A	A	A
Approach Delay	32.1	32.1	128.2	128.2	55.4	55.4	55.4	55.4	55.4	55.4	55.4	55.4
Approach LOS	C	C	F	D	F	D	F	E	A	A	A	A
Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 112 (75%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.27												
Intersection Signal Delay: 63.4												
Intersection Capacity Utilization 102.7%												
ICU Level of Service G												
Analysis Period (min) 15												



Lanes, Volumes, Timings
2: Kenwood Rd & Oakdale Rd

Future PM
5/18/2007

Lane Group	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations			↑↑↑↑					
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	0	1770	5085	1583	0	1770	1583	0
Flt Permitted		0.065				0.950		
Satd. Flow (perm)	0	121	5085	1583	0	1770	1583	0
Satd. Flow (RTOR)				34			9	
Volume (vph)	2	131	1261	33	11	145	11	8
Lane Group Flow (vph)	0	136	1287	34	0	177	21	0
Turn Type	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm	Perm
Protected Phases	1	1	6		13			
Permitted Phases	6	6		2	13		13	
Total Split (s)	12.0	12.0	74.0	62.0	20.0	20.0	20.0	0.0
Act Effct Green (s)	70.0	70.0	70.0	58.0		16.0	16.0	
Actuated g/C Ratio	0.47	0.47	0.39	0.11		0.11	0.11	
v/c Ratio	0.94	0.54	0.05	0.05		0.94	0.12	
Control Delay	117.9	29.6	8.8			116.1	42.9	
Queue Delay	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay	117.9	29.7	8.8			116.1	42.9	
LOS	F	C	A		F	D		
Approach Delay			37.4			108.3		
Approach LOS			D			F		
Intersection Summary								

HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Future PM
5/18/2007

Movement	EBL2	EBL	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.94	0.98	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00
Flt Protected	1728	1770	1629	1770	1629	1770	1629	1770	1629	1770	1629	1770
Satd. Flow (prot)	0.71	0.62	1.00	0.62	1.00	0.71	0.62	1.00	0.71	0.62	1.00	0.71
Flt Permitted	1256	1153	1629	1256	1153	1629	1256	1153	1629	1256	1153	1629
Satd. Flow (perm)	37	3	26	47	490	46	225	11	107	1842	37	215
Volume (vph)	0.75	0.75	0.75	0.75	0.97	0.97	0.97	0.97	0.99	0.99	0.99	0.99
Peak-hour factor, PHF	49	4	35	63	505	47	232	11	108	1861	37	217
Adj. Flow (vph)	0	0	17	0	0	1	0	0	0	0	0	43
RTOR Reduction (vph)	0	0	134	0	505	289	0	0	108	1898	0	174
Lane Group Flow (vph)	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Turn Type	8	8	8	8	4	4	4	4	2	2	2	6
Protected Phases	8	8	8	8	4	4	4	4	2	2	2	6
Permitted Phases	52.0	52.0	52.0	52.0	52.0	52.0	52.0	52.0	58.0	58.0	58.0	70.0
Actuated Green, G (s)	52.0	52.0	52.0	52.0	52.0	52.0	52.0	52.0	58.0	58.0	58.0	70.0
Effective Green, g (s)	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.35	0.39	0.39	0.39	0.47
Actuated g/C Ratio	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	435	400	565	400	565	400	565	400	85	1960	400	739
Lane Grp Cap (vph)	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot	v/s Ratio Prot
v/s Ratio Prot	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.49	0.37	0.49	0.11
v/c Ratio	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	1.27	0.97	1.27	0.24
Uniform Delay, d1	35.8	35.8	35.8	35.8	35.8	35.8	35.8	35.8	46.0	45.1	46.0	24.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.28	0.28	0.28	0.13
Incremental Delay, d2	0.4	0.4	0.4	0.4	0.4	0.4	0.4	0.4	130.1	2.2	130.1	0.1
Delay (s)	36.2	36.2	36.2	36.2	36.2	36.2	36.2	36.2	143.1	14.7	143.1	3.2
Level of Service	D	D	D	D	D	D	D	D	F	B	F	A
Approach Delay (s)	36.2	36.2	36.2	36.2	36.2	36.2	36.2	36.2	132.5	19.8	132.5	3.2
Approach LOS	D	D	D	D	D	D	D	D	F	B	F	A
Intersection Summary												
HCM Average Control Delay	48.2	48.2	48.2	48.2	48.2	48.2	48.2	48.2	HCM Level of Service	D	D	D
HCM Volume to Capacity ratio	1.18	1.18	1.18	1.18	1.18	1.18	1.18	1.18	1.18	1.18	1.18	1.18
Actuated Cycle Length (s)	150.0	150.0	150.0	150.0	150.0	150.0	150.0	150.0	Sum of lost time (s)	12.0	12.0	12.0
Intersection Capacity Utilization	102.7%	102.7%	102.7%	102.7%	102.7%	102.7%	102.7%	102.7%	ICU Level of Service	G	G	G
Analysis Period (min)	15	15	15	15	15	15	15	15	15	15	15	15
Critical Lane Group												

Baseline
A & R Engineering Inc.

Synchro 6 Report
Page 5

HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Future PM
5/18/2007

Movement	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	0.95	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1770	1770	1583	1583	1770	1770	1583	1583
Flt Permitted	0.06	0.06	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	120	120	5085	5085	1770	1770	1583	1583
Volume (vph)	2	131	1261	33	11	145	11	8
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.88	0.88	0.88	0.88
Adj. Flow (vph)	2	134	1287	34	12	165	12	9
RTOR Reduction (vph)	0	0	0	21	0	0	0	8
Lane Group Flow (vph)	0	136	1287	13	0	177	13	0
Turn Type	pm+pt	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm
Protected Phases	1	1	6	2	13	13	13	13
Permitted Phases	6	6	6	2	13	13	13	13
Actuated Green, G (s)	70.0	70.0	70.0	58.0	16.0	16.0	16.0	16.0
Effective Green, g (s)	70.0	70.0	70.0	58.0	16.0	16.0	16.0	16.0
Actuated g/C Ratio	0.47	0.47	0.47	0.39	0.11	0.11	0.11	0.11
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	144	2373	612	189	169	169	169	169
v/s Ratio Prot	c0.05	0.25	0.01	0.01	0.10	0.10	0.10	0.10
v/c Ratio	0.39	0.39	0.54	0.02	0.94	0.94	0.94	0.94
Uniform Delay, d1	65.5	28.6	28.4	28.4	66.5	66.5	66.5	66.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	57.9	0.9	0.1	0.1	50.7	50.7	50.7	50.7
Delay (s)	123.4	29.5	28.5	28.5	117.2	117.2	117.2	117.2
Level of Service	F	C	C	C	F	F	F	F
Approach Delay (s)	38.2	38.2	38.2	38.2	111.2	111.2	111.2	111.2
Approach LOS	D	D	D	D	F	F	F	F
Intersection Summary								

Baseline
A & R Engineering Inc.

Synchro 6 Report
Page 6

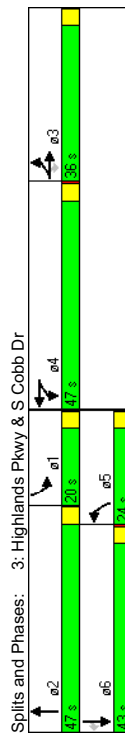
Lanes, Volumes, Timings
3: Highlands Pkwy & S Cobb Dr

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1681	1709	1583	0	1736	0	3433	4999	0	1770	5085	1583
Flt Permitted	0.950	0.966	0.975	0.975	0.975	0.975	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (perm)	1681	1709	1583	0	1736	0	3433	4999	0	1770	5085	1583
Satd. Flow (RTOR)	290	290	290	16	16	16	15	15	15	15	15	15
Volume (vph)	680	86	497	277	89	157	520	1416	155	176	1079	561
Lane Group Flow (vph)	414	435	529	0	641	0	559	1649	0	226	1112	616
Turn Type	Split	Perm	Split	Split	Split	Split	Prot	Prot	Prot	Prot	Prot	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	6
Permitted Phases	36.0	36.0	36.0	47.0	47.0	47.0	0.0	24.0	47.0	0.0	20.0	43.0
Total Split (s)	32.0	32.0	32.0	43.0	43.0	43.0	20.0	43.0	16.0	39.0	39.0	39.0
Act Effct Green (s)	0.21	0.21	0.21	0.29	0.29	0.29	0.13	0.29	0.11	0.26	0.26	0.26
Actuated g/C Ratio	1.15	1.19	0.93	1.26	1.26	1.26	1.22	1.14	1.20	0.84	1.15	1.15
Control Delay	147.0	159.7	50.6	174.3	174.3	174.3	165.8	120.1	166.1	41.3	105.7	105.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	69.4	0.0	0.6	12.2	12.2
Total Delay	147.0	159.7	50.6	174.3	174.3	174.3	165.8	189.4	166.1	41.9	117.9	117.9
LOS	F	F	D	F	F	F	F	F	F	D	F	F
Approach Delay	114.0			174.3			183.5					
Approach LOS	F			F			F					

A-64

Cycle Length: 150
Actuated Cycle Length: 150
Offset: 120 (80%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.26
Intersection Signal Delay: 134.4
Intersection Capacity Utilization 91.2%
Analysis Period (min) 15



Splits and Phases: 3: Highlands Pkwy & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
3: Highlands Pkwy & S Cobb Dr

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	1.00	1.00	0.97	0.91	1.00	0.98	1.00	0.91	1.00	0.85
Flt Protected	0.95	0.97	1.00	0.98	0.98	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1681	1709	1583	1737	1737	1737	3433	4998	1770	5085	1583	1583
Flt Permitted	0.95	0.97	1.00	0.98	0.98	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	1681	1709	1583	1737	1737	1737	3433	4998	1770	5085	1583	1583
Volume (vph)	680	86	497	277	89	157	520	1416	155	176	1079	561
Peak-hour factor, PHF	0.94	0.68	0.94	0.86	0.81	0.75	0.93	0.97	0.82	0.78	0.97	0.91
Adj. Flow (vph)	723	126	529	322	110	209	559	1460	189	226	1112	616
RTOR Reduction (vph)	0	0	228	0	11	0	0	11	0	0	0	124
Lane Group Flow (vph)	414	435	501	0	630	0	559	1638	0	226	1112	492
Turn Type	Split	Perm	Split	Split	Split	Split	Prot	Prot	Prot	Prot	Prot	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	6
Permitted Phases	32.0	32.0	32.0	43.0	43.0	43.0	20.0	43.0	16.0	39.0	39.0	39.0
Effective Green, g (s)	32.0	32.0	32.0	43.0	43.0	43.0	20.0	43.0	16.0	39.0	39.0	39.0
Actuated g/C Ratio	0.21	0.21	0.21	0.29	0.29	0.29	0.13	0.29	0.11	0.26	0.26	0.26
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	359	365	338	498	498	498	1458	1433	189	1322	412	412
v/s Ratio Prot	0.25	0.25	0.19	0.36	0.36	0.36	0.16	0.33	0.13	0.22	0.22	0.22
v/s Ratio Perm	1.15	1.19	0.89	1.26	1.26	1.26	1.22	1.14	1.20	0.84	1.20	1.20
Uniform Delay, d1	59.0	59.0	57.3	53.5	53.5	53.5	65.0	53.5	67.0	52.6	55.5	55.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.03	1.07	0.89	0.70	0.58	0.58
Incremental Delay, d2	96.0	110.3	24.0	134.2	134.2	134.2	112.4	70.7	116.4	4.4	102.6	102.6
Delay (s)	155.0	169.3	81.2	187.7	187.7	187.7	179.1	128.1	176.1	41.1	134.5	134.5
Level of Service	F	F	F	F	F	F	F	F	F	D	F	F
Approach Delay (s)	131.2			187.7			141.0					
Approach LOS	F			F			F					

Intersection Summary
HCM Average Control Delay
HCM Volume to Capacity ratio
Actuated Cycle Length (s)
Intersection Capacity Utilization
Analysis Period (min)
c Critical Lane Group

Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

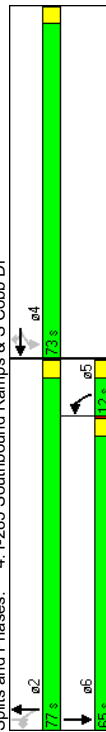
Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	0	1681	1441	1504	1770	3539	0	0	3539
Satd. Flow (prot)	0	0	0	0.950	0.950	0.950	0.063	0.063	0	0	0.063
Fit Permitted	0	0	0	1681	1441	1504	117	3539	0	0	3539
Satd. Flow (RTOR)	0	0	0	436	0	1031	71	1348	0	0	1179
Volume (vph)	0	0	0	436	0	1031	71	1348	0	0	1179
Satd. Flow (prot)	0	0	0	459	573	573	85	1390	0	0	1241
Lane Group Flow (vph)	0	0	0	459	573	573	85	1390	0	0	1241
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	0.0	0.0	0.0	73.0	73.0	73.0	12.0	77.0	0.0	0.0	65.0
Total Split (s)	0.0	0.0	0.0	62.3	62.3	62.3	79.7	79.7	0.0	0.0	67.7
Act Effct Green (s)	0.0	0.0	0.0	62.3	62.3	62.3	79.7	79.7	0.0	0.0	67.7
Actuated g/C Ratio	0.42	0.42	0.42	0.42	0.42	0.42	0.53	0.53	0.45	0.45	0.53
v/c Ratio	0.66	0.94	0.90	0.57	0.74	0.66	0.94	0.90	0.78	0.78	0.76
Control Delay	39.4	63.5	56.7	16.3	2.4	39.4	63.5	56.7	18.0	18.0	8.1
Queue Delay	0.3	0.0	0.0	0.0	0.0	0.3	0.0	0.0	0.7	0.7	0.0
Total Delay	39.7	63.5	56.7	16.3	3.1	39.7	63.5	56.7	18.7	18.7	8.1
LOS	D	E	E	B	A	D	E	E	B	B	A
Approach Delay	54.3					54.3					14.3
Approach LOS	D					D			A		B

A-65

Cycle Length: 150
Actuated Cycle Length: 150
Offset: 117 (78%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.94
Intersection Signal Delay: 23.6
Intersection Capacity Utilization 144.2%
Analysis Period (min) 15

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
4: I-285 Southbound Ramps & S Cobb Dr

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.91	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.85
Fit Protected	1.00	0.85	1.00	1.00	0.95	1.00	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1681	1441	1504	1770	3539	1681	1441	1504	1770	3539	1583
Fit Permitted	0.95	1.00	1.00	0.09	1.00	0.95	1.00	1.00	0.09	1.00	1.00
Satd. Flow (perm)	1681	1441	1504	169	3539	1681	1441	1504	169	3539	1583
Volume (vph)	0	0	0	436	0	1031	71	1348	0	0	1179
Peak-hour factor, PHF	0.92	0.92	0.92	0.95	0.92	0.90	0.84	0.97	0.92	0.92	0.95
Adj. Flow (vph)	0	0	0	459	0	1146	85	1390	0	0	1241
RTOR Reduction (vph)	0	0	0	0	13	13	0	0	0	0	0
Lane Group Flow (vph)	0	0	0	459	560	560	85	1390	0	0	1241
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	62.3	62.3	62.3	79.7	79.7	62.3	79.7	79.7	62.3	62.3	79.7
Actuated Green, G (s)	62.3	62.3	62.3	79.7	79.7	62.3	79.7	79.7	62.3	62.3	79.7
Effective Green, g (s)	0.42	0.42	0.42	0.42	0.53	0.53	0.53	0.53	0.45	0.45	0.53
Actuated g/C Ratio	0.42	0.42	0.42	0.42	0.42	0.42	0.53	0.53	0.45	0.45	0.53
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	698	598	625	175	1880	698	598	625	175	1880	841
v/s Ratio Prot	0.27	0.39	0.37	0.23	0.23	0.27	0.39	0.37	0.23	0.23	0.35
v/s Ratio Perm	0.66	0.94	0.90	0.49	0.74	0.66	0.94	0.90	0.74	0.74	0.67
Uniform Delay, d1	35.3	42.0	40.8	53.5	27.1	35.3	42.0	40.8	53.5	27.1	34.8
Progression Factor	1.00	1.00	1.00	0.31	0.07	1.00	1.00	1.00	0.31	0.07	0.43
Incremental Delay, d2	2.2	22.2	15.4	0.2	0.2	2.2	22.2	15.4	0.2	0.2	2.1
Delay (s)	37.5	64.2	56.2	16.9	2.2	37.5	64.2	56.2	16.9	2.2	17.1
Level of Service	D	E	E	B	A	D	E	E	B	A	C
Approach Delay (s)	0.0	0.0	0.0	53.7	3.1	0.0	53.7	3.1	0.0	0.0	20.5
Approach LOS	A			D		A					C
Intersection Summary											
HCM Average Control Delay				25.8							C
HCM Volume to Capacity ratio				0.84							
Actuated Cycle Length (s)				150.0							8.0
Intersection Capacity Utilization				144.2%							H
Analysis Period (min)				15							
c Critical Lane Group											

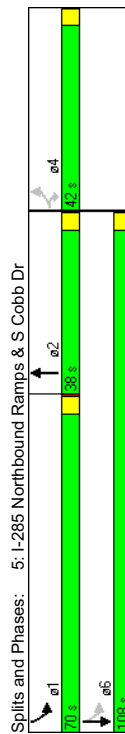
Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	0	3341	0	1770	3539	0
Flt Permitted	0.950							0.105				
Satd. Flow (perm)	3433	0	1583	0	0	0	0	3341	0	196	3539	0
Satd. Flow (RTOR)												
Volume (vph)	921	0	80	0	0	0	0	518	281	862	720	0
Lane Group Flow (vph)	940	0	94	0	0	0	0	888	0	917	774	0
Turn Type	custom	custom	custom	custom	custom	custom	custom	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt
Protected Phases	4		4				2		1		6	
Permitted Phases												
Total Split (s)	42.0	0.0	42.0	0.0	0.0	0.0	0.0	38.0	0.0	70.0	108.0	0.0
Act Effct Green (s)	38.0		38.0				34.0		34.0	104.0	104.0	
Actuated g/C Ratio	0.25		0.25				0.23		0.23	0.69	0.69	
Control Delay	106.4		106.4				106.8		106.8	100.1	16.6	
Queue Delay	0.0		0.0				0.0		0.0	145.2	0.7	
Total Delay	106.4		106.4				106.8		106.8	245.3	17.3	
LOS	F		B				F		F	F	B	
Approach Delay							106.8				141.0	
Approach LOS							F		F		F	

A-66

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 101 (67%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.11												
Intersection Signal Delay: 120.2												
Intersection Capacity Utilization 144.2%												
Analysis Period (min) 15												



Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	1.00	0.85	1.00	0.95	1.00	0.95	1.00	1.00	0.95	
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	3433	1583	1583	1583	1583	1583	3341	1770	3539	1770	3539	
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.11	1.00	0.11	1.00	
Satd. Flow (perm)	3433	1583	1583	1583	1583	1583	3341	196	3539	196	3539	
Volume (vph)	921	0	80	0	0	0	0	518	281	862	720	0
Peak-hour factor, PHF	0.98	0.92	0.85	0.92	0.92	0.92	0.93	0.85	0.94	0.93	0.92	
Adj. Flow (vph)	940	0	94	0	0	0	0	557	331	917	774	0
RTOR Reduction (vph)	0	0	66	0	0	0	0	59	0	0	0	0
Lane Group Flow (vph)	940	0	28	0	0	0	0	829	0	917	774	0
Turn Type	custom	custom	custom	custom	custom	custom	custom	pm+pt	pm+pt	pm+pt	pm+pt	pm+pt
Protected Phases	4		4				2		1		6	
Permitted Phases												
Actuated Green, G (s)	38.0		38.0				34.0		34.0	104.0	104.0	
Effective Green, g (s)	38.0		38.0				34.0		34.0	104.0	104.0	
Actuated g/C Ratio	0.25		0.25				0.23		0.23	0.69	0.69	
Clearance Time (s)	4.0		4.0				4.0		4.0	4.0	4.0	
Vehicle Extension (s)	3.0		3.0				3.0		3.0	3.0	3.0	
Lane Grp Cap (vph)	870		401				757		828	2454		
v/s Ratio Prot	c0.27		0.02				0.25		c0.49	0.22		
v/s Ratio Perm	1.08		0.07				1.10		c0.28			
Uniform Delay, d1	56.0		42.6				58.0		44.2	9.0		
Progression Factor	1.00		1.00				1.00		1.20	1.80		
Incremental Delay, d2	54.6		0.1				61.9		60.0	0.2		
Delay (s)	110.6		42.6				119.9		113.0	16.5		
Level of Service	F		D				F		F	B		
Approach Delay (s)	104.4					0.0	119.9			68.8		
Approach LOS	F					A	F			E		
Intersection Summary												
HCM Average Control Delay	91.6											
HCM Volume to Capacity ratio	1.09											
Actuated Cycle Length (s)	150.0											
Intersection Capacity Utilization	144.2%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
6: Church Rd & N Church Lane

Future PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1814	0	0	1837	0	0	1767	0	0	1674	0
Flt Permitted	0	0.976	0	0	0.983	0	0	0.983	0	0	0.983	0
Satd. Flow (perm)	0	1814	0	0	1837	0	0	1767	0	0	1674	0
Volume (vph)	99	112	1	0	348	36	8	10	6	110	2	185
Lane Group Flow (vph)	0	242	0	0	416	0	0	46	0	0	349	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	66.0%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
6: Church Rd & N Church Lane

Future PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	99	112	1	0	348	36	8	10	6	110	2	185
Peak Hour Factor	0.84	0.93	0.25	0.92	0.93	0.85	0.50	0.56	0.50	0.90	0.50	0.83
Hourly flow rate (vph)	118	120	4	0	374	42	16	18	12	122	4	223
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				None
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	374			124			978	732	122	774	756	395
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	374			124			978	732	122	774	756	395
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	90			100			88	94	99	56	99	66
cM capacity (veh/h)	1184			1462			138	313	929	275	304	654
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	242	417	46	349								
Volume Left	118	0	16	122								
Volume Right	4	42	12	223								
cSH	1184	1462	247	437								
Volume to Capacity	0.10	0.00	0.19	0.80								
Queue Length 95th (ft)	8	0	17	180								
Control Delay (s)	4.5	0.0	22.9	38.6								
Lane LOS	A	C	C	E								
Approach Delay (s)	4.5	0.0	22.9	38.6								
Approach LOS	C	C	C	E								
Intersection Summary												
Average Delay	14.8											
Intersection Capacity Utilization	66.0%											
Analysis Period (min)	15											

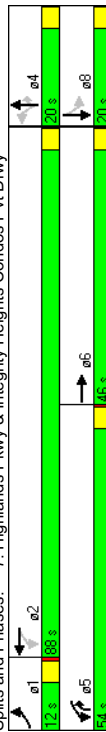
Lanes, Volumes, Timings
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3415	0	1770	3511	0	1770	1863	1583	0	1789	0
Flt Permitted	0.950	0.087		0.087	0.714		0.087	0.714	0.873		0.873	
Satd. Flow (perm)	1770	3415	0	162	3511	0	1330	1863	1583	0	1599	0
Satd. Flow (RTOR)	38	11		11	37		37	15	5		5	
Volume (vph)	5	807	236	726	467	14	82	2	456	21	17	3
Lane Group Flow (vph)	8	1148	0	764	518	0	111	4	470	0	63	0
Turn Type	Prot	pm+pt	Perm	pm+pt	Perm	pm+ov	Perm	pm+ov	Perm	pm+ov	Perm	pm+ov
Protected Phases	1	6	5	2	4	5	4	5	4	5	8	
Permitted Phases												
Total Split (s)	12.0	46.0	0.0	54.0	88.0	0.0	20.0	20.0	54.0	20.0	20.0	0.0
Act Effct Green (s)	6.2	42.3	93.7	91.5	13.4	13.4	64.7	13.4	64.7	13.4	13.4	
Actuated g/C Ratio	0.05	0.37	0.81	0.79	0.12	0.12	0.12	0.12	0.56	0.12	0.12	
Control Delay	0.09	0.90	0.96	0.19	0.72	0.02	0.52	0.33	0.33	0.33	0.33	
Queue Delay	58.4	44.7	52.1	3.5	75.2	45.5	16.3	49.0	49.0	49.0	49.0	
Total Delay	58.4	44.7	52.1	3.5	75.2	45.5	16.3	49.0	49.0	49.0	49.0	
LOS	E	D	D	A	C	C	D	B	B	D	D	
Approach Delay	44.8			32.5			27.6					
Approach LOS	D			C			C					

Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 115.1												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.96												
Intersection Signal Delay: 36.5												
Intersection Capacity Utilization 89.0%												
Analysis Period (min) 15												

Splits and Phases: 7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy



HCM Signalized Intersection Capacity Analysis
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3415	0	1770	3511	0	1770	1863	1583	0	1789	0
Flt Permitted	0.950	0.087		0.087	0.714		0.087	0.714	0.873		0.873	
Satd. Flow (perm)	1770	3415	0	162	3511	0	1330	1863	1583	0	1599	0
Satd. Flow (RTOR)	38	11		11	37		37	15	5		5	
Volume (vph)	5	807	236	726	467	14	82	2	456	21	17	3
Lane Group Flow (vph)	8	1148	0	764	518	0	111	4	470	0	63	0
Turn Type	Prot	pm+pt	Perm	pm+pt	Perm	pm+ov	Perm	pm+ov	Perm	pm+ov	Perm	pm+ov
Protected Phases	1	6	5	2	4	5	4	5	4	5	8	
Permitted Phases												
Actuated Green, G (s)	1.4	45.5	96.9	91.5	13.4	13.4	60.8	13.4	60.8	13.4	13.4	
Effective Green, g (s)	1.4	45.5	96.9	91.5	13.4	13.4	60.8	13.4	60.8	13.4	13.4	
Actuated g/C Ratio	0.01	0.38	0.82	0.77	0.11	0.11	0.11	0.51	0.11	0.51	0.11	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	21	1313	772	2717	150	211	867	180	180	180	180	
v/s Ratio Prot	0.00	0.33	c0.40	0.15	c0.08	0.00	0.21	0.00	0.21	0.00	0.21	
v/s Ratio Perm	0.38	0.86	0.99	0.19	0.74	0.02	0.52	0.08	0.52	0.08	0.52	
Uniform Delay, d1	58.0	33.4	31.0	3.6	50.8	46.6	19.1	48.3	48.3	48.3	48.3	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	11.2	5.7	29.5	0.2	17.7	0.0	0.6	1.1	1.1	1.1	1.1	
Delay (s)	69.2	39.1	60.4	3.7	68.4	46.6	19.7	49.3	49.3	49.3	49.3	
Level of Service	E	D	E	A	D	D	B	D	B	D	D	
Approach Delay (s)	39.3			37.5			29.1				49.3	
Approach LOS	D			D			C				D	
Intersection Summary												
HCM Average Control Delay	36.8											
HCM Volume to Capacity ratio	0.95											
Actuated Cycle Length (s)	118.3											
Intersection Capacity Utilization	89.0%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
8: US 278 (US 78) & Oakdale Rd

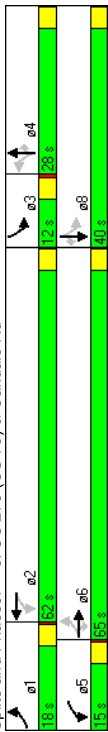
Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3497	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.066			0.424			0.416			0.465		
Satd. Flow (perm)	123	3539	1583	790	3497	0	775	1863	1583	866	1863	1583
Satd. Flow (RTOR)				80			11			119		190
Volume (vph)	187	489	72	220	1319	115	80	133	100	116	339	498
Lane Group Flow (vph)	203	532	80	239	1562	0	94	145	119	138	368	530
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt
Protected Phases	1	6	6	5	2		4	4		3	8	
Permitted Phases	6			2			4			4	8	
Total Split (s)	18.0	65.0	65.0	15.0	62.0	0.0	28.0	28.0	28.0	12.0	40.0	40.0
Act Effct Green (s)	73.4	61.3	61.3	69.5	59.3		19.3	19.3	19.3	31.3	31.3	31.3
Actuated g/C Ratio	0.64	0.53	0.53	0.61	0.52		0.17	0.17	0.17	0.27	0.27	0.27
Control Delay	0.80	0.28	0.09	0.42	0.86		0.72	0.46	0.33	0.46	0.72	0.93
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.3	16.1	3.6	11.3	31.8		74.8	47.7	9.8	38.1	46.7	50.1
LOS	D	B	A	B	C		E	D	A	D	D	D
Approach Delay		23.4			29.1			42.2			47.3	
Approach LOS		C			C			D			D	

A-69

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 114.8	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.93	
Intersection Signal Delay: 33.8	Intersection LOS: C
Intersection Capacity Utilization 86.1%	ICU Level of Service E
Analysis Period (min) 15	

Splits and Phases: 8: US 278 (US 78) & Oakdale Rd



HCM Signalized Intersection Capacity Analysis
8: US 278 (US 78) & Oakdale Rd

Future PM
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.99	1.00	1.00	0.85	1.00	1.00	0.85
Satd. Flow (prot)	1770	3539	1583	1770	3496		1770	1863	1583	1770	1863	1583
Flt Permitted	0.07	1.00	1.00	0.42	1.00		0.39	1.00	1.00	0.43	1.00	1.00
Satd. Flow (perm)	122	3539	1583	791	3496		724	1863	1583	793	1863	1583
Volume (vph)	187	489	72	220	1319	115	80	133	100	116	339	498
Peak-hour factor, PHF	0.92	0.92	0.90	0.92	0.92	0.90	0.85	0.92	0.84	0.84	0.92	0.94
Adj. Flow (vph)	203	532	80	239	1434	128	94	145	119	138	368	530
RTOR Reduction (vph)	0	0	37	0	5	0	0	0	99	0	0	138
Lane Group Flow (vph)	203	532	43	239	1557	0	94	145	20	138	368	392
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt
Protected Phases	1	6	6	5	2		4	4		3	8	
Permitted Phases	6			2			4			4	8	
Actuated Green, G (s)	73.5	61.3	61.3	69.5	59.3		19.3	19.3	19.3	31.3	31.3	31.3
Effective Green, g (s)	73.5	61.3	61.3	69.5	59.3		19.3	19.3	19.3	31.3	31.3	31.3
Actuated g/C Ratio	0.64	0.53	0.53	0.61	0.52		0.17	0.17	0.17	0.27	0.27	0.27
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	253	1890	845	566	1806		122	313	266	284	508	432
v/s Ratio Prot	c0.09	0.15		0.04	c0.45		0.08		0.03	0.20		
v/s Ratio Perm	0.43		0.03	0.22			0.13		0.01	0.10		c0.25
v/c Ratio	0.80	0.28	0.05	0.42	0.86		0.77	0.46	0.08	0.49	0.72	0.91
Uniform Delay, d1	32.4	14.7	12.8	10.4	24.2		45.6	43.1	40.2	33.3	37.8	40.3
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	16.6	0.4	0.1	0.5	5.7		25.4	1.1	0.1	1.3	5.1	22.3
Delay (s)	49.0	15.0	12.9	10.9	29.9		71.0	44.2	40.4	34.6	42.9	62.6
Level of Service	D	B	B	B	C		E	D	D	C	D	E
Approach Delay (s)		23.3			27.4			49.9			51.9	
Approach LOS		C			C			D			D	
Intersection Summary												
HCM Average Control Delay	34.9 HCM Level of Service C											
HCM Volume to Capacity ratio	0.90											
Actuated Cycle Length (s)	114.8 Sum of lost time (s)											
Intersection Capacity Utilization	86.1% ICU Level of Service E											
Analysis Period (min)	15											
Critical Lane Group												

Lanes, Volumes, Timings
9: Oak Dr & S Cobb Dr

Future PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔				↔	↔	↔	↔	↔	↔	↔
Satd. Flow (prot)	0	1617	0	0	0	1611	1770	5085	1583	0	5085	0
Flt Permitted		0.998					0.950					
Satd. Flow (perm)	0	1617	0	0	0	1611	1770	5085	1583	0	5085	0
Volume (vph)	8	0	260	0	0	143	437	1893	128	0	1786	46
Lane Group Flow (vph)	0	298	0	0	0	170	465	1932	144	0	1935	0
Sign Control		Stop			Yield		Free				Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 86.3%	ICU Level of Service E											
Analysis Period (min) 15												

HCM Unsignalized Intersection Capacity Analysis
9: Oak Dr & S Cobb Dr

Future PM
5/18/2007

Intersection has too many lanes per leg.
HCM All-Way analysis is limited to two lanes per leg.
Channelized right turn lanes are not counted.

Lanes, Volumes, Timings
10: Church Rd & Groover Dr

Future PM
5/18/2007

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Satd. Flow (prot)	0	1863	1583	1770	1863	0	0	1752	0	0	1611	0
Flt Permitted				0.950				0.959				
Satd. Flow (perm)	0	1863	1583	1770	1863	0	0	1752	0	0	1611	0
Volume (vph)	0	229	82	20	604	0	75	0	15	0	0	3
Lane Group Flow (vph)	0	283	164	29	636	0	0	124	0	0	8	0
Sign Control	Free											
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 50.2%												
Analysis Period (min) 15												
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis
10: Church Rd & Groover Dr

Future PM
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	0	229	82	20	604	0	75	0	15	0	0	3
Peak Hour Factor	0.92	0.81	0.50	0.70	0.95	0.92	0.70	0.92	0.88	0.92	0.92	0.38
Hourly flow rate (vph)	0	283	164	29	636	0	107	0	17	0	0	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	636			447			984	976	283	993	1140	636
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	636			447			984	976	283	993	1140	636
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			97			51	100	98	100	100	98
cM capacity (veh/h)	948			1114			220	245	756	215	196	478
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2	SB 1	SB 2	SB 3	SB 4	SB 5	SB 6
Volume Total	283	164	29	636	124	8						
Volume Left	0	0	29	0	107	0						
Volume Right	0	164	0	17	8							
cSH	948	1700	1114	1700	243	478						
Volume to Capacity	0.00	0.10	0.03	0.37	0.51	0.02						
Queue Length 95th (ft)	0	0	2	0	66	1						
Control Delay (s)	0.0	0.0	8.3	0.0	34.3	12.7						
Lane LOS	A	A	A	D	B	B						
Approach Delay (s)	0.0	0.4		34.3	12.7							
Approach LOS	D	D		D	B							
Intersection Summary												
Average Delay				3.7								
Intersection Capacity Utilization				50.2%						A		
Analysis Period (min)				15								

Lanes, Volumes, Timings
11: Church Rd & North Site Dr 1

Future PM
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	1770	1863	1756	0
Satd. Flow (prot)	1863	1583	1770	1863	1756	0
Flt Permitted	1863	1583	1770	1863	1756	0
Volume (vph)	297	82	20	664	108	15
Lane Group Flow (vph)	323	89	22	722	133	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization 48.5%						
Analysis Period (min) 15						
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis
11: Church Rd & North Site Dr 1

Future PM
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	1770	1863	1756	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Volume (veh/h)	297	82	20	664	108	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	323	89	22	722	117	16
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						None
Median storage (veh)						
Upstream signal (ft)		848				
pX, platoon unblocked						
VC, conflicting volume			412		1088	323
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
vCu, unblocked vol			412		1088	323
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		50	98
cM capacity (veh/h)			1147		234	718
Direction, Lane #						
	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volume Total	323	89	22	722	134	
Volume Left	0	0	22	0	117	
Volume Right	0	89	0	0	16	
cSH	1700	1700	1147	1700	255	
Volume to Capacity	0.19	0.05	0.02	0.42	0.52	
Queue Length 95th (ft)	0	0	1	0	70	
Control Delay (s)	0.0	0.0	8.2	0.0	33.6	
Lane LOS			A		D	
Approach Delay (s)	0.0	0.2	0.2		33.6	
Approach LOS					D	
Intersection Summary						
Average Delay 3.6						
Intersection Capacity Utilization 48.5%						
ICU Level of Service A						
Analysis Period (min) 15						

Lanes, Volumes, Timings
12: Church Rd & North Site Dr 3

Future PM
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	0	1859	1736	0
Satd. Flow (prot)	1863	1583	0	1859	1736	0
Flt Permitted	1863	1583	0	1859	1736	0
Volume (vph)	169	76	25	559	66	20
Lane Group Flow (vph)	184	83	0	635	94	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization 54.6%						
Analysis Period (min) 15						
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis
12: Church Rd & North Site Dr 3

Future PM
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	0	1859	1736	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Volume (veh/h)	169	76	25	559	66	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	184	83	27	608	72	22
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						None
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
VC, conflicting volume			266		846	184
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
vCu, unblocked vol			266		846	184
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		78	97
cM capacity (veh/h)			1298		326	859
Direction, Lane #						
Volume Total	184	83	635	93		
Volume Left	0	0	27	72		
Volume Right	0	83	0	22		
cSH	1700	1700	1298	381		
Volume to Capacity	0.11	0.05	0.02	0.25		
Queue Length 95th (ft)	0	0	2	24		
Control Delay (s)	0.0	0.0	0.6	17.5		
Lane LOS			A	C		
Approach Delay (s)	0.0	0.6	17.5			
Approach LOS			C			
Intersection Summary						
Average Delay 2.0						
Intersection Capacity Utilization 54.6%						
ICU Level of Service A						
Analysis Period (min) 15						

Future 2009 PM Improved

Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

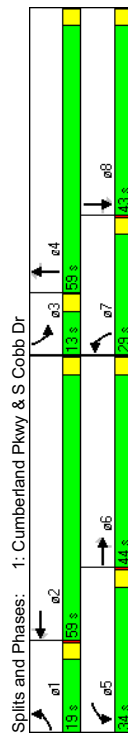
HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Future PM - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	177	907	156	205	2033	129	586	1228	174	207	954	98
Lane Group Flow (vph)	203	945	184	228	2140	143	637	1364	212	233	984	124
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases	19.0	44.0	43.0	34.0	59.0	59.0	29.0	59.0	44.0	13.0	43.0	59.0
Total Split (s)	15.0	46.7	39.0	23.3	55.0	55.0	25.0	55.0	46.7	9.0	39.0	55.0
Act Eff Green (s)	0.10	0.31	0.26	0.16	0.37	0.37	0.17	0.37	0.31	0.06	0.26	0.37
Actuated g/C Ratio	1.15	0.60	0.34	0.83	1.15	0.23	1.11	1.05	0.39	1.13	1.07	0.20
Control Delay	169.9	46.4	8.5	85.2	115.9	22.6	128.3	85.0	30.7	162.8	101.9	12.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	169.9	46.4	8.5	85.2	115.9	22.6	128.3	85.0	30.7	162.8	101.9	12.9
LOS	F	D	A	F	F	C	F	F	C	F	F	B
Approach Delay	60.0			107.8			92.3				104.3	
Approach LOS	E			F			F				F	

A-75

Intersection Summary											
Cycle Length: 150											
Control Type: Actuated-Uncoordinated											
Maximum v/c Ratio: 1.15											
Intersection Signal Delay: 93.9	Intersection LOS: F										
Intersection Capacity Utilization 105.5%	ICU Level of Service G										
Analysis Period (min) 15											



Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

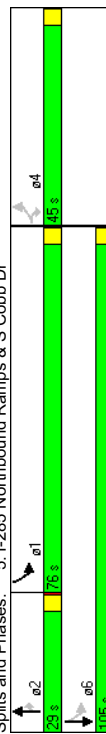
Future PM - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3539	1583	1770	3539	0	0
Flt Permitted	0.950								0.138			
Satd. Flow (perm)	3433	0	1583	0	0	0	3539	1583	257	3539	0	0
Satd. Flow (RTOR)	91								228			
Volume (vph)	921	0	80	0	0	0	518	281	862	720	0	0
Lane Group Flow (vph)	940	0	94	0	0	0	557	331	917	774	0	0
Turn Type	custom	custom	custom						Perm pm+pt			
Protected Phases	4		4				2		2	1	6	
Permitted Phases	4		4				2		2	6		
Total Split (s)	45.0	0.0	45.0	0.0	0.0	0.0	29.0	29.0	76.0	105.0	0.0	0.0
Act Effct Green (s)	41.0		41.0				25.0	25.0	101.0	101.0		
Actuated g/C Ratio	0.27	0.27	0.27				0.17	0.17	0.67	0.67		
Control Delay	83.7	9.1	86.9	28.6	70.3	7.8	0.0	0.0	34.9	0.8		
Queue Delay	0.0		0.0				0.0	0.0	34.9	0.8		
Total Delay	83.7	9.1	86.9	28.6	105.2	8.6						
LOS	F	A	F	C	F	A						
Approach Delay												
Approach LOS												

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Cycle Length: 150
Actuated Cycle Length: 150
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.02
Intersection Signal Delay: 66.6
Intersection Capacity Utilization 138.3%
Analysis Period (min) 15

Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

Future PM - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.85	1.00	1.00	1.00	0.95	1.00	1.00	1.00	0.95	
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	3433	1583		3539	1583	1770	3539	1583	1770	3539		
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.14	1.00		
Satd. Flow (perm)	3433	1583		3539	1583	257	3539	1583	257	3539		
Volume (vph)	921	0	80	0	0	0	518	281	862	720	0	0
Peak-hour factor, PHF	0.98	0.92	0.85	0.92	0.92	0.92	0.93	0.85	0.94	0.93	0.92	
Adj. Flow (vph)	940	0	94	0	0	0	557	331	917	774	0	0
RTOR Reduction (vph)	0	0	66	0	0	0	0	0	190	0	0	0
Lane Group Flow (vph)	940	0	28	0	0	0	557	141	917	774	0	0
Turn Type	custom	custom	custom						Perm pm+pt			
Protected Phases	4		4				2		2	1	6	
Permitted Phases	4		4				2		2	6		
Actuated Green, G (s)	41.0	41.0	41.0				25.0	25.0	101.0	101.0		
Effective Green, g (s)	41.0	41.0	41.0				25.0	25.0	101.0	101.0		
Actuated g/C Ratio	0.27	0.27	0.27				0.17	0.17	0.67	0.67		
Clearance Time (s)	4.0	4.0	4.0				4.0	4.0	4.0	4.0		
Vehicle Extension (s)	3.0	3.0	3.0				3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)	938	433					590	264	899	2383		
v/s Ratio Prot							0.16		0.09	0.22		
v/s Ratio Perm	c0.27	0.02	0.06				0.94	0.53	1.02	0.32		
v/c Ratio	1.00	0.06	0.40.3				61.8	57.2	34.8	10.2		
Uniform Delay, d1	54.5	40.3	40.3				1.00	1.00	1.16	0.73		
Progression Factor	1.00	1.00	1.00				25.6	7.5	29.5	0.2		
Incremental Delay, d2	29.9	0.1	0.1				87.4	64.7	69.9	7.8		
Delay (s)	84.4	40.4	40.4				F	D	F	E	A	
Level of Service	F	D	D				F	D	F	E	A	
Approach Delay (s)		80.4			0.0			78.9		41.5		
Approach LOS		F			A			E		D		
Intersection Summary												
HCM Average Control Delay			61.8							E		
HCM Volume to Capacity ratio			1.00									
Actuated Cycle Length (s)			150.0							8.0		
Intersection Capacity Utilization			138.3%							H		
Analysis Period (min)			15									
c Critical Lane Group												

Lanes, Volumes, Timings
6: Church Rd & N Church Lane

Future PM - Improved
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1814	0	0	1837	0	0	1767	0	0	1777	1583
Flt Permitted	0	0.976	0	0	0.983	0	0	0.983	0	0	0.954	0.954
Satd. Flow (perm)	0	1814	0	0	1837	0	0	1767	0	0	1777	1583
Volume (vph)	99	112	1	0	348	36	8	10	6	110	2	185
Lane Group Flow (vph)	0	242	0	0	416	0	0	46	0	0	126	223
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	54.8%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
6: Church Rd & N Church Lane

Future PM - Improved
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	99	112	1	0	348	36	8	10	6	110	2	185
Peak Hour Factor	0.84	0.93	0.25	0.92	0.93	0.85	0.50	0.56	0.50	0.90	0.50	0.83
Hourly flow rate (vph)	118	120	4	0	374	42	16	18	12	122	4	223
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	374			124			867	732	122	774	756	395
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	374			124			867	732	122	774	756	395
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	90			100			90	94	99	56	99	66
cM capacity (veh/h)	1184			1462			165	313	929	275	304	654
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	242	417	46	349								
Volume Left	118	0	16	122								
Volume Right	4	42	12	223								
cSH	1184	1462	275	764								
Volume to Capacity	0.10	0.00	0.17	0.46								
Queue Length 95th (ft)	8	0	15	60								
Control Delay (s)	4.5	0.0	20.7	18.8								
Lane LOS	A	C	C	C								
Approach Delay (s)	4.5	0.0	20.7	18.8								
Approach LOS	C	C	C	C								
Intersection Summary												
Average Delay	8.2											
Intersection Capacity Utilization	54.8%											
Analysis Period (min)	15											

Future 2009 SAT

Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	307			90			103			103		96
Volume (vph)	136	843	292	221	854	149	388	1008	200	249	1019	125
Lane Group Flow (vph)	162	878	336	257	1005	157	417	1072	222	277	1073	149
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases												
Total Split (s)	17.0	38.0	47.0	24.0	45.0	51.0	21.0	51.0	38.0	17.0	47.0	45.0
Act Effct Green (s)	13.0	34.1	41.9	19.9	41.0	46.2	17.0	46.2	34.1	12.7	41.9	41.0
Actuated g/C Ratio	0.10	0.26	0.33	0.15	0.32	0.36	0.13	0.36	0.26	0.10	0.33	0.32
v/c Ratio	0.91	0.94	0.47	0.94	0.89	0.25	0.92	0.85	0.45	0.82	0.93	0.26
Control Delay	104.2	64.2	7.2	95.7	53.2	13.9	82.0	45.6	24.2	76.7	57.0	14.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	104.2	64.2	7.2	95.7	53.2	13.9	82.0	45.6	24.2	76.7	57.0	14.0
LOS	F	E	A	F	D	B	F	D	C	E	E	B
Approach Delay												
Approach LOS												

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Intersection Summary	
Cycle Length: 130	
Actuated Cycle Length: 128.9	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.94	
Intersection Signal Delay: 54.8	Intersection LOS: D
Intersection Capacity Utilization 88.1%	ICU Level of Service E
Analysis Period (min) 15	

Splits and Phases: 1: Cumberland Pkwy & S Cobb Dr



Baseline
A & R Engineering Inc.

Synchro 6 Report
Page 1

HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.95	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3539	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	136	843	292	221	854	149	388	1008	200	249	1019	125
Peak-hour factor, PHF	0.84	0.96	0.87	0.86	0.85	0.95	0.93	0.94	0.90	0.90	0.95	0.84
Adj. Flow (vph)	162	878	336	257	1005	157	417	1072	222	277	1073	149
RTOR Reduction (vph)	0	0	207	0	0	58	0	0	76	0	0	65
Lane Group Flow (vph)	162	878	129	257	1005	99	417	1072	146	277	1073	84
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases												
Actuated Green, G (s)	13.0	34.1	41.9	19.9	41.0	46.2	17.0	46.2	34.1	12.7	41.9	41.0
Effective Green, g (s)	13.0	34.1	41.9	19.9	41.0	46.2	17.0	46.2	34.1	12.7	41.9	41.0
Actuated g/C Ratio	0.10	0.26	0.33	0.15	0.32	0.36	0.13	0.36	0.26	0.10	0.33	0.32
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	179	936	515	273	1126	567	453	1268	419	338	1150	504
v/s Ratio Prot	0.09	0.25	0.08	c0.15	c0.28	c0.12	0.30		0.08	c0.30		
v/s Ratio Perm	0.91	0.94	0.25	0.94	0.89	0.18	0.92	0.85	0.35	0.82	0.93	0.17
Uniform Delay, d1	57.3	46.4	32.0	53.9	41.9	28.3	55.3	38.1	38.4	57.0	42.1	31.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	41.1	17.8	0.3	38.8	10.8	0.1	24.0	5.4	2.3	14.3	13.4	0.7
Delay (s)	98.4	64.1	32.2	92.7	52.7	28.5	79.3	43.4	40.7	71.3	55.5	32.3
Level of Service	F	E	C	F	D	C	E	D	D	E	E	C
Approach Delay (s)												
Approach LOS												

Intersection Summary	
HCM Average Control Delay	56.1
HCM Volume to Capacity ratio	0.91
Actuated Cycle Length (s)	128.9
Intersection Capacity Utilization	88.1%
Analysis Period (min)	15
Critical Lane Group	

Baseline
A & R Engineering Inc.

Synchro 6 Report
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HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Future SAT
5/18/2007

Movement	EBL2	EBL	EBT	EBR	WBL	WBT	WBR	WBR2	NBL	NBT	NBR	NBR2
Lane Configurations		↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Flt Protected	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Satd. Flow (prot)	1713	1713	1713	1713	1713	1713	1713	1713	1713	1713	1713	1713
Flt Permitted	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72	0.72
Satd. Flow (perm)	1254	1254	1254	1254	1254	1254	1254	1254	1254	1254	1254	1254
Volume (vph)	54	6	24	73	244	20	139	11	99	1391	27	226
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	62	7	28	84	274	22	156	12	102	1434	28	233
RTOR Reduction (vph)	0	0	31	0	0	2	0	0	0	0	0	91
Lane Group Flow (vph)	0	0	150	0	274	188	0	0	102	1462	0	142
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	custom	custom
Protected Phases	8	8	8	8	4	4	4	4	2	2	6	6
Permitted Phases	8	8	8	8	4	4	4	4	2	2	6	6
Actuated Green, G (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0	42.0	42.0	54.0	54.0
Effective Green, g (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0	42.0	42.0	54.0	54.0
Actuated g/C Ratio	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.42	0.42	0.54	0.54
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	288	288	288	288	241	372	241	372	90	2130	855	855
v/s Ratio Prot	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.47	0.29	0.09	0.09
v/s Ratio Perm	0.52	0.52	0.52	0.52	1.14	0.50	1.14	0.50	1.13	0.69	0.17	0.17
Uniform Delay, d1	33.7	33.7	33.7	33.7	38.5	33.5	38.5	33.5	29.0	23.6	11.6	11.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.54	0.38	0.12	0.12
Incremental Delay, d2	1.7	1.7	1.7	1.7	99.9	1.1	99.9	1.1	72.6	0.2	0.0	0.0
Delay (s)	35.4	35.4	35.4	35.4	138.4	34.6	138.4	34.6	88.4	9.1	1.4	1.4
Level of Service	D	D	D	D	F	C	F	C	F	A	A	A
Approach Delay (s)	35.4	35.4	35.4	35.4	95.9	95.9	95.9	95.9	12.6	12.6	12.6	12.6
Approach LOS	D	D	D	D	F	F	F	F	B	B	B	B
Intersection Summary												
HCM Average Control Delay	26.3											
HCM Volume to Capacity ratio	1.02											
Actuated Cycle Length (s)	100.0											
Intersection Capacity Utilization	81.2%											
Analysis Period (min)	15											
c Critical Lane Group												

Baseline
A & R Engineering Inc.

Synchro 6 Report
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HCM Signalized Intersection Capacity Analysis
2: Kenwood Rd & Oakdale Rd

Future SAT
5/18/2007

Movement	SBL2	SBL	SBT	SBR	SWL2	SWL	SWR	SWR2
Lane Configurations		↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Satd. Flow (prot)	1770	1770	1770	1770	1770	1770	1770	1770
Flt Permitted	0.09	0.09	0.09	0.09	0.09	0.09	0.09	0.09
Satd. Flow (perm)	162	162	162	162	162	162	162	162
Volume (vph)	10	167	1544	46	13	71	4	12
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	11	178	1643	49	14	76	4	13
RTOR Reduction (vph)	0	0	0	28	0	0	0	12
Lane Group Flow (vph)	0	189	1643	21	0	90	5	0
Turn Type	pm+pt	pm+pt	pm+pt	custom	Perm	Perm	Perm	Perm
Protected Phases	1	1	6	2	13	13	13	13
Permitted Phases	6	6	6	2	13	13	13	13
Actuated Green, G (s)	54.0	54.0	54.0	42.0	11.0	11.0	11.0	11.0
Effective Green, g (s)	54.0	54.0	54.0	42.0	11.0	11.0	11.0	11.0
Actuated g/C Ratio	0.54	0.54	0.54	0.42	0.11	0.11	0.11	0.11
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	216	2746	665	665	195	174	174	174
v/s Ratio Prot	0.07	0.32	0.01	0.01	0.05	0.00	0.00	0.00
v/s Ratio Perm	0.40	0.88	0.60	0.03	0.46	0.03	0.03	0.03
Uniform Delay, d1	21.6	15.6	17.0	41.7	39.7	39.7	39.7	39.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	30.1	1.0	0.1	7.7	0.3	0.3	0.3	0.3
Delay (s)	51.7	16.6	17.1	49.4	40.1	40.1	40.1	40.1
Level of Service	D	B	B	D	D	D	D	D
Approach Delay (s)	20.1	20.1	20.1	47.9	47.9	47.9	47.9	47.9
Approach LOS	C	C	C	D	D	D	D	D
Intersection Summary								

Baseline
A & R Engineering Inc.

Synchro 6 Report
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Lanes, Volumes, Timings
3: Highlands Pkwy & S Cobb Dr

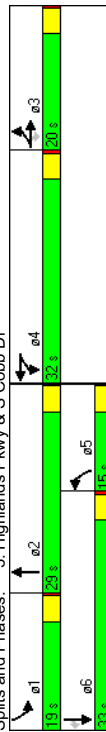
Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1681	1736	1583	0	1747	0	3433	4943	0	1770	5085	1583
Flt Permitted	0.950	0.981			0.971		0.950			0.950		
Satd. Flow (perm)	1681	1736	1583	0	1747	0	3433	4943	0	1770	5085	1583
Satd. Flow (RTOR)			241		17			49				121
Volume (vph)	311	109	274	381	88	173	315	1249	252	284	1301	320
Lane Group Flow (vph)	243	255	285	0	771	0	342	1581	0	379	1328	340
Turn Type	Split	Perm	Split	Prot	Split	Prot	Prot	Prot	Prot	Prot	Perm	Perm
Protected Phases	3	3	3	4	4	4	5	2			1	6
Permitted Phases												
Total Split (s)	20.0	20.0	20.0	32.0	32.0	0.0	15.0	29.0	0.0	19.0	33.0	33.0
Act Effct Green (s)	16.0	16.0	16.0	28.0	28.0	0.0	11.0	25.0	0.0	15.0	29.0	29.0
Actuated g/C Ratio	0.16	0.16	0.16	0.28	0.28	0.0	0.11	0.25	0.0	0.15	0.29	0.29
Control Delay	78.0	79.8	15.0	280.7	63.2	143.9	241.9	31.7	15.9			
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	78.0	79.8	15.0	280.7	63.2	143.9	241.9	31.7	15.9			
LOS	E	E	B	F	F	F	E	F	F	F	C	B
Approach Delay		55.7		280.7			129.6					E
Approach LOS		E		F			F					E

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Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 28 (28%), Referenced to phase 2:NBT and 6:SBT, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.54												
Intersection Signal Delay: 117.4												
Intersection Capacity Utilization 98.4%												
Analysis Period (min) 15												

Splits and Phases: 3: Highlands Pkwy & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
3: Highlands Pkwy & S Cobb Dr

Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1681	1736	1583	0	1747	0	3433	4943	0	1770	5085	1583
Flt Permitted	0.950	0.981			0.971		0.950			0.950		
Satd. Flow (perm)	1681	1736	1583	0	1747	0	3433	4943	0	1770	5085	1583
Satd. Flow (RTOR)			241		17			49				121
Volume (vph)	311	109	274	381	88	173	315	1249	252	284	1301	320
Lane Group Flow (vph)	243	255	285	0	771	0	342	1581	0	379	1328	340
Turn Type	Split	Perm	Split	Prot	Split	Prot	Prot	Prot	Prot	Prot	Perm	Perm
Protected Phases	3	3	3	4	4	4	5	2			1	6
Permitted Phases												
Actuated Green, G (s)	16.0	16.0	16.0	28.0	28.0	0.0	11.0	25.0	0.0	15.0	29.0	29.0
Effective Green, g (s)	16.0	16.0	16.0	28.0	28.0	0.0	11.0	25.0	0.0	15.0	29.0	29.0
Actuated g/C Ratio	0.16	0.16	0.16	0.28	0.28	0.0	0.11	0.25	0.0	0.15	0.29	0.29
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	269	278	253	489	489	378	1236		266	1475	459	
v/s Ratio Prot	0.14	c0.15		c0.43		0.10	c0.31		c0.21	0.26		
v/s Ratio Perm	0.90	0.92	0.33	1.55		0.90	1.25		1.42	0.90	0.55	
Uniform Delay, d1	41.2	41.3	37.2	36.0		44.0	37.5		42.5	34.1	30.0	
Progression Factor	1.00	1.00	1.00	1.00		0.91	0.88		1.14	0.70	0.60	
Incremental Delay, d2	30.8	32.7	0.8	258.2		19.3	117.4		206.7	7.1	3.5	
Delay (s)	72.0	74.1	38.0	294.2		59.1	150.3		255.3	31.1	21.6	
Level of Service	E	E	D	F		E	F		F	C	C	
Approach Delay (s)		60.3		294.2			134.1			71.0		
Approach LOS		E		F			F			E		
Intersection Summary												
HCM Average Control Delay												
HCM Volume to Capacity ratio												
Actuated Cycle Length (s)												
Intersection Capacity Utilization 98.4%												
Analysis Period (min) 15												
Critical Lane Group												

Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

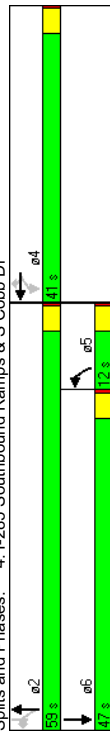
Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	0	1681	1441	1504	1770	3539	0	0	3539	1583
Satd. Flow (prot)	0	0	0	0.950	0.950	0.950	0.950	0.950	0	0	0.950	0.950
Fit Permitted	0	0	0	1681	1441	1504	158	3539	0	0	3539	1583
Satd. Flow (RTOR)	0	0	0	48	48	48	48	48	0	0	48	48
Volume (vph)	0	0	0	251	0	925	63	1218	0	0	1312	736
Lane Group Flow (vph)	0	0	0	310	508	508	77	1282	0	0	1353	775
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	0.0	0.0	0.0	41.0	41.0	41.0	12.0	59.0	0.0	0.0	47.0	59.0
Total Split (s)	0.0	0.0	0.0	35.3	35.3	35.3	56.7	56.7	0.0	0.0	47.1	56.7
Act Effct Green (s)	0.0	0.0	0.0	35.3	35.3	35.3	56.7	56.7	0.0	0.0	47.1	56.7
Actuated g/C Ratio	0.35	0.35	0.35	0.35	0.35	0.35	0.57	0.57	0.0	0.0	0.47	0.57
v/c Ratio	0.52	0.94	0.90	0.36	0.64	0.64	0.81	0.63	0.0	0.0	0.81	0.63
Control Delay	28.9	55.6	48.7	10.1	1.5	15.4	2.9	15.4	0.0	0.0	15.7	2.9
Queue Delay	0.1	0.0	0.0	0.0	1.2	0.4	0.0	0.4	0.0	0.0	0.4	0.0
Total Delay	29.0	55.6	48.7	10.1	2.7	15.7	2.9	15.7	0.0	0.0	16.1	2.9
LOS	C	E	D	B	A	A	B	A	A	A	B	A
Approach Delay	46.7											
Approach LOS	D											

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Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 11 (11%), Referenced to phase 2: NBT and 6: SBT, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.94												
Intersection Signal Delay: 18.7												
Intersection Capacity Utilization 110.3%												
Analysis Period (min) 15												

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



HCM Signalized Intersection Capacity Analysis
4: I-285 Southbound Ramps & S Cobb Dr

Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.91	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Fit Protected	0.95	1.00	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1681	1441	1504	1770	3539	3539	1583	1583	1583	1583	1583	1583
Fit Permitted	0.95	1.00	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	1681	1441	1504	148	3539	3539	1583	1583	1583	1583	1583	1583
Volume (vph)	0	0	0	251	0	925	63	1218	0	0	1312	736
Peak-hour factor, PHF	0.92	0.92	0.92	0.81	0.92	0.91	0.82	0.95	0.92	0.92	0.97	0.95
Adj. Flow (vph)	0	0	0	310	0	1016	77	1282	0	0	1353	775
RTOR Reduction (vph)	0	0	0	0	31	31	0	0	0	0	0	336
Lane Group Flow (vph)	0	0	0	310	477	477	77	1282	0	0	1353	439
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	35.3	35.3	35.3	56.7	56.7	56.7	46.3	56.7	46.3	56.7	46.3	56.7
Effective Green, G (s)	35.3	35.3	35.3	56.7	56.7	56.7	46.3	56.7	46.3	56.7	46.3	56.7
Actuated g/C Ratio	0.35	0.35	0.35	0.35	0.57	0.57	0.46	0.57	0.46	0.57	0.46	0.57
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	593	509	531	188	2007	2007	1639	898	1639	898	1639	898
v/s Ratio Prot	0.18	0.33	0.32	0.21	0.21	0.21	0.03	0.36	0.03	0.36	0.03	0.36
v/s Ratio Perm	0.52	0.94	0.90	0.41	0.64	0.64	0.83	0.49	0.83	0.49	0.83	0.49
Uniform Delay, d1	25.7	31.3	30.6	34.6	14.7	14.7	23.3	13.0	23.3	13.0	23.3	13.0
Progression Factor	1.00	1.00	1.00	0.39	0.09	0.09	0.52	1.80	0.52	1.80	0.52	1.80
Incremental Delay, d2	0.8	24.9	17.8	0.1	0.1	0.1	2.1	0.8	2.1	0.8	2.1	0.8
Delay (s)	26.5	56.2	48.4	13.7	1.5	1.5	14.3	24.2	14.3	24.2	14.3	24.2
Level of Service	C	E	D	B	A	A	B	C	B	C	B	C
Approach Delay (s)	0.0	0.0	0.0	46.3	2.2	2.2	17.9	17.9	17.9	17.9	17.9	17.9
Approach LOS	A	A	A	D	A	A	B	B	B	B	B	B
Intersection Summary												
HCM Average Control Delay	21.3											
HCM Volume to Capacity ratio	0.84											
Actuated Cycle Length (s)	100.0											
Intersection Capacity Utilization	110.3%											
Analysis Period (min)	15											
c Critical Lane Group												

Baseline
A & R Engineering Inc.

Synchro 6 Report
Page 10

Lanes, Volumes, Timings
4: I-285 Southbound Ramps & S Cobb Dr

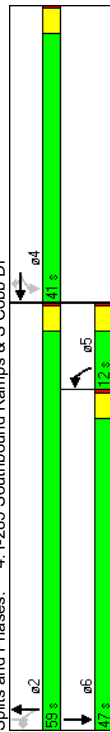
Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	0	0	0	1681	1441	1504	1770	3539	0	0	3539	1583
Satd. Flow (prot)	0	0	0	0.950	0.950	0.950	0.950	0.950	0	0	0.950	0.950
Fit Permitted	0	0	0	1681	1441	1504	158	3539	0	0	3539	1583
Satd. Flow (RTOR)	0	0	0	48	48	48	48	48	0	0	48	48
Volume (vph)	0	0	0	251	0	925	63	1218	0	0	1312	736
Lane Group Flow (vph)	0	0	0	310	508	508	77	1282	0	0	1353	775
Turn Type	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	4	4	4	4	4	4	4	4	4
Permitted Phases	0.0	0.0	0.0	41.0	41.0	41.0	12.0	59.0	0.0	0.0	47.0	59.0
Total Split (s)	0.0	0.0	0.0	35.3	35.3	35.3	56.7	56.7	0.0	0.0	47.1	56.7
Act Effct Green (s)	0.0	0.0	0.0	35.3	35.3	35.3	56.7	56.7	0.0	0.0	47.1	56.7
Actuated g/C Ratio	0.35	0.35	0.35	0.35	0.35	0.35	0.57	0.57	0.0	0.0	0.47	0.57
v/c Ratio	0.52	0.94	0.90	0.36	0.64	0.64	0.81	0.63	0.0	0.0	0.81	0.63
Control Delay	28.9	55.6	48.7	10.1	1.5	15.4	2.9	15.4	0.0	0.0	15.7	2.9
Queue Delay	0.1	0.0	0.0	0.0	1.2	0.4	0.0	0.4	0.0	0.0	0.4	0.0
Total Delay	29.0	55.6	48.7	10.1	2.7	15.7	2.9	15.7	0.0	0.0	16.1	2.9
LOS	C	E	D	B	A	A	B	A	A	A	B	A
Approach Delay	46.7											
Approach LOS	D											

A-83

Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 11 (11%), Referenced to phase 2: NBT and 6: SBT, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.94												
Intersection Signal Delay: 18.7												
Intersection Capacity Utilization 110.3%												
Analysis Period (min) 15												

Splits and Phases: 4: I-285 Southbound Ramps & S Cobb Dr



Baseline
A & R Engineering Inc.

Synchro 6 Report
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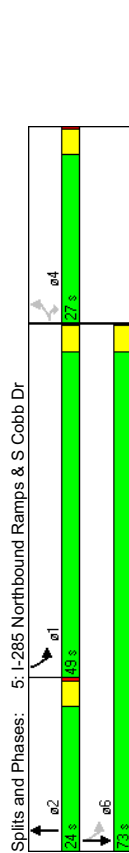
Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

Future SAT
5/18/2007



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3327	0	1770	3539	0	0
Flt Permitted	0.950								0.167			
Satd. Flow (perm)	3433	0	1583	0	0	0	3327	0	311	3539	0	0
Satd. Flow (RTOR)			64				138					
Volume (vph)	811	0	56	0	0	0	459	297	992	623	0	0
Lane Group Flow (vph)	882	0	64	0	0	0	820	0	1033	663	0	0
Turn Type	custom	custom								pm+pt		
Protected Phases	4		4				2			1		6
Permitted Phases												
Total Split (s)	27.0	0.0	27.0	0.0	0.0	0.0	24.0	0.0	49.0	73.0	0.0	0.0
Act Effct Green (s)	23.0		23.0				20.0		69.0	69.0		
Actuated g/C Ratio	0.23		0.23				0.20		0.69	0.69		
v/c Ratio	1.12		0.15				1.06		1.19	0.27		
Control Delay	105.8		9.1				81.5		120.3	4.2		
Queue Delay	0.0		0.0				0.0		0.0	0.0		
Total Delay	105.8		9.1				81.5		120.3	4.2		
LOS	F		A				F		F	A		
Approach Delay												
Approach LOS							F		F	E		

Intersection Summary												
Cycle Length: 100												
Actuated Cycle Length: 100												
Offset: 32 (32%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.19												
Intersection Signal Delay: 83.1												
Intersection Capacity Utilization 110.3%												
Analysis Period (min) 15												



Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

Future SAT
5/18/2007



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.85	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	3433	1583		1583			3328		1770	3539		
Flt Permitted	0.95	1.00		1.00			1.00		0.17	1.00		
Satd. Flow (perm)	3433	1583		1583			3328		310	3539		
Volume (vph)	811	0	56	0	0	0	459	297	992	623	0	0
Peak-hour factor, PHF	0.92	0.92	0.88	0.92	0.92	0.92	0.93	0.91	0.96	0.94	0.92	0.92
Adj. Flow (vph)	882	0	64	0	0	0	494	326	1033	663	0	0
RTOR Reduction (vph)	0	0	49	0	0	0	110	0	0	0	0	0
Lane Group Flow (vph)	882	0	15	0	0	0	710	0	1033	663	0	0
Turn Type	custom	custom								pm+pt		
Protected Phases	4		4				2			1		6
Permitted Phases												
Actuated Green, G (s)	23.0		23.0				20.0		69.0	69.0		
Effective Green, g (s)	23.0		23.0				20.0		69.0	69.0		
Actuated g/C Ratio	0.23		0.23				0.20		0.69	0.69		
Clearance Time (s)	4.0		4.0				4.0		4.0	4.0		
Vehicle Extension (s)	3.0		3.0				3.0		3.0	3.0		
Lane Grp Cap (vph)	790		364				666		871	2442		
v/s Ratio Prot							0.21		0.53	0.19		
v/s Ratio Perm	0.26		0.01						0.28			
v/c Ratio	1.12		0.04				1.07		1.19	0.27		
Uniform Delay, d1	38.5		29.9				40.0		24.5	5.9		
Progression Factor	1.00		1.00				1.00		1.19	0.88		
Incremental Delay, d2	69.0		0.0				53.6		91.5	0.2		
Delay (s)	107.5		30.0				93.6		120.7	4.2		
Level of Service	F		C				F		F	A		
Approach Delay (s)		102.3			0.0		93.6			75.1		
Approach LOS		F			A		F			E		
Intersection Summary												
HCM Average Control Delay	86.9											
HCM Volume to Capacity ratio	1.14											
Actuated Cycle Length (s)	100.0											
Intersection Capacity Utilization	110.3%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
6: Church Rd & N Church Lane

Future SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1807	0	0	1788	0	0	1736	0	0	1666	0
Flt Permitted	0	0.973	0	0	0.984	0	0	0.984	0	0	0.984	0
Satd. Flow (perm)	0	1807	0	0	1788	0	0	1736	0	0	1666	0
Volume (vph)	69	72	1	0	75	37	0	1	1	40	0	84
Lane Group Flow (vph)	0	186	0	0	142	0	0	8	0	0	157	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 35.1%												
Analysis Period (min) 15												
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis
6: Church Rd & N Church Lane

Future SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	69	72	1	0	75	37	0	1	1	40	0	84
Peak Hour Factor	0.66	0.94	0.25	0.92	0.75	0.88	0.92	0.25	0.25	0.79	0.92	0.79
Hourly flow rate (vph)	105	77	4	0	100	42	0	4	4	51	0	106
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	100			81			515	388	79	415	411	121
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
vCu, unblocked vol	100			81			515	388	79	415	411	121
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	93			100			100	99	100	90	100	89
cM capacity (veh/h)	1493			1517			394	509	982	514	494	930
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	185	142	8	157								
Volume Left	105	0	0	51								
Volume Right	4	42	4	106								
cSH	1493	1517	670	737								
Volume to Capacity	0.07	0.00	0.01	0.21								
Queue Length 95th (ft)	6	0	1	20								
Control Delay (s)	4.5	0.0	10.4	11.2								
Lane LOS	A	B	B	B								
Approach Delay (s)	4.5	0.0	10.4	11.2								
Approach LOS	B	B	B	B								
Intersection Summary												
Average Delay				5.4								
Intersection Capacity Utilization				35.1%						A		
Analysis Period (min)				15								

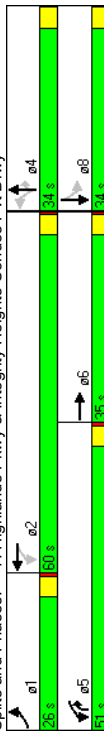
Lanes, Volumes, Timings
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3447	0	1770	3532	0	1770	1863	1583	0	1863	0
Flt Permitted	0.950	0.414		0.414			0.755					
Satd. Flow (perm)	1770	3447	0	771	3532	0	1406	1863	1583	0	1863	0
Satd. Flow (RTOR)	19			2								
Volume (vph)	1	250	43	417	236	1	37	0	417	0	2	0
Lane Group Flow (vph)	4	317	0	434	266	0	46	0	444	0	4	0
Turn Type	Prot	pm+pt		pm+pt		Perm	pm+ov	Perm				
Protected Phases	1	6	5	2		4	4	5		8		
Permitted Phases												
Total Split (s)	26.0	35.0	0.0	51.0	60.0	0.0	34.0	34.0	51.0	34.0	34.0	0.0
Act Effct Green (s)	5.8	42.9		58.7	58.9		7.9	19.0		7.7		
Actuated g/C Ratio	0.07	0.61		0.84	0.84		0.11	0.27		0.10		
v/c Ratio	0.03	0.15		0.53	0.09		0.31	0.69		0.02		
Control Delay	37.0	7.9		4.6	2.8		37.0	13.0		31.0		
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0		
Total Delay	37.0	7.9		4.6	2.8		37.0	13.0		31.0		
LOS	D	A		A	A		D	B		C		
Approach Delay		8.3			3.9							
Approach LOS		A			A							

Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 70.1												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.69												
Intersection Signal Delay: 8.6												
Intersection Capacity Utilization 50.1%												
Analysis Period (min) 15												

Splits and Phases: 7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy



HCM Signalized Intersection Capacity Analysis
7: Highlands Pkwy & Integrity Heights Condos Pvt Drwy

Future SAT
5/18/2007

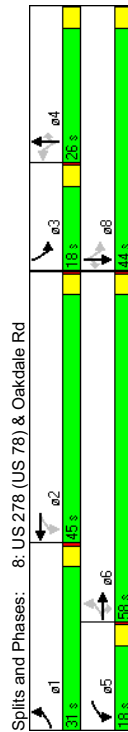
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1770	3449	1770	3531	1770	3531	1770	1583	1583	1863	1863	1863
Flt Permitted	0.95	1.00	0.52	1.00	0.78	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1770	3449	959	3531	1461	1583	1583	1583	1583	1583	1583	1583
Volume (vph)	1	250	43	417	236	1	37	0	417	0	2	0
Peak-hour factor, PHF	0.25	0.95	0.79	0.96	0.90	0.25	0.80	0.92	0.94	0.92	0.50	0.92
Adj. Flow (vph)	4	263	54	434	262	4	46	0	444	0	4	0
RTOR Reduction (vph)	0	7	0	0	0	0	0	0	225	0	0	0
Lane Group Flow (vph)	4	310	0	434	266	0	46	0	219	0	4	0
Turn Type	Prot	pm+pt		pm+pt		Perm	pm+ov	Perm				
Protected Phases	1	6	5	2		4	4	5		8		
Permitted Phases												
Actuated Green, G (s)	1.0	46.3		62.0	57.0		5.1	16.8		5.1		
Effective Green, g (s)	1.0	46.3		62.0	57.0		5.1	16.8		5.1		
Actuated g/C Ratio	0.01	0.62		0.83	0.76		0.07	0.22		0.07		
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0		
Lane Grp Cap (vph)	24	2126		918	2680		99	438		127		
v/s Ratio Prot	0.00	0.09		c0.07	0.08		0.03	c0.08		0.00		
v/s Ratio Perm				c0.32			0.03			0.06		
v/c Ratio	0.17	0.15		0.47	0.10		0.46	0.50		0.03		
Uniform Delay, d1	36.6	6.1		1.7	2.4		33.7	25.5		32.7		
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00		
Incremental Delay, d2	3.3	0.0		0.4	0.1		3.4	0.9		0.1		
Delay (s)	39.9	6.1		2.1	2.4		37.1	26.4		32.8		
Level of Service	D	A		A	A		D	C		C		
Approach Delay (s)		6.5			2.2			27.4				
Approach LOS		A			A			C				
Intersection Summary												
HCM Average Control Delay							11.3					
HCM Volume to Capacity ratio							0.47					
Actuated Cycle Length (s)							75.1					
Intersection Capacity Utilization							50.1%					
Analysis Period (min)							15					
c Critical Lane Group												

Lanes, Volumes, Timings
8: US 278 (US 78) & Oakdale Rd

Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	3539	1583	1770	3486	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.171			0.359			0.690			0.507		
Satd. Flow (perm)	319	3539	1583	669	3486	0	1285	1863	1583	944	1863	1583
Satd. Flow (RTOR)				74	10					59		
Volume (vph)	213	667	65	73	642	68	63	98	56	67	79	241
Lane Group Flow (vph)	239	741	76	85	782	0	81	115	59	82	103	268
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm
Protected Phases	1	6	5	2			4		4	3	8	
Permitted Phases	6		6	2			4		4	8		8
Total Split (s)	31.0	58.0	18.0	45.0	0.0	26.0	26.0	18.0	18.0	44.0	44.0	44.0
Act Effct Green (s)	64.1	55.5	55.5	56.8	49.7		11.4	11.4	11.4	22.0	21.7	21.7
Actuated g/C Ratio	0.68	0.59	0.59	0.59	0.53		0.12	0.12	0.12	0.23	0.23	0.23
v/c Ratio	0.63	0.36	0.08	0.18	0.43		0.52	0.51	0.24	0.28	0.24	0.47
Control Delay	15.8	13.6	3.8	8.3	16.8		54.1	49.5	13.3	31.2	30.4	6.5
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	15.8	13.6	3.8	8.3	16.8		54.1	49.5	13.3	31.2	30.4	6.5
LOS	B	B	A	A	B		D	D	B	C	C	A
Approach Delay		13.4			16.0			42.6			16.4	
Approach LOS		B			B			D			B	

Intersection Summary	
Cycle Length: 120	
Actuated Cycle Length: 94.6	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.63	
Intersection Signal Delay: 17.6	Intersection LOS: B
Intersection Capacity Utilization 52.1%	ICU Level of Service A
Analysis Period (min) 15	



HCM Signalized Intersection Capacity Analysis
8: US 278 (US 78) & Oakdale Rd

Future SAT
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.99	1.00	1.00	0.85	1.00	0.95	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3487		1770	1863	1583	1770	1863	1583
Flt Permitted	0.27	1.00	1.00	0.35	1.00		0.69	1.00	1.00	0.43	1.00	1.00
Satd. Flow (perm)	499	3539	1583	655	3487		1286	1863	1583	794	1863	1583
Volume (vph)	213	667	65	73	642	68	63	98	56	67	79	241
Peak-hour factor, PHF	0.89	0.90	0.85	0.86	0.91	0.88	0.78	0.85	0.95	0.82	0.77	0.90
Adj. Flow (vph)	239	741	76	85	705	77	81	115	59	82	103	268
RTOR Reduction (vph)	0	0	31	0	5	0	0	0	52	0	0	205
Lane Group Flow (vph)	239	741	45	85	777	0	81	115	7	82	103	63
Turn Type	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm	pm+pt	Perm
Protected Phases	1	6	5	2			4		4	3	8	
Permitted Phases	6		6	2			4		4	8		8
Actuated Green, G (s)	65.6	55.5	55.5	56.9	50.8		11.4	11.4	11.4	22.7	22.7	22.7
Effective Green, g (s)	65.6	55.5	55.5	56.9	50.8		11.4	11.4	11.4	22.7	22.7	22.7
Actuated g/C Ratio	0.68	0.58	0.58	0.59	0.53		0.12	0.12	0.12	0.24	0.24	0.24
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	482	2040	912	458	1839		152	221	187	261	439	373
v/s Ratio Prot	c0.06	0.21		0.01	0.22		c0.06	0.06		c0.02	0.06	
v/s Ratio Perm	c0.28		0.03	0.10			0.53	0.52	0.04	0.31	0.23	0.17
Uniform Delay, d1	7.1	10.9	8.9	8.5	13.8		39.9	39.9	37.6	29.7	29.8	29.3
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.8	0.5	0.1	0.2	0.7		3.6	2.2	0.1	0.7	0.3	0.2
Delay (s)	7.9	11.4	9.0	8.7	14.5		43.5	42.1	37.7	30.4	30.0	29.5
Level of Service	A	B	A	A	B		D	D	D	C	C	C
Approach Delay (s)		10.5			14.0			41.5			29.8	
Approach LOS		B			B			D			C	

Intersection Summary	
HCM Average Control Delay	18.0
HCM Volume to Capacity ratio	0.48
Actuated Cycle Length (s)	96.3
Intersection Capacity Utilization	52.1%
Analysis Period (min)	15
c Critical Lane Group	

Lanes, Volumes, Timings
9: Oak Dr & S Cobb Dr

Future SAT
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔				↔	↔	↔	↔	↔	↔	↔
Satd. Flow (prot)	0	1625	0	0	0	1611	1770	5085	1583	0	5080	0
Flt Permitted		0.996					0.950					
Satd. Flow (perm)	0	1625	0	0	0	1611	1770	5085	1583	0	5080	0
Volume (vph)	16	0	221	0	0	186	228	1687	197	0	1994	13
Lane Group Flow (vph)	0	259	0	0	0	232	285	1776	240	0	2094	0
Sign Control		Stop			Yield			Free			Free	
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 76.0%	ICU Level of Service D											
Analysis Period (min) 15												

HCM Unsignalized Intersection Capacity Analysis
9: Oak Dr & S Cobb Dr

Future SAT
5/18/2007

Intersection has too many lanes per leg.
HCM All-Way analysis is limited to two lanes per leg.
Channelized right turn lanes are not counted.

Lanes, Volumes, Timings
10: Church Rd & Groover Rd

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1861	1583	1770	1863	0	0	1736	0	0	1611	0
Flt Permitted	0	0.999	0.999	0.950	0.950	0	0	0.963	0	0	0.963	0
Satd. Flow (perm)	0	1861	1583	1770	1863	0	0	1736	0	0	1611	0
Volume (vph)	2	215	112	20	189	0	85	0	19	0	0	2
Lane Group Flow (vph)	0	283	149	23	239	0	0	139	0	0	4	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	35.8%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
10: Church Rd & Groover Rd

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	2	215	112	20	189	0	85	0	19	0	0	2
Peak Hour Factor	0.50	0.77	0.75	0.88	0.79	0.92	0.80	0.92	0.58	0.92	0.92	0.50
Hourly flow rate (vph)	4	279	149	23	239	0	106	0	33	0	0	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				None
Median storage (veh)												
Upstream signal (ft)							1244					
pX, platoon unblocked												
VC, conflicting volume	239			429			576	572	279	605	721	239
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
vCu, unblocked vol	239			429			576	572	279	605	721	239
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			75	100	96	100	100	99
cM capacity (veh/h)	1328			1131			419	420	760	385	345	800
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2	SB 1	SB 2	SB 3	SB 4	SB 5	SB 6
Volume Total	283	149	23	239	139	4						
Volume Left	4	0	23	0	106	0						
Volume Right	0	149	0	0	33	4						
cSH	1328	1700	1131	1700	468	800						
Volume to Capacity	0.00	0.09	0.02	0.14	0.30	0.01						
Queue Length 95th (ft)	0	0	2	0	31	0						
Control Delay (s)	0.1	0.0	8.2	0.0	15.9	9.5						
Lane LOS	A	A	A	C	C	A						
Approach Delay (s)	0.1	0.7	15.9	9.5								
Approach LOS	C	C	A	A								
Intersection Summary												
Average Delay	3.0											
Intersection Capacity Utilization	35.8%											
Analysis Period (min)	15											

Lanes, Volumes, Timings
11: Church Rd & North Site Dr 1

Future SAT
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	1770	1863	1754	0
Satd. Flow (prot)	1863	1583	1770	1863	1754	0
Flt Permitted	1863	1583	1770	1863	1754	0
Satd. Flow (perm)	1863	1583	1770	1863	1754	0
Volume (vph)	312	112	20	250	132	19
Lane Group Flow (vph)	339	122	22	272	143	21
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization	31.8%					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
11: Church Rd & North Site Dr 1

Future SAT
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	1770	1863	1754	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Volume (veh/h)	312	112	20	250	132	19
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	339	122	22	272	143	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						None
Median storage (veh)						
Upstream signal (ft)						848
pX, platoon unblocked						
VC, conflicting volume				461		654
VC1, stage 1 conf vol						339
VC2, stage 2 conf vol						
vCu, unblocked vol				461		654
tC, single (s)				4.1		6.4
tC, 2 stage (s)						6.2
tF (s)				2.2		3.5
p0 queue free %				98		66
cM capacity (veh/h)				1100		423
cM capacity (veh/h)						703
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volume Total	339	122	22	272	164	
Volume Left	0	0	22	0	143	
Volume Right	0	122	0	0	21	
cSH	1700	1700	1100	1700	445	
Volume to Capacity	0.20	0.07	0.02	0.16	0.37	
Queue Length 95th (ft)	0	0	2	0	42	
Control Delay (s)	0.0	0.0	8.3	0.0	17.7	
Lane LOS			A		C	
Approach Delay (s)	0.0	0.6		17.7		
Approach LOS				C		
Intersection Summary						
Average Delay	3.4					
Intersection Capacity Utilization	31.8%					
Analysis Period (min)	15					
ICU Level of Service	A					

Lanes, Volumes, Timings
12: Church Rd & North Site Dr 3

Future SAT
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	0	1848	1736	0
Satd. Flow (prot)	1863	1583	0	1848	1736	0
Flt Permitted	1863	1583	0	1848	1736	0
Volume (vph)	127	107	26	133	77	24
Lane Group Flow (vph)	138	116	0	173	110	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization 30.9%						
Analysis Period (min) 15						
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis
12: Church Rd & North Site Dr 3

Future SAT
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	0	1848	1736	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Volume (veh/h)	127	107	26	133	77	24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	138	116	28	145	84	26
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						None
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
VC, conflicting volume			254		339	138
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
vCu, unblocked vol			254		339	138
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		87	97
cM capacity (veh/h)			1311		642	910
Direction, Lane #						
Volume Total	138	116	173	110		
Volume Left	0	0	28	84		
Volume Right	0	116	0	26		
cSH	1700	1700	1311	691		
Volume to Capacity	0.08	0.07	0.02	0.16		
Queue Length 95th (ft)	0	0	2	14		
Control Delay (s)	0.0	0.0	1.4	11.2		
Lane LOS			A	B		
Approach Delay (s)	0.0	1.4	11.2			
Approach LOS			B			
Intersection Summary						
Average Delay			2.7			
Intersection Capacity Utilization			30.9%			
Analysis Period (min)			15			
ICU Level of Service A						

Future 2009 SAT Improved

Lanes, Volumes, Timings
1: Cumberland Pkwy & S Cobb Dr

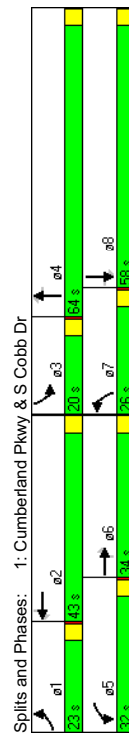
HCM Signalized Intersection Capacity Analysis
1: Cumberland Pkwy & S Cobb Dr

Future SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA	AAA
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Satd. Flow (RTOR)	1770	5085	1583	1770	5085	1583	3433	3539	1583	3433	3539	1583
Volume (vph)	136	843	292	221	854	149	388	1008	200	249	1019	125
Lane Group Flow (vph)	162	878	336	257	1005	157	417	1072	222	277	1073	149
Turn Type	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom	Prot	custom
Protected Phases	1	6	8	5	2	4	7	4	3	8		
Permitted Phases												
Total Split (s)	23.0	34.0	58.0	32.0	43.0	64.0	26.0	34.0	20.0	58.0	43.0	2
Act Effct Green (s)	16.3	32.1	47.2	23.7	39.5	52.6	20.0	52.6	32.1	14.6	47.2	39.5
Actuated g/C Ratio	0.12	0.23	0.34	0.17	0.28	0.38	0.14	0.38	0.23	0.10	0.34	0.28
v/c Ratio	0.78	0.75	0.44	0.85	0.70	0.23	0.85	0.80	0.52	0.77	0.89	0.30
Control Delay	86.4	56.5	5.1	82.2	49.3	9.4	75.4	43.9	36.4	77.1	54.3	23.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	86.4	56.5	5.1	82.2	49.3	9.4	75.4	43.9	36.4	77.1	54.3	23.0
LOS	F	E	A	F	D	A	E	D	D	E	D	C
Approach Delay	47.5											
Approach LOS	D											

A-93

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 139.1												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.89												
Intersection Signal Delay: 51.1												
Intersection Capacity Utilization 81.1%												
Analysis Period (min) 15												



Lanes, Volumes, Timings
5: I-285 Northbound Ramps & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
5: I-285 Northbound Ramps & S Cobb Dr

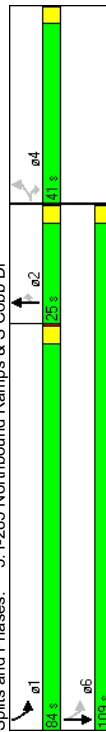
Future SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	3433	0	1583	0	0	0	3539	1583	1770	3539	0	0
Flt Permitted	0.950								0.160			
Satd. Flow (perm)	3433	0	1583	0	0	0	3539	1583	298	3539	0	0
Satd. Flow (RTOR)			64						245			
Volume (vph)	811	0	56	0	0	0	0	459	297	992	623	0
Lane Group Flow (vph)	882	0	64	0	0	0	0	494	326	1033	663	0
Turn Type	custom	custom	custom					Perm	pm+pt			
Protected Phases	4		4					2		1		6
Permitted Phases												
Total Split (s)	41.0	0.0	41.0	0.0	0.0	0.0	0.0	25.0	25.0	84.0	109.0	0.0
Act Effct Green (s)	37.0		37.0					21.0	21.0	105.0	105.0	
Actuated g/C Ratio	0.25		0.25					0.14	0.14	0.70	0.70	
Control Delay	96.1		10.4					103.4	28.2	65.6	5.2	
Queue Delay	1.7		0.0					0.0	0.0	43.3	0.3	
Total Delay	97.8		10.4					103.4	28.2	108.8	5.4	
LOS	F		B					F	C	F	A	
Approach Delay								73.5			68.4	
Approach LOS								E			E	

A-94

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 104 (69%), Referenced to phase 2:NBT and 6:SBTL, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.04												
Intersection Signal Delay: 76.0												
Intersection Capacity Utilization 106.5%												
Analysis Period (min) 15												

Splits and Phases: 5: I-285 Northbound Ramps & S Cobb Dr



Lanes, Volumes, Timings
6: Church Rd & N Church Lane

Future SAT - Improved
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1807	0	0	1788	0	0	1736	0	0	1770	1583
Flt Permitted	0	0.973	0	0	0.973	0	0	0.973	0	0	0.950	0.950
Satd. Flow (perm)	0	1807	0	0	1788	0	0	1736	0	0	1770	1583
Volume (vph)	69	72	1	0	75	37	0	1	1	40	0	84
Lane Group Flow (vph)	0	186	0	0	142	0	0	8	0	0	51	106
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization	29.9%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
6: Church Rd & N Church Lane

Future SAT - Improved
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	69	72	1	0	75	37	0	1	1	40	0	84
Peak Hour Factor	0.66	0.94	0.25	0.92	0.75	0.88	0.92	0.25	0.25	0.79	0.92	0.79
Hourly flow rate (vph)	105	77	4	0	100	42	0	4	4	51	0	106
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None	None	None	None	None
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	100			81			462	388	79	415	411	121
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	100			81			462	388	79	415	411	121
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	93			100			100	99	100	90	100	89
cM capacity (veh/h)	1493			1517			428	509	982	514	494	930
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	185	142	8	157								
Volume Left	105	0	0	51								
Volume Right	4	42	4	106								
cSH	1493	1517	670	1373								
Volume to Capacity	0.07	0.00	0.01	0.11								
Queue Length 95th (ft)	6	0	1	10								
Control Delay (s)	4.5	0.0	10.4	10.5								
Lane LOS	A	B	B	B								
Approach Delay (s)	4.5	0.0	10.4	10.5								
Approach LOS	B	B	B	B								
Intersection Summary												
Average Delay	5.2											
Intersection Capacity Utilization	29.9%											
Analysis Period (min)	15											

Future Site Access Analysis

Future 2009 PM Site Access Analysis

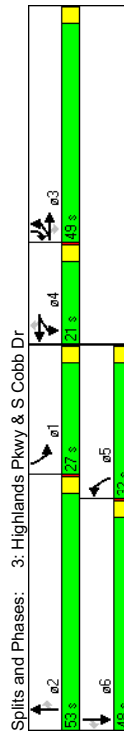
Lanes, Volumes, Timings
3: Highlands Pkwy & S Cobb Dr

Future PM - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1681	1709	1583	3433	1863	1583	3433	5085	1583	1770	5085	1583
Flt Permitted	0.950	0.966		0.950		0.950		0.950		0.950		
Satd. Flow (perm)	1681	1709	1583	3433	1863	1583	3433	5085	1583	1770	5085	1583
Satd. Flow (RTOR)			316			209				88		139
Volume (vph)	680	86	497	277	89	157	520	1416	155	176	1079	561
Lane Group Flow (vph)	413	436	529	322	110	209	559	1460	189	226	1112	616
Turn Type	Split	Perm	Split	Perm	Split	Perm	Split	Perm	Split	Perm	Split	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	3
Permitted Phases	49.0	49.0	49.0	21.0	21.0	21.0	32.0	53.0	53.0	27.0	48.0	49.0
Total Split (s)	42.6	42.6	42.6	16.6	16.6	16.6	27.3	52.5	52.5	22.3	47.5	94.1
Act Effct Green (s)	0.28	0.28	0.28	0.11	0.11	0.11	0.18	0.35	0.35	0.15	0.32	0.63
Actuated g/C Ratio	0.87	0.90	0.78	0.85	0.53	0.58	0.89	0.82	0.31	0.86	0.69	0.59
Control Delay	69.3	73.3	27.8	85.8	73.1	14.3	69.3	48.8	21.4	81.6	33.9	7.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.2	0.0	2.3	0.0	0.0	0.3	0.3
Total Delay	69.3	73.3	27.8	85.8	73.1	14.4	69.3	51.0	21.4	81.6	34.2	8.0
LOS	E	E	C	F	E	B	E	D	C	F	C	A
Approach Delay	54.6			60.4				53.1			31.4	
Approach LOS	D			E				D			C	

A-98

Cycle Length: 150
Actuated Cycle Length: 150
Offset: 127 (85%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.90
Intersection Signal Delay: 47.3
Intersection Capacity Utilization 74.9%
Analysis Period (min) 15



Splits and Phases: 3: Highlands Pkwy & S Cobb Dr

HCM Signalized Intersection Capacity Analysis
3: Highlands Pkwy & S Cobb Dr

Future PM - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	1.00	0.97	1.00	1.00	0.97	0.91	1.00	0.85	1.00	0.85
Flt Protected	0.95	0.97	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.85
Satd. Flow (prot)	1681	1709	1583	3433	1863	1583	3433	5085	1583	1770	5085	1583
Flt Permitted	0.95	0.97	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.85
Satd. Flow (perm)	1681	1709	1583	3433	1863	1583	3433	5085	1583	1770	5085	1583
Volume (vph)	680	86	497	277	89	157	520	1416	155	176	1079	561
Peak-hour factor, PHF	0.94	0.68	0.94	0.86	0.81	0.75	0.93	0.97	0.82	0.78	0.97	0.91
Adj. Flow (vph)	723	126	529	322	110	209	559	1460	189	226	1112	616
RTOR Reduction (vph)	0	0	226	0	0	186	0	0	57	0	0	56
Lane Group Flow (vph)	413	436	503	322	110	23	559	1460	132	226	1112	560
Turn Type	Split	Perm	Split	Perm	Split	Perm	Split	Perm	Split	Perm	Split	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	3
Permitted Phases	42.6	42.6	42.6	16.6	16.6	16.6	27.3	52.5	52.5	22.3	47.5	90.1
Actuated Green, G (s)	42.6	42.6	42.6	16.6	16.6	16.6	27.3	52.5	52.5	22.3	47.5	90.1
Effective Green, g (s)	0.28	0.28	0.28	0.11	0.11	0.11	0.18	0.35	0.35	0.15	0.32	0.60
Actuated g/C Ratio	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	477	485	450	380	206	175	625	1780	554	263	1610	993
Lane Grp Cap (vph)	0.25	c0.26	0.19	0.85	0.53	0.13	0.89	0.82	0.08	0.13	0.22	0.16
v/s Ratio Prot	0.87	0.90	0.67	0.85	0.53	0.13	0.89	0.82	0.24	0.86	0.69	0.56
v/s Ratio Perm	51.0	51.6	47.5	65.5	63.0	60.2	59.9	44.4	34.6	62.3	44.8	18.1
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	0.98	0.70	0.48
Progression Factor	15.1	19.1	3.9	15.9	2.6	0.3	11.1	3.0	0.7	16.3	1.6	0.5
Incremental Delay, d2	66.1	70.8	51.5	81.3	65.7	60.5	67.4	47.6	36.5	77.6	33.1	9.2
Delay (s)	E	E	D	F	E	E	E	D	D	E	C	A
Level of Service	E	E	D	F	E	E	E	D	D	E	C	A
Approach Delay (s)	62.0			71.9			51.7			30.7		
Approach LOS	E			E			D			C		
Intersection Summary												
HCM Average Control Delay	49.4											
HCM Volume to Capacity ratio	0.85											
Actuated Cycle Length (s)	150.0									12.0		
Intersection Capacity Utilization	74.9%									D		
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
10: Church Rd & Groover Dr

Future PM - Improved
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1863	1583	1770	1863	0	0	1752	0	0	1611	0
Flt Permitted				0.950				0.959				
Satd. Flow (perm)	0	1863	1583	1770	1863	0	0	1752	0	0	1611	0
Volume (vph)	0	229	82	20	604	0	75	0	15	0	0	3
Lane Group Flow (vph)	0	283	164	29	636	0	0	124	0	0	8	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 50.2%												
Analysis Period (min) 15												

HCM Unsignalized Intersection Capacity Analysis
10: Church Rd & Groover Dr

Future PM - Improved
5/18/2007

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	0	229	82	20	604	0	75	0	15	0	0	3
Peak Hour Factor	0.92	0.81	0.50	0.70	0.95	0.92	0.70	0.92	0.88	0.92	0.92	0.38
Hourly flow rate (vph)	0	283	164	29	636	0	107	0	17	0	0	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				None
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
VC, conflicting volume	636			447			984	976	283	993	1140	636
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	636			447			984	976	283	993	1140	636
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			97			51	100	98	100	100	98
cM capacity (veh/h)	948			1114			220	245	756	215	196	478
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2	SB 1	SB 2				
Volume Total	283	164	29	636	124	8						
Volume Left	0	0	29	0	107	0						
Volume Right	0	164	0	17	8							
cSH	948	1700	1114	1700	243	478						
Volume to Capacity	0.00	0.10	0.03	0.37	0.51	0.02						
Queue Length 95th (ft)	0	0	2	0	66	1						
Control Delay (s)	0.0	0.0	8.3	0.0	34.3	12.7						
Lane LOS	A	A	A	D	B	B						
Approach Delay (s)	0.0	0.4	34.3	12.7								
Approach LOS	D	D	B									
Intersection Summary												
Average Delay 3.7												
Intersection Capacity Utilization 50.2%												
Analysis Period (min) 15												

Lanes, Volumes, Timings
11: Church Rd & North Site Dr 1

Future PM - Improved
5/18/2007

Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	←	←	←	←	←	←
Satd. Flow (prot)	1863	1583	1770	1863	1756	0
Flt Permitted			0.950		0.958	
Satd. Flow (perm)	1863	1583	1770	1863	1756	0
Volume (vph)	297	82	20	664	108	15
Lane Group Flow (vph)	323	89	22	722	133	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization 48.5%						
Analysis Period (min) 15						
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis
11: Church Rd & North Site Dr 1

Future PM - Improved
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	←	←	←	←	←	←
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Volume (veh/h)	297	82	20	664	108	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	323	89	22	722	117	16
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)		848				
pX, platoon unblocked						
VC, conflicting volume			412		1088	323
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol			412		1088	323
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		50	98
cM capacity (veh/h)			1147		234	718
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volume Total	323	89	22	722	134	
Volume Left	0	0	22	0	117	
Volume Right	0	89	0	0	16	
cSH	1700	1700	1147	1700	255	
Volume to Capacity	0.19	0.05	0.02	0.42	0.52	
Queue Length 95th (ft)	0	0	1	0	70	
Control Delay (s)	0.0	0.0	8.2	0.0	33.6	
Lane LOS			A		D	
Approach Delay (s)	0.0	0.2	0.2		33.6	
Approach LOS					D	
Intersection Summary						
Average Delay	3.6					
Intersection Capacity Utilization	48.5%					
ICU Level of Service	A					
Analysis Period (min)	15					

Lanes, Volumes, Timings
12: Church Rd & North Site Dr 3

Future PM - Improved
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	0	1859	1736	0
Satd. Flow (prot)	1863	1583	0	1859	1736	0
Flt Permitted	1863	1583	0	1859	1736	0
Volume (vph)	169	76	25	559	66	20
Lane Group Flow (vph)	184	83	0	635	94	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization 54.6%						
Analysis Period (min) 15						
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis
12: Church Rd & North Site Dr 3

Future PM - Improved
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	0	1859	1736	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Volume (veh/h)	169	76	25	559	66	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	184	83	27	608	72	22
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						None
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
VC, conflicting volume			266		846	184
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
vCu, unblocked vol			266		846	184
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		78	97
cM capacity (veh/h)			1298		326	859
Direction, Lane #						
Volume Total	184	83	635	93		
Volume Left	0	0	27	72		
Volume Right	0	83	0	22		
cSH	1700	1700	1298	381		
Volume to Capacity	0.11	0.05	0.02	0.25		
Queue Length 95th (ft)	0	0	2	24		
Control Delay (s)	0.0	0.0	0.6	17.5		
Lane LOS			A	C		
Approach Delay (s)	0.0	0.6	17.5			
Approach LOS			C			
Intersection Summary						
Average Delay						
Intersection Capacity Utilization 54.6%						
ICU Level of Service A						
Analysis Period (min) 15						

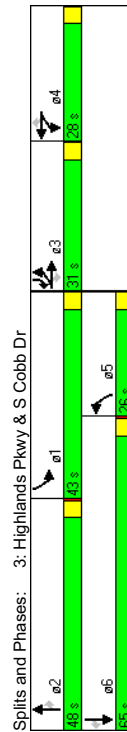
Future 2009 SAT Site Access Analysis

Lanes, Volumes, Timings
3: Highlands Pkwy & S Cobb Dr

Future SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Lane Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Satd. Flow (prot)	1681	1736	1583	3433	1863	1583	3433	5085	1583	1770	5085	1583
Flt Permitted	0.950	0.981		0.950		0.950		0.950		0.950		
Satd. Flow (perm)	1681	1736	1583	3433	1863	1583	3433	5085	1583	1770	5085	1583
Satd. Flow (RTOR)			245			197				148		138
Volume (vph)	311	109	274	381	88	173	315	1249	252	284	1301	320
Lane Group Flow (vph)	243	255	285	465	109	197	342	1288	293	379	1328	340
Turn Type	Split	Perm	Split	Perm	Split	Perm	Split	Perm	Split	Perm	Split	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	3
Permitted Phases												
Total Split (s)	31.0	31.0	31.0	28.0	28.0	28.0	26.0	48.0	48.0	43.0	65.0	31.0
Act Effct Green (s)	25.7	25.7	25.7	23.6	23.6	23.6	19.2	48.5	48.5	36.2	65.5	31.0
Actuated g/C Ratio	0.17	0.17	0.17	0.16	0.16	0.16	0.13	0.32	0.32	0.24	0.44	0.61
Control Delay	85.0	86.2	15.9	77.7	60.4	10.9	74.8	44.2	20.9	68.9	24.9	2.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.6	0.3
Total Delay	85.0	86.2	15.9	77.7	60.4	10.9	74.8	44.2	20.9	70.5	25.1	2.3
LOS	F	F	B	E	E	B	E	D	C	E	C	A
Approach Delay	60.2			58.2			46.1			29.7		
Approach LOS	E			E			D			C		

Intersection Summary												
Cycle Length: 150												
Actuated Cycle Length: 150												
Offset: 64 (43%), Referenced to phase 2:NBT and 6:SBT, Start of Green												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.89												
Intersection Signal Delay: 43.7												
Intersection Capacity Utilization 68.0%												
Analysis Period (min) 15												



HCM Signalized Intersection Capacity Analysis
3: Highlands Pkwy & S Cobb Dr

Future SAT - Improved
5/18/2007

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Lane Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	1.00	0.97	1.00	0.97	0.91	1.00	0.95	1.00	0.91	1.00
Flt Protected	0.95	0.98	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1681	1736	1583	3433	1863	1583	3433	5085	1583	1770	5085	1583
Flt Permitted	0.95	0.98	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1681	1736	1583	3433	1863	1583	3433	5085	1583	1770	5085	1583
Volume (vph)	311	109	274	381	88	173	315	1249	252	284	1301	320
Peak-hour factor, PHF	0.91	0.70	0.96	0.82	0.81	0.88	0.92	0.97	0.86	0.75	0.98	0.94
Adj. Flow (vph)	342	156	285	465	109	197	342	1288	293	379	1328	340
RTOR Reduction (vph)	0	0	203	0	0	166	0	0	100	0	0	54
Lane Group Flow (vph)	243	255	82	465	109	31	342	1288	193	379	1328	286
Turn Type	Split	Perm	Split	Perm	Split	Perm	Split	Perm	Split	Perm	Split	Perm
Protected Phases	3	3	3	4	4	4	5	2	2	1	6	3
Permitted Phases												
Actuated Green, G (s)	25.7	25.7	25.7	23.6	23.6	23.6	19.2	48.5	48.5	36.2	65.5	31.2
Effective Green, g (s)	25.7	25.7	25.7	23.6	23.6	23.6	19.2	48.5	48.5	36.2	65.5	31.2
Actuated g/C Ratio	0.17	0.17	0.17	0.16	0.16	0.16	0.13	0.32	0.32	0.24	0.44	0.61
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	288	297	271	540	293	249	439	1644	512	427	2220	962
v/s Ratio Prot	0.14	c0.15	0.05	c0.14	0.06	0.10	c0.25	0.12	c0.21	0.26	0.05	0.13
v/s Ratio Perm	0.84	0.86	0.30	0.86	0.37	0.12	0.78	0.78	0.38	0.89	0.60	0.30
Uniform Delay, d1	60.2	60.4	54.3	61.6	56.6	54.3	63.3	46.0	39.1	54.9	32.2	14.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.03	0.87	0.91	0.92	0.72	0.35
Incremental Delay, d2	19.6	21.0	0.6	13.2	0.8	0.2	6.5	2.9	1.6	15.8	0.9	0.1
Delay (s)	79.8	81.4	54.9	74.8	57.4	54.5	71.9	43.1	37.2	66.2	24.1	5.0
Level of Service	E	F	D	E	E	D	E	D	D	E	C	A
Approach Delay (s)	71.3			67.2			47.3			28.7		
Approach LOS	E			E			D			C		

Intersection Summary												
HCM Average Control Delay	46.6											
HCM Volume to Capacity ratio	0.84											
Actuated Cycle Length (s)	150.0											
Intersection Capacity Utilization	68.0%											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings
10: Church Rd & Groover Dr

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Satd. Flow (prot)	0	1861	1583	1770	1863	0	0	1736	0	0	1611	0
Flt Permitted	0	0.999	0.999	0.950	0.950	0	0	0.963	0	0	0.963	0
Satd. Flow (perm)	0	1861	1583	1770	1863	0	0	1736	0	0	1611	0
Volume (vph)	2	215	112	20	189	0	85	0	19	0	0	2
Lane Group Flow (vph)	0	283	149	23	239	0	0	139	0	0	4	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Control Type: Unsignalized												
Intersection Capacity Utilization 35.8%												
Analysis Period (min) 15												

HCM Unsignalized Intersection Capacity Analysis
10: Church Rd & Groover Dr

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Volume (veh/h)	2	215	112	20	189	0	85	0	19	0	0	2
Peak Hour Factor	0.50	0.77	0.75	0.88	0.79	0.92	0.80	0.92	0.58	0.92	0.92	0.50
Hourly flow rate (vph)	4	279	149	23	239	0	106	0	33	0	0	4
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None				None
Median storage (veh)												
Upstream signal (ft)							1244					
pX, platoon unblocked												
VC, conflicting volume	239			429			576	572	279	605	721	239
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	239			429			576	572	279	605	721	239
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			75	100	96	100	100	99
cM capacity (veh/h)	1328			1131			419	420	760	385	345	800
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2	SB 1	SB 2	SB 3	SB 4	SB 5	SB 6
Volume Total	283	149	23	239	139	4						
Volume Left	4	0	23	0	106	0						
Volume Right	0	149	0	0	33	4						
cSH	1328	1700	1131	1700	468	800						
Volume to Capacity	0.00	0.09	0.02	0.14	0.30	0.01						
Queue Length 95th (ft)	0	0	2	0	31	0						
Control Delay (s)	0.1	0.0	8.2	0.0	15.9	9.5						
Lane LOS	A	A	A	C	C	A						
Approach Delay (s)	0.1	0.7	15.9	9.5								
Approach LOS	C	C	A	A								
Intersection Summary												
Average Delay												
Intersection Capacity Utilization 35.8%												
ICU Level of Service A												
Analysis Period (min) 15												

Lanes, Volumes, Timings
11: Church Rd & North Site Dr 1

Future SAT - Improved
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	1770	1863	1754	0
Satd. Flow (prot)	1863	1583	1770	1863	1754	0
Flt Permitted	1863	1583	1770	1863	1754	0
Satd. Flow (perm)	1863	1583	1770	1863	1754	0
Volume (vph)	312	112	20	250	123	19
Lane Group Flow (vph)	339	122	22	272	134	21
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization	31.3%					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
11: Church Rd & North Site Dr 1

Future SAT - Improved
5/18/2007

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	1770	1863	1754	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Volume (veh/h)	312	112	20	250	123	19
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	339	122	22	272	134	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						None
Median storage (veh)						
Upstream signal (ft)		848				
pX, platoon unblocked						
VC, conflicting volume			461		654	339
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
vCu, unblocked vol			461		654	339
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		68	97
cM capacity (veh/h)			1100		423	703
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volume Total	339	122	22	272	154	
Volume Left	0	0	22	0	134	
Volume Right	0	122	0	0	21	
cSH	1700	1700	1100	1700	447	
Volume to Capacity	0.20	0.07	0.02	0.16	0.35	
Queue Length 95th (ft)	0	0	2	0	38	
Control Delay (s)	0.0	0.0	8.3	0.0	17.3	
Lane LOS			A		C	
Approach Delay (s)	0.0	0.6			17.3	
Approach LOS					C	
Intersection Summary						
Average Delay	3.1					
Intersection Capacity Utilization	31.3%					
Analysis Period (min)	15					
ICU Level of Service	A					

Lanes, Volumes, Timings
12: Church Rd & North Site Dr 3

HCM Unsignalized Intersection Capacity Analysis
12: Church Rd & North Site Dr 3

Future SAT - Improved
5/18/2007

Future SAT - Improved
5/18/2007

Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	0	1848	1736	0
Satd. Flow (prot)	1863	1583	0	1848	1736	0
Flt Permitted	1863	1583	0	1848	1736	0
Volume (vph)	127	107	26	133	77	24
Lane Group Flow (vph)	138	116	0	173	110	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Control Type: Unsignalized						
Intersection Capacity Utilization 30.9%						
Analysis Period (min) 15						
ICU Level of Service A						

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1863	1583	0	1848	1736	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Volume (veh/h)	127	107	26	133	77	24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	138	116	28	145	84	26
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						None
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
VC, conflicting volume			254		339	138
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
vCu, unblocked vol			254		339	138
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		87	97
cM capacity (veh/h)			1311		642	910
Direction, Lane #						
Volume Total	138	116	173	110		
Volume Left	0	0	28	84		
Volume Right	0	116	0	26		
cSH	1700	1700	1311	691		
Volume to Capacity	0.08	0.07	0.02	0.16		
Queue Length 95th (ft)	0	0	2	14		
Control Delay (s)	0.0	0.0	1.4	11.2		
Lane LOS			A	B		
Approach Delay (s)	0.0	1.4	11.2			
Approach LOS			B			
Intersection Summary						
Average Delay 2.7						
Intersection Capacity Utilization 30.9%						
ICU Level of Service A						
Analysis Period (min) 15						

Traffic Volume Worksheets

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

South Cobb Drive (SR 280) / Cumberland Parkway

P.M. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	Tot	L	T	R	Tot	Tot
Existing:	536	1147	102	1785	195	844	92	1131	167	855	123	1145
Existing Traffic after deductions:	523	1080	93	1697	195	829	92	1116	167	855	120	1142
Growth Factor (%):	3	3	3	3	3	3	3	3	3	3	3	3
Base Condition (Before Reduction):	569	1217	108	1893.7	207	895	98	1200	177	907	130	1215
Base Condition (After Reduction):	555	1146	99	1800	207	879	98	1184	177	907	127	1212
Total New Trips	31	82	75	188	0	75	0	75	0	0	29	29
Future Traffic Volumes:	586	1228	174	1988	207	954	98	1259	177	907	156	1241

SAT. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	Tot	L	T	R	Tot	Tot
Existing:	337	894	108	1339	235	873	118	1226	128	795	239	1162
Existing Traffic after deductions:	330	856	103	1289	235	860	118	1213	128	795	236	1159
Growth Factor (%):	3	3	3	3	3	3	3	3	3	3	3	3
Base Condition (Before Reduction):	358	948	115	1421	249	926	125	1301	136	843	254	1233
Base Condition (After Reduction):	350	909	109	1368	249	912	125	1286	136	843	251	1230
Total New Trips	38	99	91	228	0	107	0	107	0	0	41	41
Future Traffic Volumes:	388	1008	200	1596	249	1019	125	1393	136	843	292	1271

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

South Cobb Drive (SR 280) / Church Road / Kenwood Road / Oakdale Road

P.M. Peak Hour

Condition	Northbound				Southwestbound				Southbound				Eastbound				Westbound			
	L	T	Bear R	R	Tot	Hard L	Bear L	R	Hard R	Tot	L	T	R	Bear R	Tot	L	T	R	Hard R	Tot
Existing:	95	1717	27	94	1933	4	132	10	8	154	45	1123	31	2	1201	35	18	40	3	96
Existing Traffic after deductions:	95	1630	27	94	1846	4	132	10	8	154	25	1123	31	2	1181	35	18	40	3	96
Growth Factor (%):	3	3	3	3		3	3	3	3		3	3	3	3		3	3	3	3	
Base Condition (Before Reduction):	101	1822	29	100	2051	4	140	11	8	163	48	1191	33	2	1274	37	19	42	3	102
Base Condition (After Reduction):	101	1729	29	100	1958	4	140	11	8	163	27	1191	33	2	1253	37	19	42	3	102
Total New Trips	6	113	8	115	242	7	5	0	0	12	104	70	0	0	174	0	7	5	0	12
Future Traffic Volumes	107	1842	37	215	2200	11	145	11	8	175	131	1261	33	2	1427	37	26	47	3	114

SAT Peak Hour

Condition	Northbound				Southwestbound				Southbound				Eastbound				Westbound			
	L	T	Bear R	R	Tot	Hard L	Bear L	R	Hard R	Tot	L	T	R	Bear R	Tot	L	T	R	Hard R	Tot
Existing:	86	1230	17	60	1393	3	60	4	11	78	36	1363	43	9	1451	51	13	62	6	132
Existing Traffic after deductions:	86	1182	17	60	1345	3	60	4	11	78	18	1363	43	9	1433	51	13	62	6	132
Growth Factor (%):	3	3	3	3		3	3	3	3		3	3	3	3		3	3	3	3	
Base Condition (Before Reduction):	91	1305	18	64	1478	3	64	4	12	83	38	1446	46	10	1539	54	14	66	6	140
Base Condition (After Reduction):	91	1254	18	64	1427	3	64	4	12	83	19	1446	46	10	1520	54	14	66	6	140
Total New Trips	8	137	9	162	316	10	7	0	0	17	148	98	0	0	246	0	10	7	0	17
Future Traffic Volumes	99	1391	27	226	1743	13	71	4	12	100	167	1544	46	10	1766	54	24	73	6	157

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

South Cobb Drive (SR 280) / Highland Parkway / Main Site Driveway

P.M. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	456	1353	23	1832	25	1000	495	1520	598	19	468	1085
Existing Traffic after deductions:	456	1266	23	1745	25	987	495	1507	598	19	468	1085
Growth Factor (%):	3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	484	1435	24	1943.6	27	1061	525	1613	634	20	497	1151
Base Condition (After Reduction):	484	1343	24	1851	27	1048	525	1599	634	20	497	1151
Pass by adjustments	0	-39	39	0	69	-69	0	0	0	0	0	0
Total New Trips	36	112	92	240	80	100	36	216	46	66	0	112
Future Traffic Volumes:	520	1416	155	2091	176	1079	561	1815	680	86	497	1263

SAT. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	255	1141	55	1451	24	1265	260	1549	234	14	258	506
Existing Traffic after deductions:	255	1093	55	1403	24	1251	260	1535	234	14	258	506
Growth Factor (%):	3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	271	1210	58	1539	25	1342	276	1643	248	15	274	537
Base Condition (After Reduction):	271	1160	58	1488	25	1327	276	1628	248	15	274	537
Pass by adjustments	0	-62	62	0	147	-147	0	0	0	0	0	0
Total New Trips	44	151	132	327	112	121	44	277	63	94	0	157
Future Traffic Volumes:	315	1249	252	1815	284	1301	320	1905	311	109	274	694

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

South Cobb Drive (SR 280) / I-285 Southbound Off-Ramp

P.M. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	67	1214	0	1281	0	941	730	1671	0	0	0	0
Existing Traffic after deductions:	67	1162	0	1229	0	933	725	1658	0	0	0	0
Growth Factor (%):	3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	71	1288	0	1359	0	998	774	1773	0	0	0	0
Base Condition (After Reduction):	71	1232	0	1303	0	990	769	1759	0	0	0	0
Total New Trips	0	116	0	116	0	189	63	252	0	0	0	0
Future Traffic Volumes:	71	1348	0	1419	0	1179	832	2011	0	0	0	0

SAT. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	59	1015	0	1074	0	1031	628	1659	0	0	0	0
Existing Traffic after deductions:	59	994	0	1053	0	1022	622	1645	0	0	0	0
Growth Factor (%):	3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	63	1077	0	1139	0	1094	666	1760	0	0	0	0
Base Condition (After Reduction):	63	1054	0	1117	0	1084	660	1745	0	0	0	0
Total New Trips	0	164	0	164	0	228	76	304	0	0	0	0
Future Traffic Volumes:	63	1218	0	1281	0	1312	736	2049	0	0	0	0

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

South Cobb Drive (SR 280) / I-285 Northbound Off-Ramp

P.M. Peak Hour

Condition	Northbound				Southbound				Eastbound				Westbound				
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	
Existing:	0	442	265	707		701	620	0	1321	858	0	75	933	0	0	0	0
Existing Traffic after deductions:	0	434	265	699		694	619	0	1313	814	0	75	889	0	0	0	0
Growth Factor (%):	3	3	3		3	3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	0	469	281	750.06		744	658	0	1401	910	0	80	990	0	0	0	0
Base Condition (After Reduction):	0	460	281	742		736	657	0	1393	863	0	80	943	0	0	0	0
Total New Trips	0	58	0	58		126	63	0	189	58	0	0	58	0	0	0	0
Future Traffic Volumes:	0	518	281	800		862	720	0	1582	921	0	80	1001	0	0	0	0

SAT. Peak Hour

Condition	Northbound				Southbound				Eastbound				Westbound			
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	0	359	280	639	800	517	0	1317	705	0	53	758	0	0	0	0
Existing Traffic after deductions:	0	356	280	636	792	516	0	1308	687	0	53	740	0	0	0	0
Growth Factor (%):	3	3	3		3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	0	381	297	678	849	548	0	1397	748	0	56	804	0	0	0	0
Base Condition (After Reduction):	0	377	297	674	840	547	0	1388	729	0	56	785	0	0	0	0
Total New Trips	0	82	0	82	152	76	0	228	82	0	0	82	0	0	0	0
Future Traffic Volumes:	0	459	297	756	992	623	0	1616	811	0	56	867	0	0	0	0

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

Church Road / North Church Lane / Valley Crest Co Private Driveway

P.M. Peak Hour

Condition	Northbound					Southbound					Eastbound					Westbound				
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	8	9	6	23	104	2	147	253	64	100	1	165	0	331	34	365				
Existing Traffic after deductions:	8	9	6	23	104	2	147	253	64	93	1	158	0	317	34	351				
Growth Factor (%):	3	3	3		3	3	3		3	3	3		3	3	3					
Base Condition (Before Reduction):	8	10	6	24.401	110	2	156	268	68	106	1	175	0	351	36	387				
Base Condition (After Reduction):	8	10	6	24	110	2	156	268	68	99	1	168	0	336	36	372				
Total New Trips	0	0	0	0	0	0	29	29	31	13	0	44	0	12	0	12				
Future Traffic Volumes:	8	10	6	24	110	2	185	297	99	112	1	212	0	348	36	384				

SAT. Peak Hour

Condition	Northbound					Southbound					Eastbound					Westbound				
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	0	1	1	2	38	0	41	79	29	61	1	91	0	63	35	98				
Existing Traffic after deductions:	0	1	1	2	38	0	41	79	29	54	1	84	0	56	35	91				
Growth Factor (%):	3	3	3		3	3	3		3	3	3		3	3	3					
Base Condition (Before Reduction):	0	1	1	2	40	0	43	84	31	65	1	97	0	67	37	104				
Base Condition (After Reduction):	0	1	1	2	40	0	43	84	31	57	1	89	0	59	37	97				
Total New Trips	0	0	0	0	0	0	41	41	38	15	0	53	0	16	0	16				
Future Traffic Volumes:	0	1	1	2	40	0	84	125	69	72	1	142	0	75	37	113				

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

Highlands Parkway / Oakdale Road / Integrity Heights Condos Private Driveway

P.M. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	77	2	353	432	20	16	3	39	5	733	222	960
Existing Traffic after deductions:	77	2	353	432	20	16	3	39	5	733	222	960
Growth Factor (%):	3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	82	2	374	458.31	21	17	3	41	5	778	236	1018
Base Condition (After Reduction):	82	2	374	458	21	17	3	41	5	778	236	1018
Total New Trips	0	0	82	82	0	0	0	0	0	29	0	29
Future Traffic Volumes:	82	2	456	540	21	17	3	41	5	807	236	1047

SAT. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	35	0	286	321	0	2	0	2	1	197	41	239
Existing Traffic after deductions:	35	0	286	321	0	2	0	2	1	197	41	239
Growth Factor (%):	3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	37	0	303	341	0	2	0	2	1	209	43	254
Base Condition (After Reduction):	37	0	303	341	0	2	0	2	1	209	43	254
Total New Trips	0	0	114	114	0	0	0	0	0	41	0	41
Future Traffic Volumes:	37	0	417	455	0	2	0	2	1	250	43	295

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

US 278 (US 78) / Oakdale Road / Discovery Boulevard

P.M. Peak Hour

Condition	Northbound				Southbound				Eastbound				Westbound			
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	75	125	94	294	97	320	422	839	133	461	68	662	207	1243	97	1547
Existing Traffic after deductions:	75	125	94	294	97	320	422	839	133	461	68	662	207	1243	97	1547
Growth Factor (%):	3	3	3		3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	80	133	100	311.9	103	339	448	890	141	489	72	702	220	1319	103	1641
Base Condition (After Reduction):	80	133	100	312	103	339	448	890	141	489	72	702	220	1319	103	1641
Total New Trips	0	0	0	0	13	0	50	63	46	0	0	46	0	0	12	12
Future Traffic Volumes:	80	133	100	312	116	339	498	953	187	489	72	748	220	1319	115	1653

SAT. Peak Hour

Condition	Northbound				Southbound				Eastbound				Westbound			
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	59	92	53	204	49	74	170	293	139	629	61	829	69	605	49	723
Existing Traffic after deductions:	59	92	53	204	49	74	170	293	139	629	61	829	69	605	49	723
Growth Factor (%):	3	3	3		3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	63	98	56	216	52	79	180	311	147	667	65	879	73	642	52	767
Base Condition (After Reduction):	63	98	56	216	52	79	180	311	147	667	65	879	73	642	52	767
Total New Trips	0	0	0	0	15	0	61	76	66	0	0	66	0	0	16	16
Future Traffic Volumes:	63	98	56	216	67	79	241	387	213	667	65	945	73	642	68	783

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

South Cobb Drive (SR 280) / Oak Drive / Tibarron Parkway (Right-in / Right -out Driveway)

P.M. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	L	T	R	L	T	R
Existing:	412	1686	114	2212	0	1459	8	0	245	0	0	87
Existing Traffic after deductions:	412	1686	0	2098	0	1446	8	0	245	0	0	0
Growth Factor (%):	3	3	3		3	3	3	3	3	3	3	3
Base Condition (Before Reduction):	437	1789	121	2346.7	0	1548	8	0	260	0	0	92
Base Condition (After Reduction):	437	1789	0	2226	0	1534	8	0	260	0	0	0
Pass by adjustments	0	-59	59	0	0	0	0	0	0	0	0	64
Total New Trips	0	163	69	232	0	252	0	0	0	0	0	79
Future Traffic Volumes:	437	1893	128	2458	0	1786	8	0	260	0	0	143

SAT. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	L	T	R	L	T	R
Existing:	215	1465	46	1726	0	1607	15	0	208	0	0	48
Existing Traffic after deductions:	215	1465	0	1680	0	1593	15	0	208	0	0	0
Growth Factor (%):	3	3	3		3	3	3	3	3	3	3	3
Base Condition (Before Reduction):	228	1554	49	1831	0	1705	16	0	221	0	0	51
Base Condition (After Reduction):	228	1554	0	1782	0	1690	16	0	221	0	0	0
Pass by adjustments	0	-98	98	0	0	0	0	0	0	0	0	90
Total New Trips	0	231	99	330	0	304	0	0	0	0	0	96
Future Traffic Volumes:	228	1687	197	2112	0	1994	16	0	221	0	0	186

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

Church Road / Groover Drive / North Site Driveway 2

P.M. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	L	T	R	L	T	Tot
Existing:	14	0	7	21	0	0	0	136	20	14	512	526
Existing Traffic after deductions:	0	0	0	0	0	0	0	136	0	0	512	512
Growth Factor (%):	3	3	3		3	3	3	3	3	3	3	
Base Condition (Before Reduction):	15	0	7	22	0	0	0	144	21	15	543	558
Base Condition (After Reduction):	0	0	0	0	0	0	0	144	0	0	543	543
Pass by adjustments	9	0	2	11	0	0	0	-2	2	8	-8	0
Total New Trips	66	0	13	79	0	0	0	87	80	12	69	81
Future Traffic Volumes:	75	0	15	90	0	0	0	229	82	20	604	624

SAT. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	L	T	R	L	T	Tot
Existing:	16	0	7	23	0	0	2	92	18	7	95	102
Existing Traffic after deductions:	0	0	0	0	0	0	2	92	0	0	95	95
Growth Factor (%):	3	3	3		3	3	3	3	3	3	3	
Base Condition (Before Reduction):	17	0	7	24	0	0	2	98	19	7	101	108
Base Condition (After Reduction):	0	0	0	0	0	0	2	98	0	0	101	101
Pass by adjustments	3	0	3	6	0	0	0	-3	3	3	-3	0
Total New Trips	82	0	16	98	0	0	0	120	109	17	91	108
Future Traffic Volumes:	85	0	19	104	0	0	2	215	112	20	189	209

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

Church Road / North Site Driveway 1

P.M. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	L	T	R	L	T	R
Existing:	0	0	0	0	0	0	0	156	0	0	529	0
Existing Traffic after deductions:	0	0	0	0	0	0	0	136	0	0	515	0
Growth Factor (%):	3	3	3		3	3	3	3	3	3	3	3
Base Condition (Before Reduction):	0	0	0	0	0	0	0	166	0	0	561	0
Base Condition (After Reduction):	0	0	0	0	0	0	0	144	0	0	546	0
Pass by adjustments	9	0	2	11	0	0	0	-2	2	8	-8	0
Total New Trips	99	0	13	112	0	0	0	155	80	12	126	0
Future Traffic Volumes:	108	0	15	123	0	0	0	297	82	20	664	0

SAT. Peak Hour

Condition	Northbound			Southbound			Eastbound			Westbound		
	L	T	R	Tot	L	T	L	T	R	L	T	R
Existing:	0	0	0	0	0	0	0	112	0	0	113	0
Existing Traffic after deductions:	0	0	0	0	0	0	0	94	0	0	97	0
Growth Factor (%):	3	3	3		3	3	3	3	3	3	3	3
Base Condition (Before Reduction):	0	0	0	0	0	0	0	119	0	0	120	0
Base Condition (After Reduction):	0	0	0	0	0	0	0	100	0	0	103	0
Pass by adjustments	3	0	3	6	0	0	0	-3	3	3	-3	0
Total New Trips	120	0	16	136	0	0	0	215	109	17	150	0
Future Traffic Volumes:	123	0	19	142	0	0	0	312	112	20	250	0

07-017 Aspens Hills DRI
Traffic Volumes
Future Conditions

A&R Engineering
May-07

Church Road / North Site Driveway 3

P.M. Peak Hour

Condition	Northbound				Southbound				Eastbound				Westbound			
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	0	0	0	0	0	0	0	0	0	143	0	143	0	526	0	526
Existing Traffic after deductions:	0	0	0	0	0	0	0	0	0	136	0	136	0	512	0	512
Growth Factor (%):	3	3	3		3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	0	0	0	0	0	0	0	0	0	152	0	152	0	558	0	558
Base Condition (After Reduction):	0	0	0	0	0	0	0	0	0	144	0	144	0	543	0	543
Pass by adjustments	9	0	2	11	0	0	0	0	0	-2	2	0	8	-8	0	0
Total New Trips	57	0	18	75	0	0	0	0	0	27	74	101	17	24	0	41
Future Traffic Volumes:	66	0	20	86	0	0	0	0	0	169	76	245	25	559	0	584

SAT. Peak Hour

Condition	Northbound				Southbound				Eastbound				Westbound			
	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot	L	T	R	Tot
Existing:	0	0	0	0	0	0	0	0	0	99	0	99	0	102	0	102
Existing Traffic after deductions:	0	0	0	0	0	0	0	0	0	92	0	92	0	95	0	95
Growth Factor (%):	3	3	3		3	3	3		3	3	3		3	3	3	
Base Condition (Before Reduction):	0	0	0	0	0	0	0	0	0	105	0	105	0	108	0	108
Base Condition (After Reduction):	0	0	0	0	0	0	0	0	0	98	0	98	0	101	0	101
Pass by adjustments	3	0	3	6	0	0	0	0	0	-3	3	0	3	-3	0	0
Total New Trips	74	0	21	95	0	0	0	0	0	32	104	136	23	35	0	58
Future Traffic Volumes:	77	0	24	101	0	0	0	0	0	127	107	234	26	133	0	159

Planned and Programmed Improvements

Short Title

SR 280 (SOUTH COBB DRIVE) FROM SR 5 (ATLANTA ROAD) IN COBB COUNTY TO SR 70 (BOLTON ROAD) IN CITY OF ATLANTA

GDOT Project No.

752760-

Federal ID No.

STP-9000(29)

Status

Long Range

Detailed Description and Justification

None

Service Type

Roadway Capacity

Sponsor

GDOT

Jurisdiction

Cobb County

Existing Thru Lane

4 (applicable for road projects only)

Planned Thru Lane

6 (applicable for road projects only)

Corridor Length

10 miles (not applicable for all project types)

Network Year

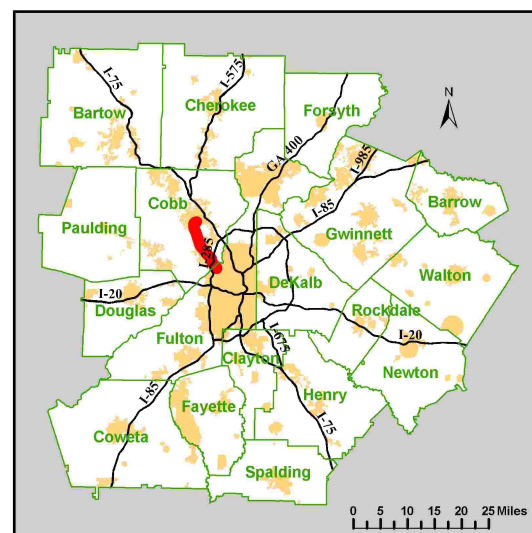
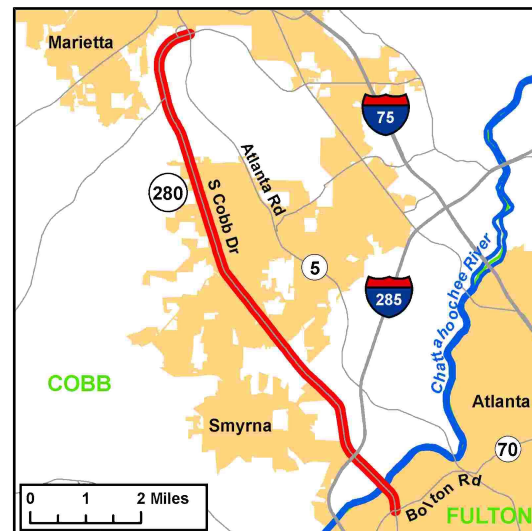
2030 (required if modeled for conformity)

Completion Date

2030

Analysis Level

In the Region's Air Quality Conformity Analysis



Phase Status & Funding Information for 06-11 TIP		FISCAL YEAR	TOTAL PHASE COST	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
				FEDERAL	STATE	BONDS	LOCAL/OTHER
PE	FEDAID-2012-2030	LR 2012-2020	\$850,000	\$680,000	\$170,000	\$0,000	\$0,000
ROW	FEDAID-2012-2030	LR 2012-2020	\$10,900,000	\$8,720,000	\$2,180,000	\$0,000	\$0,000
CST	FEDAID-2012-2030	LR 2021-2030	\$8,500,000	\$6,800,000	\$1,700,000	\$0,000	\$0,000
				\$16,200,000	\$4,050,000	\$0,000	\$0,000

PE: Preliminary Engineering / Design / Study

ROW: Right-of-way Acquisition

CST: Construction / Implementation



For additional information about this project, please visit the Atlanta Regional Commission at www.atlantaregional.com or call (404) 463-3100.



Short Title

CUMBERLAND PARKWAY FROM SR 280 (SOUTH COBB PARKWAY) TO ATLANTA ROAD

GDOT Project No.

N/A

Federal ID No.**Status**

Programmed

Detailed Description and Justification

The Cumberland Parkway road improvement project consists of widening Cumberland Parkway from 4 to 6 lanes from Atlanta Road to South Cobb Drive/SR 280, providing bicycle and pedestrian facilities, and enhancing the ITS and ATMS to coordinate with traffic signaling countywide.

Service Type

Roadway Capacity

Sponsor

Cobb County

Jurisdiction

Cobb County

Existing Thru Lane

4 (applicable for road projects only)

Planned Thru Lane

6 (applicable for road projects only)

Corridor Length

0.63 miles (not applicable for all project types)

Network Year

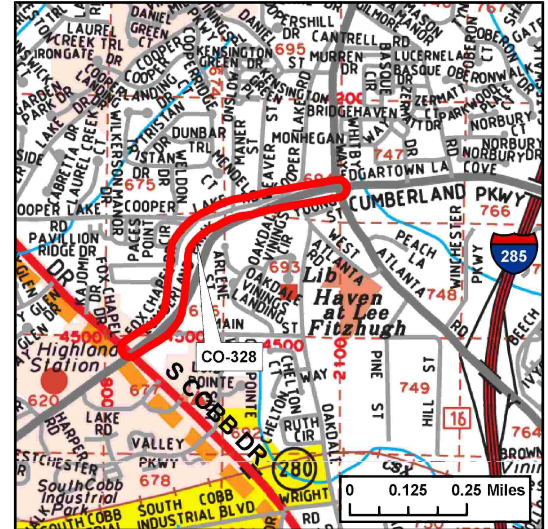
2010 (required if modeled for conformity)

Completion Date

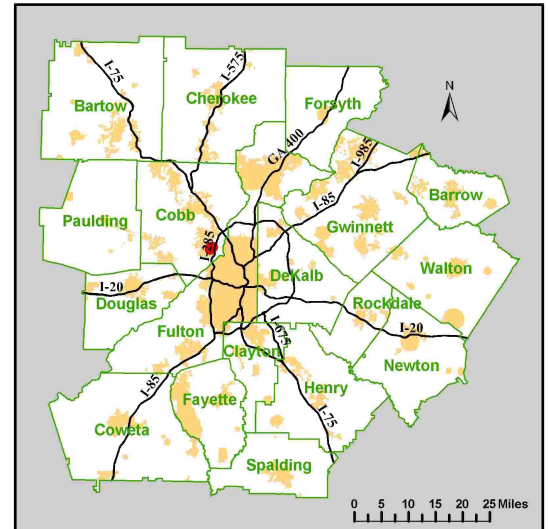
2008

Analysis Level

In the Region's Air Quality Conformity Analysis



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Phase Status & Funding Information for 06-11 TIP		FISCAL YEAR	TOTAL PHASE COST	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
				FEDERAL	STATE	BONDS	LOCAL/OTHER
PE	Local Jurisdiction/Municipality Funds	AUTH	\$0,000	\$0,000	\$0,000	\$0,000	\$0,000
ROW	Local Jurisdiction/Municipality Funds	2005	\$0,000	\$0,000	\$0,000	\$0,000	\$0,000
CST	Local Jurisdiction/Municipality Funds	2006	\$3,000,000	\$0,000	\$0,000	\$0,000	\$3,000,000
				\$0,000	\$0,000	\$0,000	\$3,000,000

PE: Preliminary Engineering / Design / Study

ROW: Right-of-way Acquisition

CST: Construction / Implementation



For additional information about this project, please visit the Atlanta Regional Commission at www.atlantaregional.com or call (404) 463-3100.



Short Title

I-285 WEST AT ATLANTA ROAD BRIDGE [SEE ALSO CO-AR-070A AND CO-AR-070C]

GDOT Project No.

712760-

Federal ID No.

NH-IM-285-1(316)

Status

Long Range

Detailed Description and Justification

This project will construct an interchange at the I-285 West and the East West Connector. It is a companion project to CO-AR 070C.

Service Type

Interchange Capacity

Sponsor

Cobb County

Jurisdiction

Cobb County

Existing Thru Lane

0 (applicable for road projects only)

Planned Thru Lane

4 (applicable for road projects only)

Corridor Length

N/A miles (not applicable for all project types)

Network Year

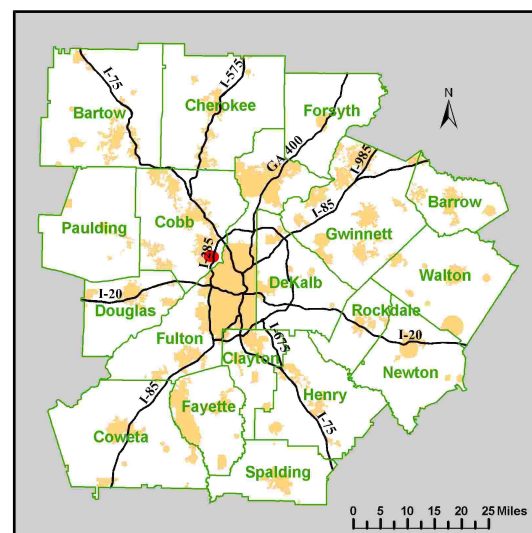
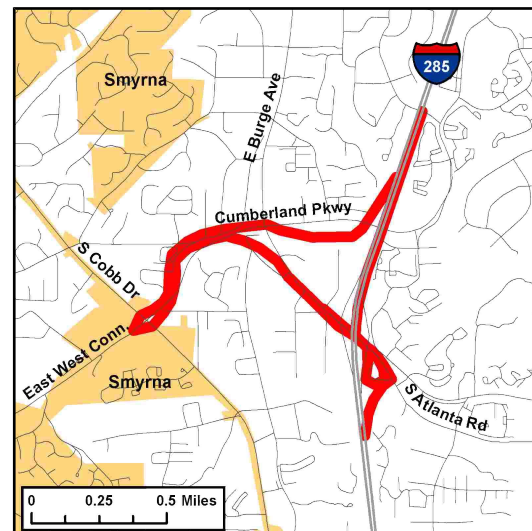
2015 (required if modeled for conformity)

Completion Date

2015

Analysis Level

In the Region's Air Quality Conformity Analysis



Phase Status & Funding Information for 06-11 TIP			FISCAL YEAR	TOTAL PHASE COST	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
					FEDERAL	STATE	BONDS	LOCAL/OTHER
PE	Local Jurisdiction/Municipality Funds	AUTH		\$0,000	\$0,000	\$0,000	\$0,000	\$0,000
ROW	Local Jurisdiction/Municipality Funds	LR 2012-2020		\$5,500,000	\$0,000	\$0,000	\$0,000	\$5,500,000
CST	High Priority Projects from TEA-21	LR 2012-2020		\$19,170,000	\$15,336,000	\$3,834,000	\$0,000	\$0,000
					\$15,336,000	\$3,834,000	\$0,000	\$5,500,000

PE: Preliminary Engineering / Design / Study

ROW: Right-of-way Acquisition

CST: Construction / Implementation

For additional information about this project, please visit the Atlanta Regional Commission at www.atlantaregional.com or call (404) 463-3100.

Bus Routes



Effective 10/09/2006

Route 70 SATURDAY

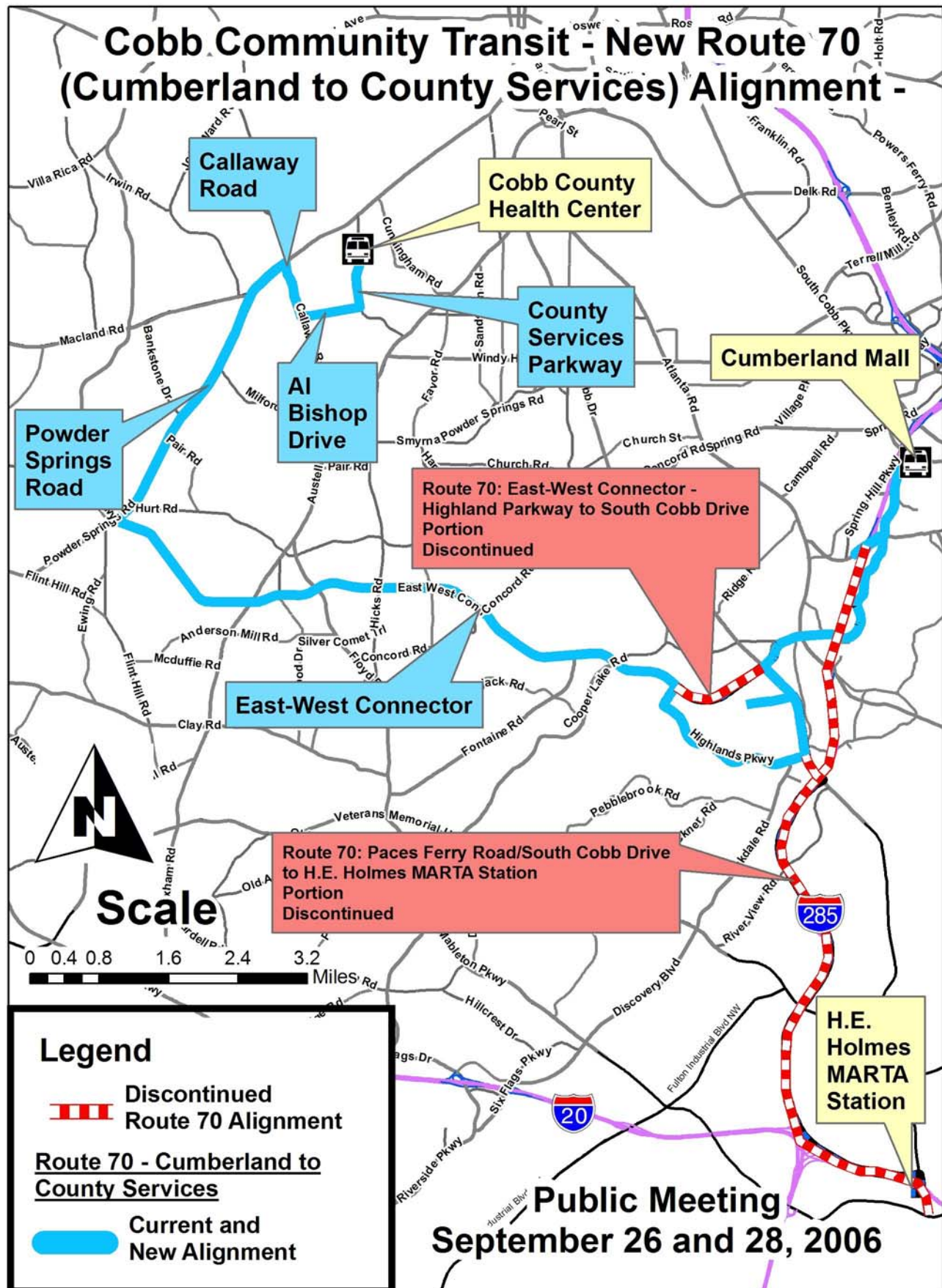
From County Services to Cumberland Transfer Center

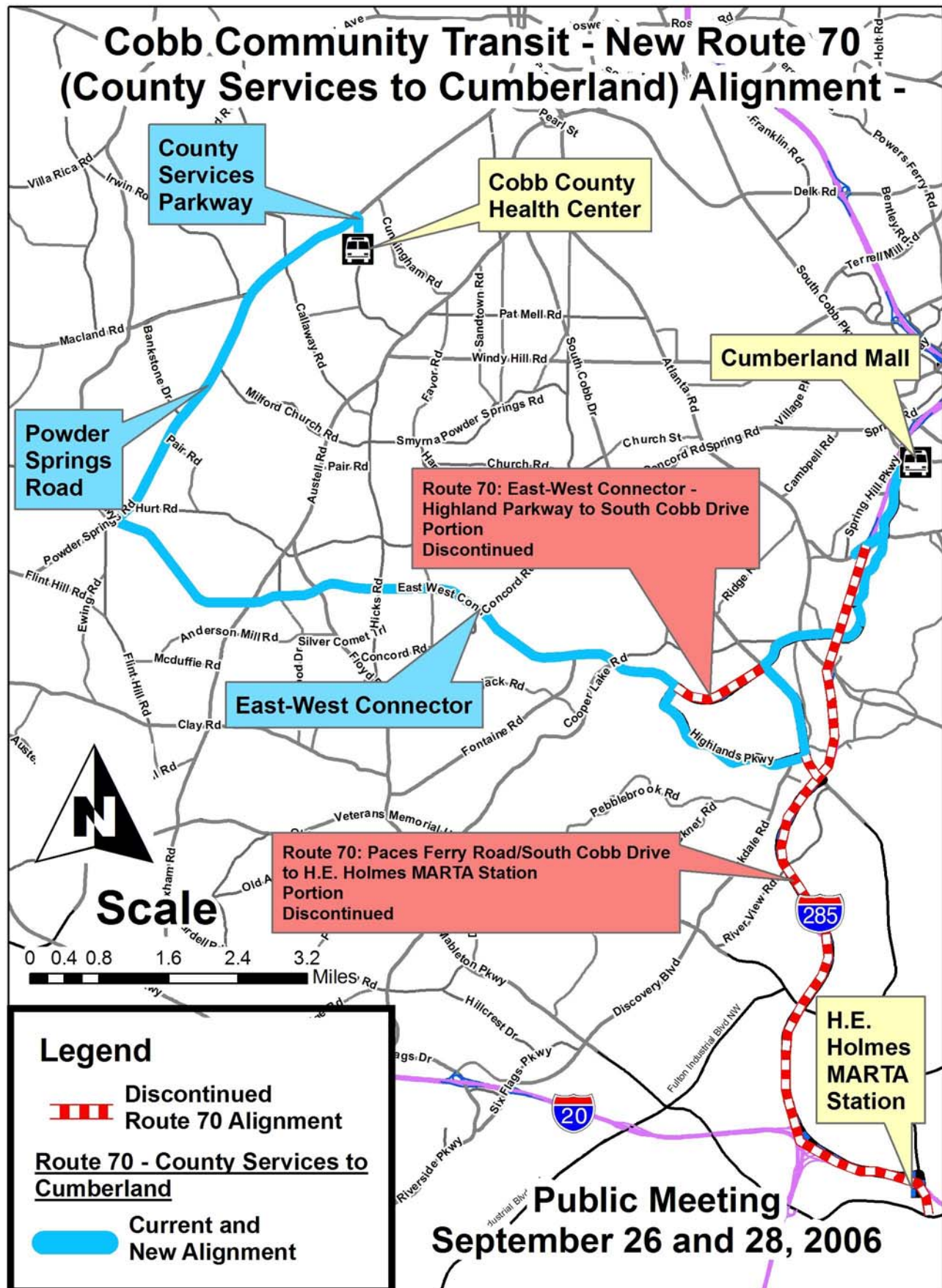
County Services Pkwy @ Health Dept.	Kroger Plaza	E/W Connector @ Brookwood	Cumberland @ S. Cobb Drive	Arrives CTC
6:30 AM	6:47 AM	6:56 AM	7:23 AM	7:45 AM
9:10 AM	9:27 AM	9:36 AM	10:03 AM	10:25 AM
11:50 AM	12:07 PM	12:16 PM	12:43 PM	1:05 PM
2:30 PM	2:47 PM	2:56 PM	3:23 PM	3:45 PM
5:10 PM	5:27 PM	5:36 PM	6:03 PM	6:25 PM
7:50 PM	8:07 PM	8:16 PM	8:43 PM	9:05 PM

Route 70 SATURDAY

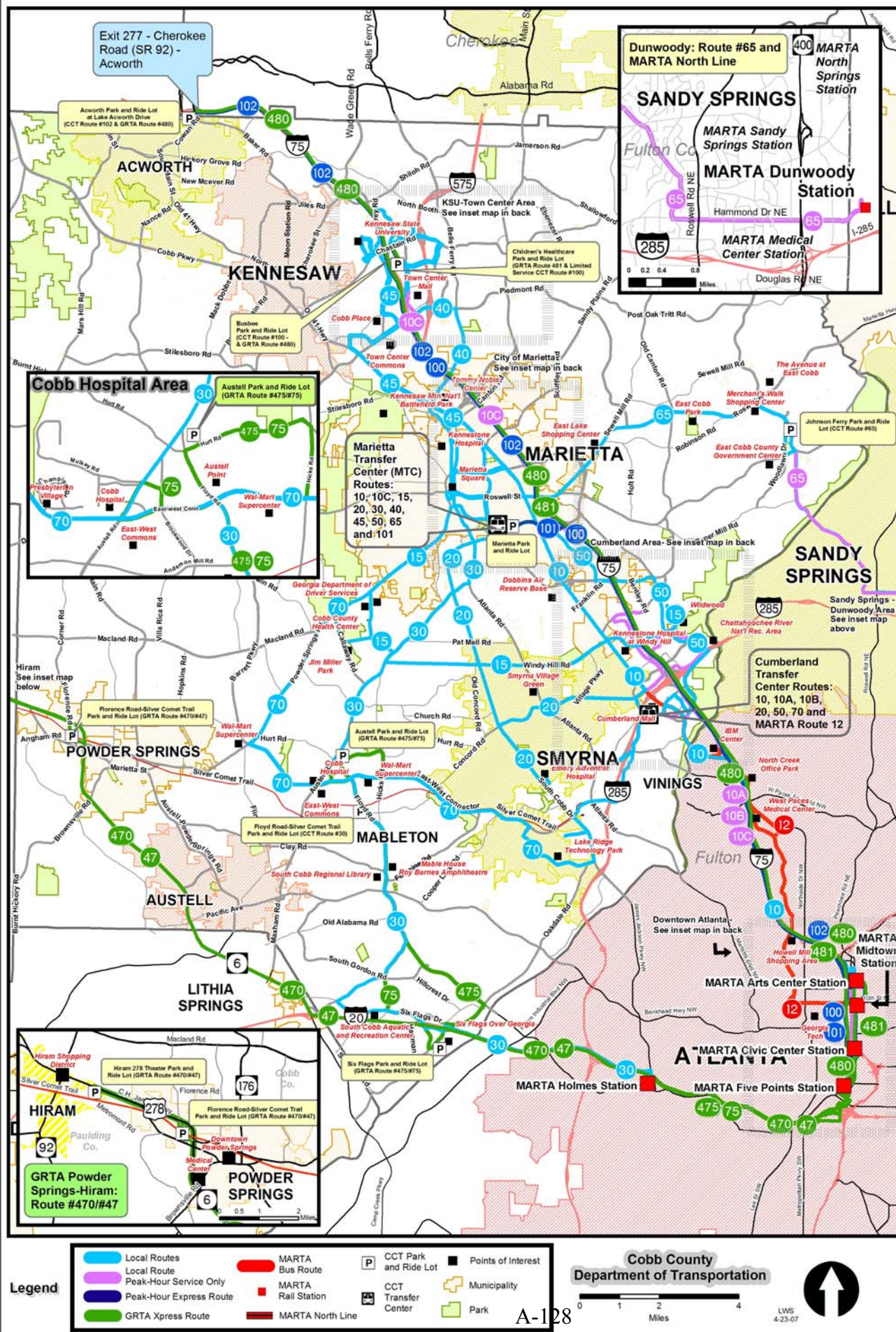
From Cumberland Transfer Center to County Services

Leaves CTC	Cumberland @ S. Cobb Drive	Brookwood Home Depot	Powder Springs Rd @ Marathon Gas Station	County Services Pkwy @ Health Dept.
7:50 AM	8:12 AM	8:39 AM	8:48 AM	9:05 AM
10:30 AM	10:52 AM	11:19 AM	11:28 AM	11:45 AM
1:10 PM	1:32 PM	1:59 PM	2:08 PM	2:25 PM
3:50 PM	4:12 PM	4:39 PM	4:48 PM	5:05 PM
6:30 PM	6:52 PM	7:19 PM	7:28 PM	7:45 PM
9:10 PM	9:32 PM	9:59 PM	10:08 PM	10:25 PM





Cobb Community Transit (CCT) System Map
May 2007





Effective 10/09/2006

Route 70 WEEKDAY

From County Services to Cumberland Transfer Center

County Services Pky @ Health Dept.	Kroger Plaza	E/W Connector @ Brookwood	Cumberland @ S. Cobb Drive	Arrives CTC
5:45 AM	6:07 AM	6:21 AM	6:48 AM	7:10 AM
6:45 AM	7:07 AM	7:21 AM	7:48 AM	8:10 AM
7:35 AM	7:57 AM	8:11 AM	8:38 AM	9:00 AM
8:45 AM	9:02 AM	9:11 AM	9:38 AM	10:00 AM
10:25 AM	10:42 AM	10:51 AM	11:18 AM	11:40 AM
11:25 AM	11:42 AM	11:51 AM	12:18 PM	12:40 PM
1:05 PM	1:22 PM	1:31 PM	1:58 PM	2:20 PM
2:05 PM	2:22 PM	2:31 PM	2:58 PM	3:20 PM
3:55 PM	4:17 PM	4:31 PM	4:58 PM	5:20 PM
5:00 PM	5:22 PM	5:36 PM	6:03 PM	6:25 PM
5:45 PM	6:02 PM	6:11 PM	6:38 PM	7:00 PM
6:45 PM	7:02 PM	7:11 PM	7:38 PM	8:00 PM
7:50 PM	8:07 PM	8:16 PM	8:43 PM	9:05 PM

Route 70 WEEKDAY

From Cumberland Transfer Center to County Services

Leaves CTC	Cumberland @ S. Cobb Drive	Brookwood Home Depot	Powder Springs Rd @ Marathon Gas Station	County Services Pky @ Health Dept.
6:15 AM	6:37 AM	7:04 AM	7:13 AM	7:30 AM
7:15 AM	7:37 AM	8:04 AM	8:13 AM	8:30 AM
8:15 AM	8:37 AM	9:04 AM	9:13 AM	9:30 AM
9:05 AM	9:27 AM	9:54 AM	10:03 AM	10:20 AM
10:05 AM	10:27 AM	10:54 AM	11:03 AM	11:20 AM
11:45 AM	12:07 PM	12:34 PM	12:43 PM	1:00 PM
12:45 PM	1:07 PM	1:34 PM	1:43 PM	2:00 PM
2:25 PM	2:47 PM	3:14 PM	3:28 PM	3:50 PM
3:25 PM	3:47 PM	4:14 PM	4:28 PM	4:50 PM
5:25 PM	5:47 PM	6:14 PM	6:23 PM	6:40 PM
6:30 PM	6:52 PM	7:19 PM	7:28 PM	7:45 PM
7:05 PM	7:27 PM	7:54 PM	8:03 PM	8:20 PM
8:05 PM	8:27 PM	8:54 PM	9:03 PM	9:20 PM
9:10 PM	9:32 PM	9:59 PM	10:08 PM	10:25 PM

Proposed Facility Site Plan