Transportation Impact Study

425 Chappell Road DRI #4295

City of Atlanta, Georgia

November 2024

Prepared for:

Atlanta BeltLine, Inc.

Prepared by:

Kimley-Horn and Associates, Inc. 1200 Peachtree Street NE, Suite 800 Atlanta, GA 30309

019927014

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EXECUTIVE SUMMARY

This report presents the analysis of the anticipated traffic impacts of the proposed *425 Chappell Road* development located in Atlanta, Georgia. The approximate 31-acre site is located south of North Avenue, east of Chappell Road, and north of Mayson Turner Road. The site is currently vacant.

The site was <u>previously reviewed as the Chappell Road DRI #3096 in June 2020</u>. The former project analyzed 710 residential units and 9,000 SF of restaurant and retail space. The Chappell Road DRI #3096 development studied three (3) driveways along North Avenue, three (3) driveways along Chappell Road, and two (2) driveways along Mayson Turner Road. At that time, the project was Approved with Conditions through the DRI review with the Atlanta Regional Commission (ARC) and Georgia Regional Transportation Authority (GRTA). The ARC Final Report was issued on July 7, 2020, and the GRTA Notice of Decision was issued on June 30, 2020.

Upon review of the <u>updated proposed 425 Chappell Road site plan in 2024</u>, the City of Atlanta and ARC determined that a new DRI review would be required for the proposed mixed-use development, due to differences in the proposed land use, trip generation, and site driveways from the previously reviewed DRI and the currently proposed development. The 425 Chappell Road development is proposed to study 770 multifamily residential units (140 low-rise units and 630 mid-rise units), 330 affordable housing units, and 5,000 SF restaurant and retail space with two site driveways along North Avenue, one site driveway along Chappell Road, and one site driveway along Mayson Turner Road.

The proposed development will consist of the following land uses and densities contained in **Table 1**. The project is expected to be completed by 2030 (approximately 6 years).

| Table 1: Proposed Land Use and Density | | | | | | |
|--|--------------------|--|--|--|--|--|
| Land Use | Proposed | | | | | |
| Multifamily Residential (Low-Rise) | 140 dwelling units | | | | | |
| Multifamily Residential (Mid-Rise) | 630 dwelling units | | | | | |
| Affordable Housing | 330 dwelling units | | | | | |
| Retail/Commercial | 5,000 SF | | | | | |

The DRI analysis includes an estimation of the overall trips projected to be generated by the development, also known as gross trips. Mixed-use, and pass-by reductions to gross trips are included in the trip generation following ITE methodologies, and alternative mode reductions are included as determined by stakeholders during the Methodology Meeting and outlined in the Georgia Regional Transportation Authority (GRTA) Letter of Understanding (dated October 9, 2024).

Capacity analyses were performed for the study intersections under the Existing 2024 conditions, the Projected 2030 No-Build conditions, and the Projected 2030 Build conditions.

- Existing 2024 conditions represent current traffic volumes collected in October 2024.
- Projected 2030 No-Build conditions represent Existing 2024 traffic volumes grown for six (6) years using a 1.5% per year growth rate plus background project trips from nearby 1060 Donald Lee Hollowell DRI #4187.
- Projected 2030 Build conditions represent the Projected 2030 No-Build conditions plus the addition of the project trips that are anticipated to be generated by the *425 Chappell Road* development.

A brief summary of system (background/No-Build) and development (Build condition) improvements and recommendations are noted below; additional details follow.

2030 NO-BUILD CONDITIONS (SYSTEM IMPROVEMENTS)

GRTA LOS standards are satisfied for all study intersections under the 2024 Existing and 2030 No-Build conditions. Therefore, no system improvements were identified.

2030 BUILD CONDITIONS (DEVELOPMENT & SITE ACCESS IMPROVEMENTS)

Chappell Road at Kennesaw Drive / Proposed Driveway C (Intersection 8)

 Construct Proposed Driveway C to operate as full movement driveway stop-control with one (1) ingress lane and one (1) egress lane.

North Avenue at Proposed Driveway A (Intersection 9)

• Construct Proposed Driveway A to operate as full movement driveway stop-control with one (1) ingress lane and one (1) egress lane.

North Avenue at Proposed Driveway B (Intersection 10)

• Construct Proposed Driveway B to operate as full movement driveway stop-control with one (1) ingress lane and one (1) egress lane.

Mayson Turner Road at Proposed Driveway D (Intersection 11)

• Construct Proposed Driveway D to operate as full movement driveway stop-control with one (1) ingress lane and one (1) egress lane.

Detailed driveway conditions are shown below under the 2030 Build conditions.

Chappell Road at Kennesaw Drive / Proposed Driveway C (Intersection 8)

| Overall LOS Standard: D Approach LOS Standard: D | | Chappell Road | | Chappell Road | | | Kennesaw Drive | | | Proposed Driveway C | | | | | |
|---|----|---------------|---------|---------------|---|---------|----------------|----|----------|------------------------|---|----------|----|---|--|
| | | Northbound | | Southbound | | | Eastbound | | | Westbound | | | | | |
| | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R | |
| | | Overall LOS | | (2.7) | | | | | | | | | | | |
| | AM | Approach LOS | A (7.9) | | | A (8.1) | | | B (12.0) | | | C (15.4) | | | |
| - | | Storage | | | | | | | | | | | | | |
| с Г | | 50th Queue | | | | | | | | | | | | | |
| INS / | | 95th Queue | 0 | | 0 | | 3 | | | 23 | | | | | |
| | | Overall LOS | | | | | | (2 | .0) | | | | | | |
| 203 (| _ | Approach LOS | | A (8.7) | | | A (8.2) | | C (21.1) | | | C (23.6) | | | |
| , , | M | Storage | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | |
| | | 95th Queue | | 0 | | | 3 | | | 3 | | | 28 | | |

Proposed Driveway C will become the 4th leg to the existing two-way stop-controlled intersection of Chappell Road at Kennesaw Drive in the City of Atlanta. The modified two-way stop-controlled intersection of Chappell Road at Kennesaw Drive / Proposed Driveway C (Intersection 8) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2030 Build traffic conditions during the AM and PM peak hours.

The recommended lane configuration for Proposed Driveway C is one lane entering the site and one lane exiting the site, as shown in the site plan. The recommended build improvements are shown in **Figure 10**.

| Overall LOS Standard: D Approach LOS Standard: D | | Proposed Driveway A | | | | North Avenue | | | North Avenue | | | | | |
|---|----|------------------------|---------|----------|------------|--------------|-----------|----|--------------|-----------|---|---|---------|---|
| | | No | orthbou | nd | Southbound | | Eastbound | | | Westbound | | | | |
| | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R |
| | | Overall LOS | | | | | | (2 | 2.3) | | | | | |
| | AM | Approach LOS | | A (9.2) | | | | | | A (0.0) | | | A (7.4) | |
| - | | Storage | | | | | | | | | | | | |
| ت ت ا | | 50th Queue | | | | | | | | | | | | |
| In Science | | 95th Queue | 5 | | | | 0 | | | 0 | | | | |
| ° F | | Overall LOS | | | | | | (0 |).8) | | | | | |
| , 203 | | Approach LOS | | B (10.0) |) | | | | | A (0.0) | | | A (7.4) | |
| | Σd | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 3 | | | | | | 0 | | | 3 | |

North Avenue at Proposed Driveway B (Intersection 9)

The proposed two-way stop-controlled intersection of North Avenue at Proposed Driveway A (Intersection 9) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2030 Build traffic conditions during the AM and PM peak hours.

The recommended lane configuration for Proposed Driveway A is one lane entering the site and one lane exiting the site, as shown in the site plan. The recommended build improvements for each alternative are shown in **Figure 10.**

North Avenue at Proposed Driveway B (Intersection 10)

| Overall LOS Standard: D Approach LOS Standard: D | | Proposed Driveway B | | | | | | North Avenue | | | North Avenue | | | | |
|---|----|------------------------|---|------------|---|---|-----------|--------------|-----|-----------|--------------|---|-------|---|--|
| | | Northbound | | Southbound | | | Eastbound | | | Westbound | | | | | |
| | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R | |
| | | Overall LOS | | (3.0) | | | | | | | | | | | |
| | | Approach LOS | | A (9.3) | | | | | | A (0.0) | | | (7.4) | | |
| - | AM | Storage | | | | | | | | | | | | | |
| ت ا ا | | 50th Queue | | | | | | | | | | | | | |
| NSC SU | | 95th Queue | 5 | | | | | 0 | | | 0 | | | | |
| ₩ 2 | | Overall LOS | | | | | | (1 | .1) | | | | | | |
| 503 | ۲ | Approach LOS | | B (10.7) |) | | | | | A (0.0) | | | (7.5) | | |
| 2 | | Storage | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | |
| | | 95th Queue | | 5 | | | | | | 0 | | | 3 | | |

The proposed two-way stop-controlled intersection of North Avenue at Proposed Driveway B (Intersection 10) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2030 Build traffic conditions during the AM and PM peak hours.

The recommended lane configuration for Proposed Driveway B is one lane entering the site and one lane exiting the site, as shown in the site plan. The recommended build improvements are shown in **Figure 10**.

| Overall LOS Standard: D Approach LOS Standard: D | |)S Standard: D .OS Standard: D | Northbound | Proposed Drive D Southbound | Mayson Turner Road Eastbound | | | Mayson Turner Road Westbound | | ner | |
|---|----|-----------------------------------|------------|-----------------------------------|------------------------------------|-----|---------|------------------------------------|---|---------|---|
| | | | L T R | L T | R | L | Т | R | L | Т | R |
| | | Overall LOS | | | | | | | | | |
| | AM | Approach LOS | | A (9.9) | A (7.4) | | | A (0.0) | | | |
| | | Storage | | | | | | | | | |
| З Г С | | 50th Queue | | | | | | | | | |
| In SC | | 95th Queue | | 8 | | | 0 | | | 0 | |
| ° F | | Overall LOS | | | (1 | .2) | | | | | |
| , (| | Approach LOS | | B (10.5) | | | A (7.6) | | | A (0.0) | |
| ? | Δ | Storage | | | | | | | | | |
| | | 50th Queue | | | | | | | | | |
| | | 95th Queue | | 5 | | | 0 | | | 0 | |

Mayson Turner Road at Proposed Driveway D (Intersection 11)

The proposed two-way stop-controlled intersection of Mayson Turner Road at Proposed Driveway D (Intersection 11) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2030 Build traffic conditions during the AM and PM peak hours.

The recommended lane configuration for Proposed Driveway D is one lane entering the site and one lane exiting the site, as shown in the site plan. The recommended build improvements are shown in **Figure 10**.

1.0 PROJECT DESCRIPTION

1.1 Introduction

This report presents the analysis of the anticipated traffic impacts of the proposed *425 Chappell Road* development located in Atlanta, Georgia. The approximate 31-acre site is located south of North Avenue, east of Chappell Road, and north of Mayson Turner Road. The project site is currently zoned PD-MU (Planned Development – Mixed Use with Conditions), BeltLine Overlay. The Rezoning Application to rezone the site as PD-MU (change in zoning conditions) was filed with the City of Atlanta Zoning Review Board in August 2024 (Permit #Z-24-058). **Figure 1** provides a location map of the project site. **Figure 2** provides an aerial view of the project site and surrounding area.

The site was <u>previously reviewed as the Chappell Road DRI #3096 in June 2020</u>. The project analyzed 710 residential units and 9,000 SF of restaurant and retail space. The Chappell Road DRI #3096 development studied three (3) driveways along North Avenue, three (3) driveways along Chappell Road, and two (2) driveways along Mayson Turner Road. At that time, the project was Approved with Conditions through the DRI review with the Atlanta Regional Commission (ARC) and Georgia Regional Transportation Authority (GRTA). The ARC Final Report was issued on July 7, 2020, and the GRTA Notice of Decision was issued on June 30, 2020.

Upon review of the <u>updated proposed 425 Chappell Road site plan in 2024</u>, the City of Atlanta and ARC determined that a new DRI review would be required for the proposed mixed-use development, due to differences in the proposed land use, trip generation, and site driveways from the previously reviewed DRI and the currently proposed development. The 425 Chappell Road development is proposed to study 770 multifamily residential units (140 low-rise units and 630 mid-rise units), 330 affordable housing units, and 5,000 SF restaurant and retail space with two site driveways along North Avenue, one site driveway along Chappell Road, and one site driveway along Mayson Turner Road.

The site is currently vacant. The proposed development will consist of the following land uses and densities contained in **Table 2**. The project is expected to be completed by 2030 (approximately 6 years).

| Table 2: Proposed Land Use and Density | | | | | | |
|--|--------------------|--|--|--|--|--|
| Land Use | Proposed | | | | | |
| Multifamily Residential (Low-Rise) | 140 dwelling units | | | | | |
| Multifamily Residential (Mid-Rise) | 630 dwelling units | | | | | |
| Affordable Housing | 330 dwelling units | | | | | |
| Retail/Commercial | 5,000 SF | | | | | |

The proposed site plan is provided in **Appendix A**. A full-sized site plan consistent with GRTA's Site Plan Guidelines is also being submitted as part of the DRI review package.

The project is considered a Development of Regional Impact (DRI) and is subject to Georgia Regional Transportation Authority (GRTA) and Atlanta Regional Commission (ARC) review due to the project size exceeding 500,000 SF of mixed-use development in the Maturing Neighborhoods Area (per UGPM). The DRI was formally triggered with the filing of the Rezoning to change the zoning conditions of the current PD-MU zoning. This Transportation Impact Study (TIS) analysis includes all inputs and methodologies discussed at the DRI Methodology Meeting with GRTA, ARC, and other stakeholders. The inputs and methodologies are outlined in the GRTA Letter of Understanding (LOU) dated October 9, 2024.







Kimley **»Horn**

425 Chappell Road DRI #4295 Transportation Analysis

Site Aerial (zoomed in)

1.2 Site Access

As currently envisioned, the proposed development will be accessible by vehicle via four (4) access points:

- 1. Site Driveway A a proposed driveway which is to be constructed as a full-movement driveway located along North Avenue, approximately 1,170 feet east of Pierce Avenue that is proposed to operate under side-street stop control.
- 2. Site Driveway B a proposed driveway which is to be constructed as a full-movement driveway located along North Avenue, approximately 730 feet east of Pierce Avenue that is proposed to operate under side-street stop control.
- 3. Site Driveway C a proposed driveway which is to be constructed as a full-movement driveway located along Chappell Road, aligning with Kennesaw Drive that is proposed to operate under side-street stop control.
- 4. **Site Driveway D** a proposed driveway which is to be constructed as a full-movement driveway located along Mayson Turner Road, approximately 245 feet east of Chappell Road that is proposed to operate under side-street stop control.

1.3 Internal Circulation Analysis

Internal private roadways throughout the site will provide access to all of the buildings and parking facilities.

1.4 Parking

The current required and proposed estimated number of site parking spaces to be provided are listed below in **Table 3**. Code requirements applicable to the site include City of Atlanta PD-MU Zoning and BeltLine Overlay minimum and maximum parking requirements. Proposed parking is an estimate and may change based on market demand. Proposed parking will be within the allowable minimum and maximum limits established by code.

| Table 3: Required and Proposed Vehicle Parking | | | | | | | |
|--|-------------------------------------|---------------|--|--|--|--|--|
| Minimum (MRC-3/BeltLine Overlay) | Maximum (MRC-3/BeltLine Overlay) | Proposed* | | | | | |
| Min: 13 | Max: 1,860 | 1,272 spaces* | | | | | |

* Proposed parking is an estimate and may change based on market demand. Proposed parking will be within the allowable maximum and maximum limits established by code.

Vehicle parking provided will be shared, where possible. Carpool and vanpool parking spaces and alternative fuel vehicle charging stations, or similar facilities, will be provided to meet city code.

Bicycle parking will also be provided on-site in addition to commuter showering facilities to meet city code.

Additional parking details are provided on the proposed site plan in **Appendix A**.

1.5 Alternative Transportation Facilities

Trail networks in the vicinity are significant and include the 2.5-mile Proctor Creek Greenway that extends north of Donald Lee Hollowell Parkway, through Westside Reservoir Park, and to the West Highlands neighborhood. The Atlanta BeltLine Westside trail is currently under construction east of the site. Programmed trail connections in the vicinity of the project include connecting BeltLine Spur Trails through the development as shown in the site plan, as well as Proctor Creek Greenway extension trails led by the Department of Atlanta Watershed Management.

The BeltLine spur trails shown on the site plan will connect directly with the Atlanta BeltLine Westside trail that is currently under construction east of the site. The Proctor Creek Greenway Master Plan originally planned an extension of the trail that exists today but lacked funds to execute the vision. Fortunately, the City of Atlanta

Watershed department has several Proctor Creek restoration projects that include trail plans that will further the Proctor Creek trail network connectivity. The Proctor Creek Wetlands at Hollowell will feature pedestrian trails and boardwalks along with wetland restoration areas west of Maddox Park and between Donald Lee Hollowell Parkway and North Avenue. The on-site BeltLine Spur trail is expected to be coordinated with the Watershed Department project to provide connectivity with the two trail projects. South of the development, the Proctor Creek Restoration at Hunter Hills will provide similar stream and wetlands restoration with greenspace and trail connections along Proctor Creek.

On-road bicycle facilities run along Donald Lee Hollowell Parkway/SR 8 north of the site connecting along the MARTA Bankhead station frontage. Bicycle facilities are also present along Joseph E Boone Boulevard east of Archer Way, where the on-road network will tie into the Westside BeltLine Trail. There are sidewalks that exist on both sides of Chappell Road, Mayson Turner Road, Donald Lee Hollowell Parkway, and Joseph E Boone Boulevard. Pierce Avenue has sidewalk facilities on the east side of the road between Donald Lee Hollowell Parkway and approximately 250 feet north of North Avenue along the frontage of the new development. There is an RRFB and pedestrian refuge to cross Donald Lee Hollowell Parkway approximately 125 feet east of Pierce Avenue, providing good pedestrian connectivity with the Bankhead MARTA station nearby.

MARTA Bus Route 853 has stops adjacent to the project site along Chappell Road. Route 50 currently serves Donald Lee Hollowell Parkway/SR 8 with stops within 0.3 of the site. Additionally, Route 51 serves Joseph E Boone Boulevard within approximately 0.24 miles of the development. The Bankhead MARTA rail station is approximately 0.5 miles away with pedestrian connectivity as noted above.

1.6 Dense Urban Environments Enhanced Focus Area

Per Section 3.2.4.2 of the GRTA Development of Regional Impact Review Procedures, the *425 Chappell Road* development is not located in dense urban environment. A Dense Urban Environment Area is defined as areas within the Midtown Community Improvement District (CID), the Central Atlanta Progress CID, or the Buckhead CID, or additional area meeting the criteria as determined by the Regional Commission or Local Government.

1.7 Heavy Vehicle Enhanced Focus Area

Per Section 3.2.4.1 of the GRTA Development of Regional Impact Review Procedures, the *425 Chappell Road* development <u>does not</u> qualify for a "Heavy Vehicle Enhanced Focus Area" review as the proposed land usage is not industrial in nature and does not generate significant heavy vehicle traffic. Therefore a "Heavy Vehicle Enhanced Focus Area" is not required for the *425 Chappell Road* mixed-use development.

2.0 TRAFFIC ANALYSES, METHODOLOGY AND ASSUMPTIONS

2.1 Study Network Determination

The study area was determined at the methodology meeting with input from GRTA, ARC, and other local agency stakeholders. The study includes the following eight (8) off-site intersections described in **Table 4** and shown in **Figure 4**.

| | Table 4: Intersection Control Summary | | | | | | | |
|----|---|----------------------|-------------------------|--|--|--|--|--|
| | Intersection | Jurisdiction | Control | | | | | |
| 1. | Donald Lee Hollowell Parkway/SR 8 at Chappell Road | City of Atlanta/GDOT | Signalized | | | | | |
| 2. | Chappell Road at North Avenue | City of Atlanta | All-Way Stop Control | | | | | |
| 3. | Chappell Road at Mayson Turner Road/West Avenue | City of Atlanta | All-Way Stop Control | | | | | |
| 4. | Joseph E Boone Boulevard at Chappell Road | City of Atlanta | Signalized | | | | | |
| 5. | Joseph E Boone Boulevard at Mayson Turner Road/Burbank Drive | City of Atlanta | Signalized | | | | | |
| 6. | North Avenue at North Avenue/Private Driveways | City of Atlanta | All-Way Stop Control | | | | | |
| 7. | Donald Lee Hollowell Parkway/SR 8 at Pierce Avenue | City of Atlanta/GDOT | Side-Street Control | | | | | |
| 8. | Chappell Road at Kennesaw Drive | City of Atlanta | Side-Street Control | | | | | |

2.2 Existing Roadway Facilities

Roadway classification descriptions and estimated Annual Average Daily Traffic (AADT) for roadway segments within the study network are provided in **Table 5** (bolded roadways are adjacent to the site).

| Та | ble 5: Roa | dway Classifica | tions | |
|------------------------------------|------------|-----------------------|----------------------|--------------------------------------|
| Roadway | Lanes | Posted Speed Limit | AADT (GDOT, 2023) | GDOT Functional Classification |
| North Avenue | 2 | 30 MPH | 1,810 | Local |
| Chappell Road | 2 | 30 MPH | 4,600 | Local |
| Mayson Turner Road | 2 | 30 MPH | 2,130 | Local |
| West Avenue | 2 | 25 MPH* | - | Local |
| Donald Lee Hollowell Parkway/ SR 8 | 3 | 35 MPH | 24,300 | Principal Arterial |
| Joseph E Boone Boulevard | 3 | 35 MPH | 5,300 | Major Collector |
| Pierce Avenue | 2 | 25 MPH* | - | Local |
| Kennesaw Drive | 2 | 25 MPH* | - | Local |
| Burbank Drive | 2 | 30 MPH | - | Local |

* Speed limit not visibly posted. Assumed to be 25 MPH.



2.3 Traffic Data Collection and Calibration

Traffic counts were collected at the eight (8) existing study intersections on Tuesday, October 8, 2024, during the AM and PM peak periods. Traffic count peak hours for all the study intersections are shown in **Table 6**. The collected peak hour turning movement traffic counts are available upon request.

| | Table 6: Traffic Cou | nt Summary | | |
|----|---|------------|----------------|----------------|
| | Intersection | Count Date | AM Peak Hour | PM Peak Hour |
| 1. | Donald Lee Hollowell Parkway/SR 8 at Chappell Road | 10/2024 | 7:45 – 8:45 AM | 4:15 – 5:15 PM |
| 2. | Chappell Road at North Avenue | 10/2024 | 7:30 – 8:30 AM | 4:30 – 5:30 PM |
| 3. | Chappell Road at Mayson Turner Road/West Avenue | 10/2024 | 7:45 – 8:45 AM | 4:00 – 5:00 PM |
| 4. | Joseph E Boone Boulevard at Chappell Road | 10/2024 | 7:30 – 8:30 AM | 4:00 – 5:00 PM |
| 5. | Joseph E Boone Boulevard at Mayson Turner Road/Burbank Drive | 10/2024 | 7:30 – 8:30 AM | 4:45 – 5:45 PM |
| 6. | North Avenue at North Avenue/Private Driveways | 10/2024 | 7:00 – 8:00 AM | 4:45 – 5:45 PM |
| 7. | Donald Lee Hollowell Parkway/SR 8 at Pierce Avenue | 10/2024 | 7:45 – 8:45 AM | 4:15 – 5:15 PM |
| 8. | Chappell Road at Kennesaw Drive | 10/2024 | 7:45 – 8:45 AM | 4:30 – 5:30 PM |

2.4 Background Growth

Background traffic is defined as expected traffic on the roadway network in future year(s) absent the construction and opening of the proposed *425 Chappell Road* development. Background traffic includes a base growth rate, which is based on historical count data and population growth data. It can also include trips anticipated from nearby or adjacent other projects.

Based on methodology outlined in the GRTA Letter of Understanding (LOU), a 1.5 percent per year background traffic growth rate from 2024 to 2030 (6 years) was used for all roadways.

The Projected 2030 No-Build conditions represent the Existing 2024 traffic volumes grown for six (6) years at 1.5% per year throughout the study network. Additionally, background traffic considered future project trips from the nearby *1060 Donald Lee Hollowell DRI #4187*. While the projected build-out of *DRI #4187* is anticipated by 2031, project trips from this development were included in future build-out years to anticipate combined impacts of the expected nearby development.

The Projected 2030 Build conditions represent the project trips generated by the *425 Chappell Road* development (discussed in Section 3.0 and 4.0) added to the Projected 2030 No-Build Conditions.

2.5 Programmed and Planned Projects

Programmed and planned projects near the project site were researched to account for any improvements or modifications within the study network expected to be installed before or by the build-out year of the development. The programmed and planned projects were discussed in the methodology meeting with GRTA, ARC, and other local stakeholders.

| | Т | able 7: Prog | rammed Proje | ects | | | |
|--|--|---------------------------|----------------|-------------------|--------------|-----------------|-----------|
| Project Name | From / To Points: | Sponsor | GDOT PI # | ARC ID # (TIP) | Design FY | ROW / UTL FY | CST FY |
| SR 8/US 278 from SR 280 to CS 6701/Stiff Street | SR 280 to CS 6701/Stiff Street | GDOT | <u>0017926</u> | - | 2022 | - | 2023 |
| Westside Trail of Atlanta BeltLine (Segment 4) | Lena Street/Washington Park to Law Street | ABI | - | - | - | - | 2023 |
| DL Hollowell Sidewalks | Proctor Creek Greenway to W Lake Avenue NW | ATL | <u>0010322</u> | - | 2022 | - | - |
| SR 8 from Proctor Creek Greenway to Atlanta BeltLine - VRU | Proctor Creek Greenway to Atlanta Beltline | GDOT | 0020200 | - | - | - | - |
| Bankhead Station Improvements/ Platform Extension | Bankhead MARTA Station | MARTA | - | - | 2024 | - | 2027 |
| CS 716/CS 3498/Marietta Blvd from CS 13/Coronet Way to SR 8 | CS 13/Coronet Way to SR 8/Donalde Lee Hollowell Parkway | GDOT | <u>0017803</u> | - | - | - | - |
| Proctor Creek Wetlands at Hollowell/at Hunter Hills | Vicinity of Proctor Creek and Maddox Park | ATL Watershed Dept. | - | - | 2024 | 2024 | - |

The following projects shown in **Table 7** are programmed to occur near the development.

*Project information was obtained from GeoPI (GDOT), the Atlanta Region's Plan (ARC), and MARTA.

The following programmed project was considered in the analysis under the specified scenarios for roadway geometry and vehicular analysis parameters:

 SR 8/US 278 from SR 280 to CS 6701/Stiff Street (PI #0017926): Existing condition – project under construction/nearing completion

It should be noted that this programmed project is not anticipated to impact the roadway geometry or signal timings that are currently in place. Thus, no updates were made from the Existing to the No-Build synchro model.

The following projects shown in Table 8 are planned to occur near the development.

| | Т | able 8: Planned | Projects | | | |
|--|---|----------------------|--------------|-------------------|---------------------|--|
| Project Name | From / To Points: | Potential Sponsor | GDOT PI # | ARC ID # (TIP) | Project Timeline | Planning Document |
| North Avenue Corridor Bus Rapid Transit | MARTA North Avenue Rail Station to MARTA Bankhead Rail Station | MARTA | - | <u>AR-491B</u> | 2041-5050 | ARC Document |
| Atlanta Streetcar – Northwest Beltline Corridor | Near Intersection of Westview Drive at Langhorn Street to MARTA Bankhead Rail Station | MARTA | - | <u>AR-490F</u> | 2041-2050 | ARC Document |
| Atlanta BeltLine Street Framework Plan – Subarea 10 | Multiple new roadway connections/ alignments in the vicinity of the project | Atlanta BeltLine | - | N/A | TBD | Atlanta BeltLine Subarea 10 Master Plan |
| MARTA Bankhead Station Transit Oriented Development | MARTA Bankhead development | MARTA | - | - | - | - |
| Envisioned City of Atlanta and BeltLine Partnership Development | Site south of North Avenue, east of Chappell Road, and north of Mayson Turner Road | City of Atlanta | - | - | - | <u>Chappell-</u> <u>Maddox Site</u> <u>Redevelopment</u> <u>Summary</u> |

It is notable that this development is adjacent to land owned by the City of Atlanta Public Works. This particular development falls within the footprint of land currently owned by the Atlanta BeltLine. However, the area plan envisioned with both the Atlanta BeltLine property and City of Atlanta Public Works property has been shared as a combined vision in the *Chappell-Maddox Site Redevelopment Summary* (August 2024). This particular DRI is subject only to the property owned by the Atlanta BeltLine. Future development on the adjacent parcel owned by the City of Atlanta is not currently programmed.

Available fact sheets for projects listed in the table above can be found in **Appendix D**.

2.6 Level-of-Service Overview

Level-of-service (LOS) is used to describe the operating characteristics of a road segment or intersection in relation to its capacity. LOS is defined as a qualitative measure that describes operational conditions and motorists' perceptions within a traffic stream. The *Highway Capacity Manual* defines six levels-of-service, LOS A through LOS F, with A being the best and F being the worst. LOS analyses were conducted at all intersections within the study network using *Synchro 12*.

LOS for signalized intersections is reported for the intersection as a whole. One or more movements at an intersection may experience a low LOS while the intersection as a whole may operate acceptably.

LOS for unsignalized intersections with all-way stop control is reported for all approaches.

LOS for unsignalized intersections with stop control on the minor street only is reported for the side street approaches and the major street left-turn movements. Low LOS for side street approaches is not uncommon, as vehicles may experience delays in turning onto a major roadway.

2.7 Level-of-Service Standards

Per GRTA Development of Regional Impact Review Procedures, a LOS standard of 'E' is applicable to the following intersections based on Study Network located within ½-mile of the Bankhead MARTA rail station:

- Donald Lee Hollowell Parkway at Chappell Road
- Donald Lee Hollowell Parkway at Pierce Avenue

All other study intersections are located in the Maturing Neighborhoods area as specified in the Atlanta Regional Commission's Unified Growth Policy Map. Therefore, for the purposes of this traffic analysis, a LOS standard of D was assumed for all other intersections not listed above per section 3.2.2.1 of the GRTA *Development of Regional Impact Review Procedures,* and as specified in the LOU.

GDOT Intersection Control Evaluation (ICE) Stage 1 is required for GDOT-maintaining intersections or approaches that do not meet LOS standards and where the project is increasing trips to the approach by twenty (20) percent or more.

• It is notable that GDOT-maintained intersections studied for this DRI meet LOS standards and therefore do not require GDOT ICE Stage 1.

3.0 TRIP GENERATION

Gross trips associated with the proposed development were estimated using the *Institute of Transportation Engineers' (ITE) Trip Generation Manual, 11th Edition*, using equations and rates as documented in the Methodology Meeting Packet and discussed in the Methodology Meeting. Reductions to gross trips including mixed-use reductions, alternative transportation mode reductions, and pass-by reductions for retail uses are considered in the analysis based on methodology outlined in the GRTA Letter of Understanding (LOU).

Mixed-use reductions occur when a site has a combination of different land uses that interact with one another. For example, people living in a residential development may walk to the restaurants and retail instead of driving offsite or to the site. This reduces the number of vehicle trips that will be made on the roadway, thus reducing traffic congestion. Mixed-use reductions were taken in this analysis per the LOU.

Alternative modes reductions are taken when a site can be accessed by modes other than vehicles (walking, bicycling, transit, etc.). Alternative modes reductions were taken in this analysis per the LOU.

Pass-by reductions are considered when traffic already traveling along a roadway may choose to visit a retail or restaurant establishment that is along the vehicle's path. These trips were already on the road and would continue to travel the same route regardless of the build-out of the new development. Therefore, the pass-by trips visiting retail and restaurants would not be a new trip on the adjacent roadway but would contribute to new trips on the driveways. Pass-by reductions were taken in this analysis per the LOU.

| Table 9: Trip Generation | | | | | | | | | | | | | |
|---|-----------------------|----------|------------------------|-------|--------|--------|--------|--------|--|--|--|--|--|
| Landllas | Density | D | aily Traffi | С | AM Pea | k Hour | PM Pea | k Hour | | | | | |
| Land Use | Density | Total | Enter | Exit | Enter | Exit | Enter | Exit | | | | | |
| | | Proposed | I Project ⁻ | Trips | | | | | | | | | |
| 220 - Multifamily Housing (Low-Rise) | 140 dwelling units | 972 | 486 | 486 | 16 | 50 | 51 | 30 | | | | | |
| 221 - Multifamily Housing (Mid-Rise) | 630 dwelling units | 2,958 | 1,479 | 1,479 | 61 | 205 | 150 | 96 | | | | | |
| 223 – Affordable Housing | 330 dwelling units | 1,370 | 685 | 685 | 25 | 62 | 90 | 62 | | | | | |
| 822 – Strip Retail Plaza (<40K) | 5,000 SF | 440 | 220 | 220 | 7 | 5 | 24 | 24 | | | | | |
| Gross Project 1 | Trips | 5,740 | 2,870 | 2,870 | 109 | 322 | 315 | 212 | | | | | |
| Mixed-U | Use Reductions | -88 | -44 | -44 | -2 | -2 | -8 | -8 | | | | | |
| Alternative Mode Re | eductions (25%) | -1,413 | -707 | -706 | -27 | -80 | -77 | -52 | | | | | |
| Pass-by Reduction | s (Retail - 40%) | -118 | -59 | -59 | 0 | 0 | -6 | -6 | | | | | |
| New Trips | | 4,121 | 2,060 | 2,060 | 80 | 240 | 224 | 146 | | | | | |

Table 9 summarizes the gross trip generation, reductions, net trip generation, and driveway volumes for the proposed 425 Chappell Road DRI development.

A more detailed trip generation analysis summary table is provided in Appendix B.

4.0 TRIP DISTRIBUTION AND ASSIGNMENT

The distribution of new project trips was based on the project land uses, a review of land use densities and road facilities in the area, engineering judgement, and methodology discussions with GRTA, ARC, GDOT, City of Atlanta, Atlanta BeltLine and other local stakeholders.

The anticipated distribution and assignment of the trips throughout the study roadway network for non-residential land uses is shown in **Figure 5.** The anticipated distribution and assignment of the trips throughout the study roadway network for residential land uses is shown in **Figure 6**. These trip assignment percentages were applied to the net project trips expected to be generated by the development, and the volumes were assigned to the roadway network. The peak hour project trips are shown by turning movement throughout the study network in **Figure 7**.

Detailed intersection volume worksheets are provided in Appendix C.

5.0 TRAFFIC ANALYSIS

Capacity analyses were performed using *Synchro 12* for the AM and PM peak hours under the Existing 2024 conditions, 2030 No-Build conditions, and 2030 Build conditions. The capacity analyses were performed using methodologies from the *Highway Capacity Manual (HCM), 6th Edition* unless otherwise noted.

These analyses included existing roadway laneage for each of the scenarios because no programmed roadway geometry changes were identified that would be installed prior to the 2030 build out of the development. The traffic volumes and roadway laneage used for each scenario are shown in **Figure 8** for Existing 2024 conditions, **Figure 9** for 2030 No-Build conditions, and **Figure 10** for 2030 Build conditions.

Sections 5.1 – 5.11 provide the results of the capacity analyses are presented for each study intersection and site driveway including projected LOS, delay, and queue lengths.







5.1 Donald Lee Hollowell Parkway/SR 8 at Chappell Road (Intersection 1)

Intersection 1 is located within ½-mile of the Bankhead MARTA station, therefore per GRTA *Development of Regional Impact Review Procedures,* Section 3.2.2.1 guidance, the LOS standard for the study intersection is LOS E.

| Over Appro | rall L(bach l | DS Standard: E LOS Standard: E | Cł | nappell Rd | Chappell | Rd | D Pa | onald Lee Hollowell rkway/SR 8 | Do H Par | onald Lee Hollowell kway/SR 8 |
|----------------------|--|-----------------------------------|-----|------------|----------|-------|-----------|--------------------------------------|----------------|-------------------------------------|
| | | | N | orthbound | Southbou | nd | E | astbound | W | estbound |
| | | Overall LOS | L | IK | | | | IK | L | IK |
| | | | | E (72 0) | A (0.0) | Б(Т | 9.4) | P (11 1) | | A (7 2) |
| (1) | Σ | Storago | | 125 | A (0.0) | 75 | 75 | | 175 | A (7.3) |
| ĬŽ 🦳 | ◄ | 50th Queue | 65 | 125 | 0 | 75 | 75 | 209 | 25 | 72 |
| AL | | O5th Queue | 110 | 195 | 0 | 0 | 0 | 590 | - 30 - 60 | 120 |
| XX | | | 110 | 310 | 0 | | 4 1) | 500 | 00 | 130 |
| SIC SIC | | | | E (77 2) | A (0 0) | Б(І | 4.1) I | A (1 2) | | A (7.6) |
| 05 | Σ | Appilacii LOS | | 125 | A (0.0) | 75 | 75 | A (4.2) | 175 | A (7.0) |
| ~ | ₽ | 50th Quouo | 109 | 120 | 0 | 75 | 75 | 108 | 59 | 279 |
| | | 95th Queue | 100 | 223 | 0 | 0 | 0 | 103 | 105 | /13 |
| | 95th Queue Overall LOS Approach LOS Storage | | 130 | 225 | 0 | | 23.4) | 195 | 105 | 415 |
| | | Approach LOS | | E (72.8) | | 0 (2 | <u></u> | B (175) | | B (10.5) |
| Δ | Σ | Storage | | 125 | A (0.0) | 75 | 75 | | 175 | B (10.3) |
| -BUILD VAL) AM | ◄ | 50th Oueue | 70 | 213 | 0 | 0 | 0 | 640 | 55 | 98 |
| ₽Ë | | 95th Queue | 125 | 333 | 0 | 0 | 0 | 848 | 98 | 175 |
| ġ ģ | | Overall LOS | 120 | 000 | Ū | B (1 | 61) | 010 | 00 | 170 |
| I SI | | Approach LOS | | E (76.5) | A (0.0) | | 0.1) | A (5.1) | | B (11.2) |
| 503 | Σ | Storage | | 125 | /(0.0) | 75 | 75 | | 175 | |
| | α. | 50th Queue | 118 | 140 | 0 | 0 | 0 | 143 | 80 | 438 |
| | | 95th Queue | 205 | 235 | 0 | 0 | 0 | 243 | 143 | 610 |
| | | Overall LOS | | | | B (1 | 8.2) | 1 | | |
| | | Approach LOS | | E (76.5) | A (0.0) | - (' | | B (11.7) | | A (7.4) |
| | Σ | Storage | | 125 | · · · · | 75 | 75 | | 175 | |
| L L L L | | 50th Queue | 100 | 138 | 0 | 0 | 0 | 455 | 53 | 68 |
| | | 95th Queue | 183 | 235 | 0 | 0 | 0 | 630 | 93 | 123 |
| | | Overall LOS | | | | B (1 | 2.5) | | | |
| (S | 5 | Approach LOS | | E (74.9) | A (0.0) | | | A (4.7) | | B (10.3) |
| | P | Storage | | 125 | - | 75 | 75 | | 175 | |
| | | 50th Queue | 135 | 13 | 0 | 0 | 0 | 135 | 95 | 393 |
| | | 95th Queue | 233 | 20 | 0 | 0 | 0 | 233 | 170 | 555 |

The existing signalized intersection of Donald Lee Hollowell Parkway/SR 8 at Chappell Road (Intersection 1) is projected to meet GRTA's standards per approach and for the <u>overall</u> LOS under the 2024 Existing conditions, 2030 No-Build conditions, and 2030 Build conditions during the AM and PM peak hours.

| Over | all LC | S Standard: D | Ch | appell R | oad | Ch | appell Ro | bad | No | rth Aver | nue | No | orth Aver | iue |
|-----------------------|--------|----------------|-----|-----------------|-----|----|-----------------|-----------|------|-----------------|-----|----|-----------------|------|
| Appro | ach L | OS Standard: D | N N | lorthbou | nd | S | outhbour | <u>nd</u> | E | astboun | d | | /estbour | nd D |
| | | 0 | L | | R | L | | <u> </u> | L | | К | L | | К |
| | | Overall LOS | | | | | A (0 7) | В (1 | 0.4) | A (0 E) | | 1 | A (0.0) | |
| | 5 | Approach LOS | | B (11.6) | | | A (8.7) | | | A (8.5) | | | A (9.0) | |
| ŮN O | A | Storage | | | | | | | | | | | | |
| E CO | | 50th Queue | | | | | | | | - | | | | |
| NS NS | | 95th Queue | | 58 | | | 15 | | > | 8 | | | 8 | |
| ΞA | | Overall LOS | | | | | | C (1 | 6.0) | | | | | |
|)24 | 5 | Approach LOS | | <u>C (15.9)</u> | | | <u>C (15.6)</u> | | | <u>B (10.1)</u> | | | <u>C (17.2)</u> | |
| 2(| đ | Storage | | | | | | | | | | | | |
| | | 50th Queue | | - | | | | | | | | | | |
| | | 95th Queue | | 78 | | | 83 | | | 8 | | | 93 | |
| | | Overall LOS | | | | | | B (1 | 1.0) | | | | | |
| 30 NO-BUILD (AWSC) | _ | Approach LOS | | B (12.6) | | | A (8.9) | | | A (8.7) | | | A (9.2) | |
| | ₹ | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 70 | | | 18 | | | 8 | | | 10 | |
| | | Overall LOS | | | | | | C (1 | 9.8) | | | | | |
| | _ | Approach LOS | | C (19.5) | | | C (19.3) | | | B (10.8) | | | C (21.7) | |
| 20 | Σ | Storage | | | | | | | | | | | | |
| | _ | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 108 | | | 113 | | | 8 | | | 128 | |
| | | Overall LOS | | | | | | B (1 | 2.3) | | | | | |
| | | Approach LOS | | B (14.7) | | | A (9.6) | | | A (9.2) | | | A (9.8) | |
| - | Σ | Storage | | | | | | | | | | | | |
| 2030 BUILD (AWSC) | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 93 | | | 23 | | | 10 | | | 15 | |
| | | Overall LOS | | | | | | D (2 | 7.3) | | | | | |
| | | Approach LOS | | D (25.4) | | | D (30.0) | | | B (12.1) | | | D (28.6) | |
| | Σ | Storage | | | | | | | | | | | | |
| | _ | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 143 | | | 188 | | | 13 | | | 168 | |

5.2 Chappell Road at North Avenue (Intersection 2)

The existing all-way stop-controlled intersection of Chappell Road at North Avenue (Intersection 2) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2024 Existing conditions, 2030 No-Build conditions, and 2030 Build conditions during the AM and PM peak hours.

5.3 Chappell Road at Mayson Turner Road / West Avenue (Intersection 3)

| Over | all LC | S Standard: D | Ch | appell R | oad | Cha | appell Ro | bad | W | est Aven | ue | Mayso | n Turnei | r Road |
|-------------------------|--------|----------------------------------|----|-----------|----------|-----|-----------|------|------|----------|----|-------|----------|--------|
| Appro | ach L | OS Standard: D | N | lorthbour | nd | S | outhbour | nd | E | astboun | d | N | /estboun | nd |
| | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R |
| | | Overall LOS | | | | | | A (9 | 9.2) | | | | | |
| | _ | Approach LOS | | A (9.5) | | | A (9.3) | | | A (7.8) | | | A (7.9) | |
| <u>U</u> | AN | Storage | | | | | | | | | | | | |
| Ē | | 50th Queue | | | | | | | | | | | | |
| ISC IS | | 95th Queue | | 33 | | | 30 | | | 0 | | | 8 | |
| ₽₹ | | Overall LOS | | | | | | B (1 | 2.6) | | | | | |
| 24 (| | Approach LOS | | A (9.5) | | | B (14.8) | | | A (8.6) | | | A (9.0) | |
| 20 | Σd | Storage | | | | | | | | | | ĺ | | |
| | | 50th Queue | | | | | | | | | | ĺ | | |
| | | 95th Queue | | 28 | | | 110 | | | 0 | | | 15 | |
| | | Overall LOS | | | | | | A (9 | 9.6) | | | | | |
| 2030 NO-BUILD (AWSC) | | Approach LOS | | A (9.9) | | | A (9.7) | | | A (8.0) | | | A (8.1) | |
| | Δ | Storage | | | | | | | | | | ĺ | | |
| | | 50th Queue | | | | | | | | | | ĺ | | |
| | | 95th Queue | | 40 | | | 35 | | | 0 | | ĺ | 8 | |
| | | Overall LOS | | | | | | B (1 | 4.3) | | | | | |
| | | Approach LOS | | A (10.0) | | | C (17.3) | | | A (8.8) | | | A (9.3) | |
| | Σd | Overall LOS Approach LOS Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 30 | | | 140 | | | 0 | | | 18 | |
| | | Overall LOS | | | | | | B (1 | 0.5) | | | | | |
| | _ | Approach LOS | | B (10.6) | | | B (10.9) | | | A (8.3) | | | A (8.7) | |
| | AN | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| 2030 BUIL (AWSC) | | 95th Queue | | 48 | | | 53 | | | 0 | | | 10 | |
| | | Overall LOS | | | | | | C (1 | 6.9) | | | | | |
| | _ | Approach LOS | | B (11.4) | r | | C (21.4) | | | A (9.2) | | | A (9.9) | |
| | PΖ | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 48 | | | 180 | | | 0 | | | 20 | |

The existing all-way stop-controlled intersection of Chappell Road at Mayson Turner Road / West Avenue (Intersection 3) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2024 Existing conditions, 2030 No-Build conditions, and 2030 Build conditions during the AM and PM peak hours.

5.4 Joseph E Boone Boulevard at Chappell Road (Intersection 4)

| Over Appro | all LC | DS Standard: D .OS Standard: D | Chapp | ell Road | Ch | appell R | oad | Jose E | eph E Bo Boulevaro | one 1 | Jose E | eph E B Boulevai | oone d |
|---------------|---------------------------------------|-----------------------------------|-------|----------|---------|----------|------|-----------------|-----------------------|----------|-----------|---------------------|-----------|
| | | | Nortl | nbound | S | outhbou | nd | E | astboun | b | N | /estbou | nd |
| | | | L | T R | L | Т | R | L | Т | R | L | Т | R |
| | | Overall LOS | | | | | C (2 | 23.3) | | | | | |
| | _ | Approach LOS | D (| 35.3) | | C (26.3 |) | | B (14.3) | | | B (18.2) |) |
| 9 9 | AN | Storage | | | | | | 100 | | | | | |
| ĒĴ | | 50th Queue | 1 | 45 | | 50 | | 13 | 73 | 3 | 15 | 6 | 63 |
| SIS 2 | | 95th Queue | 2 | 245 | | 88 | | 23 | 13 | 0 | 28 | 1 | 10 |
| ΞŚ | | Overall LOS | | | | | C (2 | 23.8) | | | 1 | | |
|)24 (\$ | F | Approach LOS | C (| 23.2) | | C (31.9 |) | | B (12.4) | | | C (20.3 |) |
| 50 | P | Storage | | | | | | 100 | | | | | |
| | | 50th Queue | | 78 | | 173 | | 3 | 35 | 5 | 23 | ç | 8 |
| | | 95th Queue | 1 | 40 | | 280 | | 5 | 60 |) | 43 | 1 | 75 |
| | | Overall LOS | | | | | C (2 | 24.3) | | | 1 | | |
| - | F | Approach LOS | D (| 38.1) | | C (26.6 |) | | B (15.8) | | | B (19.6) |) |
| 2 | Approach LOS Storage 50th Queue | | | | | | | 100 | | | | | |
| Ľ Ĩ | Storag 50th Queu 95th Queu | | 1 | 68 | | 53 | | 13 | 10 | 5 | 15 | 8 | 35 |
| - N | | 95th Queue | 2 | 275 | | 98 | | 25 | 19 | 0 | 30 | 1 | 53 |
| Σö | | Overall LOS | | | | | C (2 | 24.9) | | | 0 (00 4) | | |
| (S 33 | - | Approach LOS | C (| | C (33.3 |) | | <u>B (14.0)</u> | | C (23.4) | | | |
| 20 | P | Storage | | | | | | 100 | | | | | |
| | | 50th Queue | | 85 | | 195 | | 3 | 50 |) | 28 | 1. | 48 |
| | | 95th Queue | 1 | 53 | | 310 | | 8 | 88 | 3 | 48 | 2 | 40 |
| | | Overall LOS | | | | | C (2 | 25.3) | | | 1 | | |
| | F | Approach LOS | D (| 39.4) | | C (29.5 |) | | B (15.6) | | | B (19.6) |) |
| | AN | Storage | | | | | | 100 | | | | | |
| ΓË | | 50th Queue | 1 | 78 | | 93 | | 18 | 10 | 5 | 15 | 8 | 35 |
| B | | 95th Queue | 2 | 288 | | 168 | | 30 | 19 | 0 | 30 | 1 | 53 |
| 800 | Overall LOS | | | | | | C (2 | 26.5) | | | 1 | | |
| 50 | - | Approach LOS | C (| 22.1) | | D (37.4 |) | | <u>B (14.4)</u> | | | <u>C (24.2</u> |) |
| | Р | Storage | | | | | | 100 | | | | | |
| | | 50th Queue | | 95 | | 233 | | 13 | 50 |) | 28 | 1 | 50 |
| | | 95th Queue | 1 | 73 | | 358 | | 20 | 93 | 3 | 50 | 2 | 40 |

The existing signalized intersection of Northside Drive/SR 3 at Joseph E Boone Boulevard at Chappell Road (Intersection 4) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2024 Existing conditions, 2030 No-Build conditions, and 2030 Build conditions during the AM and PM peak hours.

5.5 Joseph E Boone Boulevard at Mayson Turner Rd / Burbank Drive (Intersection 5)

| Over Appro | all LC ach L | DS Standard: D .OS Standard: D | Βι | ırbank l | Drive | 1 | Maysor Ro | n Tui bad | ner | Jos | eph E E Bouleva | loone rd | Jos E | eph E Bo Boulevar | oone d |
|---------------|---|-----------------------------------|----|----------|-------|-------|--------------|--------------|------|------|--------------------|-------------|----------|----------------------|-----------|
| | | | N | lorthbo | und | | South | nbou | nd | E | astbou | nd | V | Vestbour | nd |
| | | | L | Т | R | L | - | Т | R | L | Т | R | L | Т | R |
| | | Overall LOS | | | | | | | Α (| 8.8) | | | 1 | | |
| | F | Approach LOS | | C (28. | 0) | | C (2 | 28.3) | | | A (0.6 |) | | A (6.4) | |
| 5 D | AN | Storage | | | | | | | | | | | | | |
| L J | | 50th Queue | | 40 | | | 4 | 15 | | | 5 | | 28 | | 25 |
| KIS N⊿ | | 95th Queue | | 73 | | | 8 | 30 | | | 10 | | 50 | | 48 |
| ΞÖ | | Overall LOS | | | | | | | A (| 9.8) | | | | | |
|)24 (S | _ | Approach LOS | | C (23. | 0) | | C (2 | 24.6) | | | A (0.3 |) | | A (7.1) | |
| 20 | Approach LOS Storage 50th Queue 95th Queue Overall LOS Approach LOS Storage | | | | | | | | | | | | | | |
| | 50th Queu 95th Queu 95th Queu Overall LO Approach LO Storag | | | 25 | | | 4 | 18 | | | 3 | | 35 | | 33 |
| | | 95th Queue | | 48 | | | 8 | 35 | | | 5 | | 60 | | 58 |
| | | Overall LOS | | | | | Α (| 8.4) | | | | | | | |
| | _ | Approach LOS | | C (28.4 | 4) | | C (2 | 28.6) | | | A (0.9 |) | | A (6.6) | |
| 2 | AN | Storage | | | | | | | | | | | | | |
| Ľ ũ | - | 50th Queue | | 45 | | | 4 | 18 | | | 8 | | 33 | | 33 |
| A A | | 95th Queue | | 80 | | 88 13 | | | | | | 60 | | 58 | |
| N D | | Overall LOS | | | | | | | A (| 9.6) | | | - | | |
| 30 (S | _ | Approach LOS | | C (23. | 1) | | C (2 | 25.0) | | | A (0.4 |) | | A (7.5) | |
| 20 | РΝ | Storage | | | | | | | | | | | | | |
| | | 50th Queue | | 28 | | | 5 | 53 | | | 3 | | 45 | | 40 |
| | | 95th Queue | | 53 | | | 9 | 95 | | | 5 | | 80 | | 75 |
| | | Overall LOS | | | | | | | A (| 9.8) | | | | | |
| | _ | Approach LOS | | C (28.3 | 3) | | C (3 | 31.5) | | | A (0.9 |) | | A (6.7) | |
| | AN | Storage | | | | | | | | | | | | | |
| L L | | 50th Queue | | 45 | | | 8 | 30 | | | 8 | | 35 | | 35 |
| NA NU | | 95th Queue | | 80 | | | 14 | 43 | | | 13 | | 63 | | 63 |
| 10 E | | Overall LOS | | | | | | | В (1 | 0.3) | | | | | |
| 203 (S | | Approach LOS | | C (23. | 1) | | C (2 | 26.3) | | | A (0.4) |) | | A (7.8) | |
| | PM | Storage | | | | | | | | | | | | | |
| | _ | 50th Queue | | 28 | | | 6 | 8 | | | 3 | | 50 | | 45 |
| | | 95th Queue | | 50 | | | 12 | 20 | | | 5 | | 90 | | 83 |

The existing signalized intersection of Joseph E Boone Boulevard at Mayson Turner Road / Burbank Drive (Intersection 5) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2024 Existing conditions, 2030 No-Build conditions, and 2030 Build conditions during the AM and PM peak hours.

5.6 North Avenue at North Avenue / Private Driveways (Intersection 6)

| Over | all LC | S Standard: D | Priv | ate Drive | eway | No | rth Aver | ue | Nc | orth Aven | ue | Priva | ate Drive | way |
|----------------------------------|--------|----------------|------|-----------|------|----|----------|------|------|-----------|----|-------|-----------|-----|
| Appro | ach L | OS Standard: D | N | lorthbou | nd | Sc | outhbour | nd | E | astboun | d | W | /estboun | d |
| | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R |
| | | Overall LOS | | | | | | A (7 | 7.6) | | | | | |
| | _ | Approach LOS | | A (7.4) | | | A (7.5) | | | A (7.4) | | | A (8.1) | |
| 5 | AN | Storage | | | | | | | | | | | | |
| Ē | | 50th Queue | | | | | | | | | | | | |
| (IS | | 95th Queue | | 3 | | | 3 | | | 5 | | | 5 | |
| β₩ | | Overall LOS | | | | | | A (8 | 3.1) | | | | | |
| 24 (| _ | Approach LOS | | A (7.7) | • | | A (8.1) | | | A (8.0) | | | A (7.5) | |
| 20 | РΝ | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 3 | | | 35 | | | 5 | | | 0 | |
| | | Overall LOS | | | | | | A (7 | 7.6) | | | | | |
| | _ | Approach LOS | | A (7.4) | | | A (7.5) | | | A (7.4) | | | A (8.2) | |
| 2030 NO-BUILD (AWSC) PM AM | Δ | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 3 | | | 3 | | | 5 | | | 5 | |
| | | Overall LOS | | | | | | 8) A | 3.3) | | | | | |
| | _ | Approach LOS | | A (7.7) | | | A (8.4) | | | A (8.1) | | | A (7.6) | |
| | Δd | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 3 | | | 40 | | | 5 | | | 0 | |
| | | Overall LOS | | | | | | A (7 | 7.9) | | | | | |
| | _ | Approach LOS | | A (7.6) | | | A (7.7) | | | A (8.0) | | | A (8.3) | |
| | AN | Storage | | | | | | | | | | | | |
| Э Ц | | 50th Queue | | | | | | | | | | | | |
| ng Ng | | 95th Queue | | 3 | | | 5 | | | 13 | | | 5 | |
| 0 ¥ | | Overall LOS | | | | | | A (8 | 3.9) | | | | | |
| 2030 E (AW | _ | Approach LOS | | A (7.9) | • | | A (9.0) | | | A (8.6) | | | A (7.8) | |
| | PR | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 3 | | | 50 | | | 10 | | | 0 | |

The existing all-way stop-controlled intersection of North Avenue at North Avenue / Private Driveways (Intersection 6) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2024 Existing conditions, 2030 No-Build conditions, and 2030 Build conditions during the AM and PM peak hours.

5.7 Donald Lee Hollowell Parkway (SR 8) at Pierce Avenue / Private Driveway (Intersection 7)

Intersection 7 is located within ½-mile of the Bankhead MARTA station, therefore per GRTA *Development of Regional Impact Review Procedures,* Section 3.2.2.1 guidance, the LOS standard for the study intersection is LOS E.

| Over Appro | all LC ach L | OS Standard: E .OS Standard: E | Pie | rce Ave | enue | Priva | ate Drive | eway | D Hollo | onald Le well Park (SR 8) | e way | D Hollo | onald Le well Parl (SR 8) | e kway |
|---------------|--|-----------------------------------|-----|---------|------|-------|-----------|------|------------|---------------------------------|----------|------------|---------------------------------|-----------|
| | | | No | orthbou | und | Sc | outhbou | nd | E | astbound | b | V | /estboun | d |
| - | - | | L | Т | R | L | Т | R | L | Т | R | L | Т | R |
| | | Overall LOS | | | | | | (0 | .5) | | | 1 | | |
| | CONTRACTOR OF CO | | | D (27.6 | 5) | | A (0.0) | | | A (0.0) | | | B (11.5) | |
| <u>0</u> | AN | Storage | | | | | | | 150 | | | 150 | | |
| É | | 50th Queue | | | | | | | | | | | | |
| ISC IS | | 95th Queue | | 13 | | | 0 | | 0 | | | 0 | | |
| ≌≥ | COVERAIL LO Approach LO Stora 50th Que 95th Que Overall LO | | | | | | | (0 | .3) | | | | | |
| 24 | Coverall LC Approach LC Storag 50th Queu 95th Queu Overall LC Overall LC | | Ţ | C (21.4 | 4) | | A (0.0) | | | A (0.0) | | | A (9.0) | |
| 20 | Σ | Storage | | | | | | | 150 | | | 150 | | |
| | e Free Content of Cont | | | | | | | | | | | | | |
| | | 95th Queue | | 8 | | | 0 | | 0 | | | 3 | | |
| | Overall LO Approach LO Storag | | | | | | | (0 | .6) | | | | | |
| | | Approach LOS | | E (36.0 |)) | | A (0.0) | • | | A (0.0) | | | B (12.6) | |
| Q | Σ | Storage | | • | 1 | | | | 150 | | | 150 | | |
| | | 50th Queue | | | | | | | | | | | | |
| SCB | | 95th Queue | | 18 | | | 0 | | 0 | | | 0 | | |
| ₽ž | | Overall LOS | 10 | | (0, | | | .4) | | | | • | | |
| | | Approach LOS | | D (26.1 | 1) | | A (0.0) | | | A (0.0) | | | A (9.3) | |
| 20 | Σ | Storage | | | ' | | | | 150 | | 150 | | | |
| | - | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 13 | | | 0 | | 0 | | | 3 | | |
| | | Overall LOS | | | | | - | (0 | .6) | | | | | |
| | | Approach LOS | | E (37.9 | 9) | | A (0.0) | | / | A (0.0) | | | B (12.9) | |
| | Σ | Storage | | | / | | | | 150 | | | 150 | 1 (| |
| 2 | 4 | 50th Queue | | | | | | | | | | | | |
| SC | | 95th Queue | | 20 | | | 0 | | 0 | | | 0 | | |
| °≥ | | Overall LOS | | | | | | (0 | .4) | | | | | |
| 2030 I (TM | | Approach LOS | | D (27.2 | 2) | | A (0.0) | | / | A (0.0) | | | A (9.4) | |
| 2 | Σ | Storage | | | , | | . , | | 150 | | | 150 | 1 | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 13 | | | 0 | | 0 | | | 3 | | |

The existing two-way stop-controlled intersection of Donald Lee Hollowell Parkway (SR 8) at Pierce Avenue / Private Driveway (Intersection 7) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2024 Existing conditions, 2030 No-Build conditions, and 2030 Build conditions during the AM and PM peak hours.

| Overall LOS Standard: D Approach LOS Standard: | | | Chappell Road | | | | Cha | ppell R | load | Ker | nesaw [| Drive | Proposed Driveway C | | | |
|---|----|--------------|---------------|---------|-----|---|-----|---------|------|-----|----------|-------|------------------------|---------|---|--|
| D | | | N | lorthbo | und | | So | uthbou | Ind | E | Eastbour | nd | Westbound | | | |
| | | | L | Т | R | | L | Т | R | L | Т | R | L | Т | R | |
| | | Overall LOS | | (0.3) | | | | | | | | | | | | |
| ting () | | Approach LOS | | A (7.6 | 5) | | | A (0.0) | | | B (11.0) | | A (0.0) | | | |
| | ٩M | Storage | | | • | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | | |
| SC | | 95th Queue | 0 | | | | | 0 | | | 3 | | | 0 | | |
| ₩≥ | | Overall LOS | | | | | | | (0 | .2) | | | | | | |
| 057 | | Approach LOS | | A (8.5 | 5) | | | A (0.0) | | | C (15.7) |) | A (0.0) | | | |
| 5 | РМ | Storage | | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | | |
| | | 95th Queue | 0 | | | | | 0 | | | 3 | | 0 | | | |
| | | Overall LOS | | | | | | | (0 | .3) | | | | | | |
| | AM | Approach LOS | A (7.9) | | | | | A (0.0) | | | B (11.3) |) | | A (0.0) | | |
| 9 | | Storage | | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | | |
| ISC B | | 95th Queue | 0 | | | | | 0 | | | 3 | | | 0 | | |
| ĭ₹Ĕ | | Overall LOS | | | | | | | (0 | .2) | | | | | | |
|)30 | - | Approach LOS | | A (8.7 | 7) | | | A (0.0) | | | C (17.9) |) | | A (0.0) | | |
| 20 | ΡN | Storage | | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | | |
| | | 95th Queue | 0 | | | | | 0 | | | 3 | | 0 | | | |
| | | Overall LOS | | | | | | | (2 | .7) | | | | | | |
| | L | Approach LOS | | A (7.9 | 9) | | | A (8.1) | | | B (12.0) |) | | C (15.4 |) | |
| | AN | Storage | | | | _ | | | | | | | | | | |
| <u>ن ا</u> | | 50th Queue | | | | | | | | | | | | | | |
| NS(| | 95th Queue | | 0 | | | | 0 | | | 3 | | 23 | | | |
| 8È | | Overall LOS | | | | | | | (2 | .0) | | | 1 | | | |
| 50 | L | Approach LOS | | A (8. | 7) | - | | A (8.2) | | | C (21.1 |) | | C (23.6 |) | |
| | Р | Storage | | | | _ | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | | |
| | | 95th Queue | | 0 | | | | 3 | | | 3 | | 28 | | | |

5.8 Chappell Road at Kennesaw Drive / Proposed Driveway C (Intersection 8)

The existing two-way stop-controlled intersection of Chappell Road at Kennesaw Drive (Intersection 8) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2024 Existing conditions and 2030 No-Build conditions during the AM and PM peak hours.

Proposed Driveway C will become the 4th leg to the existing two-way stop-controlled intersection of Chappell Road at Kennesaw Drive. The modified two-way stop-controlled intersection of Chappell Road at Kennesaw Drive / Proposed Driveway C (Intersection 8) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2030 Build traffic conditions during the AM and PM peak hours.

The recommended lane configuration for Proposed Driveway C is one lane entering the site and one lane exiting the site operating under driveway stop-control, as shown in the site plan. The recommended build improvements are shown in **Figure 10**.

| Overall LOS Standard: D Approach LOS Standard: D | | | Proposed Driveway A | | | | | | North Avenue | | | North Avenue | | | |
|---|----|--------------|------------------------|---------|---|------------|---|---|--------------|---------|---|--------------|---|---|--|
| | | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | | |
| | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R | |
| | | Overall LOS | | (2.3) | | | | | | | | | | | |
| | | Approach LOS | | A (9.2) | | | | | | A (0.0) | | A (7.4) | | | |
| SC) | AM | Storage | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | |
| | | 95th Queue | | 5 | | | | | 0 | | | 0 | | | |
| | | Overall LOS | | (0.8) | | | | | | | | | | | |
| 203 (| | Approach LOS | B (10.0) | | | | | | A (0.0) | | | A (7.4) | | | |
| | M | Storage | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | |
| | | 95th Queue | | 3 | | | | | | 0 | | | 3 | | |

5.9 North Avenue at Proposed Driveway A (Intersection 9)

The proposed two-way stop-controlled intersection of North Avenue at Proposed Driveway A (Intersection 9) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2030 Build traffic conditions during the AM and PM peak hours.

The recommended lane configuration for Proposed Driveway A is one lane entering the site and one lane exiting the site operating under driveway stop-control, as shown in the site plan. The recommended build improvements for each alternative are shown in **Figure 10**.

| Overall LOS Standard: D Approach LOS Standard: D | | | Proposed Driveway B | | | | | | North Avenue | | | North Avenue | | | |
|---|----|--------------|------------------------|----------|---|------------|---|----|--------------|---------|----|--------------|---------|---|--|
| | | | Northbound | | | Southbound | | | E | astbour | nd | Westbound | | | |
| | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R | |
| | | Overall LOS | | (3.0) | | | | | | | | | | | |
| 2030 BUILD (TWSC) | | Approach LOS | | A (9.3) | | | | | A (0.0) | | | A (7.4) | | | |
| | AM | Storage | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | |
| | | 95th Queue | | 5 | | | | | 0 | | | 0 | | | |
| | | Overall LOS | | | | | | (1 | .1) | | | | | | |
| | | Approach LOS | | B (10.7) | | | | | | A (0.0) | | | A (7.5) | | |
| | Σd | Storage | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | |
| | | 95th Queue | | 5 | | | | | | 0 | | | 3 | | |

5.10 North Avenue at Proposed Driveway B (Intersection 10)

The proposed two-way stop-controlled intersection of North Avenue at Proposed Driveway B (Intersection 10) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2030 Build traffic conditions during the AM and PM peak hours.

The recommended lane configuration for Proposed Driveway B is one lane entering the site and one lane exiting the site operating under driveway stop-control, as shown in the site plan. The recommended build improvements are shown in **Figure 10**.

| 0 | | ayoon ranno | 1100 | aur | 1000 | | | | 11110 | 0000 | | / | | | | |
|---|----|--------------|------------|-------|------|------------------------|----------|----|-----------------------|---------|----|-----------------------|---------|---|--|--|
| Overall LOS Standard: D Approach LOS Standard: D | | | | | | Proposed Driveway D | | | Mayson Turner Road | | | Mayson Turner Road | | | | |
| | | | Northbound | | | So | outhbou | nd | E | astbour | nd | Westbound | | | | |
| | | | L | Т | R | L | Т | R | L | Т | R | L | Т | R | | |
| | | Overall LOS | | (2.6) | | | | | | | | | | | | |
| CD () | AM | Approach LOS | | | | A (9.9) | | | A (7.4) | | | A (0.0) | | | | |
| | | Storage | | | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | | | |
| In Science | | 95th Queue | | | | | 8 | | | 0 | | | 0 | | | |
| ₩ P | | Overall LOS | | (1.2) | | | | | | | | | | | | |
| 503 | | Approach LOS | | | | | B (10.5) | | | A (7.6) | | | A (0.0) | | | |
| | Σd | Storage | | | | | | | | | | | | | | |
| | _ | 50th Queue | | | | | | | | | | | | | | |
| | | 95th Queue | | | | | 5 | | | 0 | | | 0 | | | |

5.11 Mayson Turner Road at Proposed Driveway D (Intersection 11)

The proposed two-way stop-controlled intersection of Mayson Turner Road at Proposed Driveway D (Intersection 11) is projected to meet GRTA's LOS standards per approach and for the <u>overall</u> LOS under the 2030 Build traffic conditions during the AM and PM peak hours.

The recommended lane configuration for Proposed Driveway D is one lane entering the site and one lane exiting the site operating under driveway stop-control, as shown in the site plan. The recommended build improvements are shown in **Figure 10**.






Proposed Site Plan





| 52(C2.00 - DRI SITE PLAN.dwg | | | | | | | | | | | | | | | | > - SCALE: 1" = 400' | Proctor Gree | antikarian ta |
|------------------------------|-------|---------------|------------|--------------------------|--------------------------|-------|-------------------|-------------------|----------------------|--|-----|---|-------------------|--|---|---|--|--|
| lish_148 | PROPO | SED LAN | D USES AN | D DENSITI | IES | | | | SITE DEVELOPME | NT SUMMARY | 7 6 | SITE NOTES | PROJECT CONT | ACTS | | UDestroved Mobile phone re | VIIIII | The second secon |
| \AcPubl | Block | Land Use | MFLow-Rise | Residentia MFMid-Rise | al (Units) Affordable | TOTAL | Commercia (SF) | Parking Spaces | OWNER: | ATLANTA BELTLINE, INC. | i ŀ | 1. THE BUILDING FOOTPRINTS, STREET LOCATIONS, OPEN SPACE LOCATIONS, SIDEWALK DESIGNS AND | CLIENT/APPLICANT: | ATLANTA BELTLINE, INC. 100 PEACHTREE STREET, NW, SUITE 2300 | | | | Man XXXIIII |
| emp | 1 | Commercial | - | - | - | - | 5,000 | 30 | DRI NUMBER: | 4295 | | PARKING LOCATIONS ON THIS CONCEPTUAL SITE | | ATLANTA, GA 30303 | | | | <i>\////////////////////////////////////</i> |
| LNeo | 2 | Residential | 20 | - | 8 | 28 | - | 16 | OVERALL SITE AREA: | 31.00 ACRES | | SHAPES, LOCATIONS, AND AMOUNTS MAY VARY AS | | PHONE: 404-477-3003 | NW | | | <i>/////////////////////////////////////</i> |
| /Po | 3 | Residential | 32 | - | 12 | 44 | - | 33 | CURRENT ADDRESS: | 425 CHAPPELL ROAD | | ALLOWED FOR BY DISTRICT REGULATIONS. | TRAFFIC | | W | | | |
| Dat | 4 | Residential | 1/ | - | 6 | 25 | - | 34 | PROPOSED ZONING: | PD-MU, BELTLINE OVERLAY | | 2. SHARED PARKING STRATEGIES WILL BE USED WHERE | CONSULTANT: | KIMLEY-HORN & ASSOCIATES, INC. | | V///////////////////////////////////// | <i>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</i> | |
| App | 6 | Residential | 20 | - 84 | 36 | 120 | - | 46 | | (UPDATED ZONING CONDITIONS) | | POSSIBLE TO REDUCE THE TOTAL NUMBER OF | | 1200 PEACHTREE STREET, NE | NW | • V//////////////////////////////////// | | |
| UR | 7 | Residential | - | 168 | 74 | 242 | | 218 | PROGRAM | | | PARKING SPACES PROVIDED. | | ATLANTA, GA 30309 | West Avenue | | 1111 Martin | |
| 1.M | 8 | Residential | - | 223 | 97 | 320 | - | 372 | | | | 3. LOCATION DEDICATED TO LOADING OR PICK / | | CONTACT: ANA EISENMAN | Conway PI NW | | | |
| é | 9 | Residential | - | 155 | 66 | 221 | • | 220 | ESIDENTIAL ONITS. | DENSITY: 1100 UNITS / 31 ACRES = 35.48 | | TRUCKS. PACKAGE DELIVERY, AND/OR OTHER HEAVY | | PHONE: 404-201-6155 | | | | |
| IME | 10 | Residential | 43 | - | 17 | 60 | - | 60 | L L | JNITS/ACRE | | VEHICLE DELIVERIES, WILL MEET CITY OF ATLANTA | SITE DESIGNER: | TSW | 1 | | | |
| s\RI | | On Street Par | king | | | | | 208 | RETAIL/COMMERCIAL: 5 | 5,000 SF | | CODE REQUIREMENTS. | | 1447 PEACHTREE STREET NE SUITE 850 | tian the second s | Perty A | Fairway Cor | |
| Jser | | TOTA | 140 | 630 | 330 | 1,100 | 5,000 | 1,272 | 0 | COMMERCIAL FAR = 0.174 | | 4. PARKING DETAILS ARE PRELIMINARY AND WILL BE | | ATLANTA, GA 30309 | Cha | NW. | Apartments | |
| C:\L | | | | | | | | | PARKING | | | DETERMINED FURTHER IN THE SITE DEVELOPMENT PROCESS. | | CONTACT: RYAN SNODGRASS PHONE: 404-873-6730 | Jore Store | | | |
| :em | | | | | | | | | | | | | J [| | | | | Kitchen Queen |
| g nai | | | | | | | | | REQUIRED: 1 | ,860 SPACES (MAXIMUM ALLOWABLE) | | | | | 0 | Joseph E. Boone BJ | Ivd NW Ebony Beau | uty Supply 👩 Joseph E. Boone Blvd NW 🖸 |
| awing | | | | | | | | | PROVIDED: 1 | ,272 SPACES | | | | | - | | | - |



| | Kimley»Horn | 1200 PEACHTREE STREET, NE PHONE 404 419 8700 SUITE 800 NOV KIMLEY-HORN COM |
|--------------------------------|--|---|
| | ATLANTA BELTLINE, INC. | 100 PEACHTREE STREET, NN, SUITE 2300 ATLANTA, 6A 30303 PHONE: 404477-3003 |
| | 11/16/2024 RTM CLE | |
| | L ROAD <u>2 FINAL DRISITE R.M.</u> 95 | MTA, GA 30318 INTRIOT INTRIOT |
| DRI SITE PLAN - SCALE: 1* = 20 | 425 CHAPPEI DRI #42 | 425 CHARPELL ROAD, ATL AND LOT 43, 3481 PARCEL D: 40 443, 4481 |
| | GSWCC CERT. (LEVEL II) DRAWN BY DESIGNED BY REVIEWED BY DATE PROJECT NO. TITLE DRI SIT | 000000000 11/18/202 01340400 FE PLAN |



Trip Generation Analysis

| | | Trip Constation Analysis (11th | | ad Edition Hondh | aale Daily I | 0 0 0 4 7 | dition AN | | | | | | |
|-----------|------------------------------------|--------------------------------|-------|------------------|--------------|------------|------------|------------|----------|-----|-------|----------|-----|
| | | The Generation Analysis (Thin | | | OOK Dally I | C & SIU E | Caltion AI | ///PIVLIC) | | | | | |
| | | | 420 | Chappell Rd DRI | | | | | | | | | |
| | | | Cit | y of Aliania, GA | | aily Trips | | AM | Peak Hou | ır | PM | Peak Hou | ır |
| Land Us | e | Setting | [| Density | Total | In | Out | Total | In | Out | Total | In | Out |
| Propose | ed Project Trips | | | | | | | | | | | | |
| 220 | Multifamily Housing (Low-Rise) | General Urban/Suburban | 140 | dwelling units | 972 | 486 | 486 | 66 | 16 | 50 | 81 | 51 | 30 |
| 221 | Multifamily Housing (Mid-Rise) | General Urban/Suburban | 630 | dwelling units | 2,958 | 1,479 | 1,479 | 266 | 61 | 205 | 246 | 150 | 96 |
| 223 | Affordable Housing | General Urban/Suburban | 330 | dwelling units | 1,370 | 685 | 685 | 87 | 25 | 62 | 152 | 90 | 62 |
| 822 | Strip Retail Plaza (<40k) | General Urban/Suburban | 5,000 | Sq. Ft. GFA | 440 | 220 | 220 | 12 | 7 | 5 | 48 | 24 | 24 |
| | | | | | | | | | | | | | |
| Gross P | roject Trips | | | | 5,740 | 2,870 | 2,870 | 431 | 109 | 322 | 527 | 315 | 212 |
| | | | | | | | | | | | | | |
| Reside | ntial Trips | | | | 5,300 | 2,650 | 2,650 | 419 | 102 | 317 | 479 | 291 | 188 |
| | Mixed-Use Reductions | | | | -44 | -22 | -22 | -2 | -1 | -1 | -8 | -6 | -2 |
| | Alternative Mode Reductions | | | | -1,313 | -657 | -656 | -104 | -25 | -79 | -118 | -71 | -47 |
| | Adjusted Residential Trips | | | | 3,943 | 1,971 | 1,972 | 313 | 76 | 237 | 353 | 214 | 139 |
| Retail 7 | Frips | | | | 440 | 220 | 220 | 12 | 7 | 5 | 48 | 24 | 24 |
| | Mixed-Use Reductions | | | | -44 | -22 | -22 | -2 | -1 | -1 | -8 | -2 | -6 |
| | Alternative Mode Reductions | | | | -100 | -50 | -50 | -3 | -2 | -1 | -10 | -6 | -5 |
| | Pass By Reductions (Based on ITE R | Pates) | | | -118 | -59 | -59 | 0 | 0 | 0 | -12 | -6 | -6 |
| | Adjusted Retail Trips | | | | 178 | 89 | 89 | 7 | 4 | 3 | 18 | 10 | 7 |
| Mixed-U | lse Reductions - TOTAL | | | | -88 | -44 | -44 | -4 | -2 | -2 | -16 | -8 | -8 |
| Alternati | ve Mode Reductions - TOTAL | | | | -1,413 | -707 | -706 | -107 | -27 | -80 | -128 | -77 | -52 |
| Pass-By | Reductions - TOTAL | | | | -118 | -59 | -59 | 0 | 0 | 0 | -12 | -6 | -6 |
| New Tri | ps | | | | 4,121 | 2,060 | 2,061 | 320 | 80 | 240 | 371 | 224 | 146 |
| Drivewa | y Volumes | | | | 4,239 | 2,119 | 2,120 | 320 | 80 | 240 | 383 | 230 | 152 |

Intersection Volume Worksheets

INTERSECTION VOLUME DEVELOPMENT INTERSECTION #1 GA-8 Donald Lee Hollowell Pkwy NW (West)/GA-8 Donald Lee Hollowell Pkwy NW (East) at Chappell Rd NW/Driveway

| | | | | | | AM PE | ak hour | | | | | | | | | |
|---------------------------------------|--------|---------|----------|---------|--------|-------|---------|-------|-----------|---------------|------------|-----------|----------|-------------|-------------|-----------|
| | | Chappe | ll Rd NW | | | Driv | eway | | GA-8 Dona | ald Lee Hollo | owell Pkwy | NW (West) | GA-8 Don | ald Lee Hol | lowell Pkwy | NW (East) |
| | | North | bound | | | South | bound | | | Eastb | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 66 | 0 | 173 | 0 | 0 | 0 | 0 | 0 | 0 | 955 | 37 | 0 | 58 | 344 | 0 |
| Count Balancing | | | | | | | | | | | | | | | | 1 |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 0 | | | | 0 | | | | 1 | | | | 0 |
| Heavy Vehicles | 0 | 4 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 52 | 2 | 0 | 3 | 33 | 0 |
| Heavy Vehicle % | 2% | 6% | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 5% | 2% | 5% | 10% | 2% |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 66 | 0 | 173 | 0 | 0 | 0 | 0 | 0 | 0 | 955 | 37 | 0 | 58 | 344 | 0 |
| | | | | | | | | | | | | | | r | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 6 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 89 | 3 | 0 | 5 | 32 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | 64 | | | | 30 | - |
| 2030 No-Build Traffic | 0 | 72 | 0 | 189 | 0 | 0 | 0 | 0 | 0 | 0 | 1,108 | 40 | 0 | 63 | 406 | 0 |
| 2030 No-Build Heavy Vehicle % | 2% | 6% | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 5% | 2% | 5% | 10% | 2% |
| 2030 No-Build Pedestrians | | 1 | 0 | | | | 0 | | | | 0 | | | | 0 | |
| 2030 No-Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | 10% | | 15% | | |
| Trip Distribution OUT | | (10%) | | (15%) | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 24 | 0 | 36 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 11 | 0 | 0 |
| T | | - | | | - | | | | | | | 4.00/ | | 0.001 | | |
| Trip Distribution IN | | (4.00)) | | (0.00() | | | | | | | | 10% | | 20% | | |
| Trip Distribution OUT | | (10%) | | (20%) | | | | | | | | | | | | |
| Balancing Adjustment | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 |
| Retail Hips | U | U | U | 1 | U | U | U | U | U | U | U | U | U | 1 | U | . 0 |
| Trip Distribution IN | 1 | 1 | 1 | | | | | | r | 1 | 1 | 109/ | | 200/ | 1 | |
| Trip Distribution OUT | | (10%) | | (20%) | | | | | | | | 10% | | 20% | | |
| Palancing Adjustment | | (10%) | | (20%) | | | | | | | | | | | | |
| Postaurant Trins | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Restaurant mps | U | U | 0 | U | 0 | U | U | U | 0 | 0 | 0 | U | U | U | 0 | |
| Total Primary Site Trips | 0 | 24 | 0 | 37 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 12 | 0 | 0 |
| Total Filling Site Trips | Ű | | Ū | 01 | Ū | Ū | Ū | Ū | Ū | Ū | Ū | 0 | Ū | 12 | Ū | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | - | | - | | | - | - | | | | - | | |
| Total Vehicular Project Trips | 0 | 24 | 0 | 37 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 12 | 0 | 0 |
| | | | | | | | | - | | | | | | | | - |
| 2030 Build Traffic | 0 | 96 | 0 | 226 | 0 | 0 | 0 | 0 | 0 | 0 | 1,108 | 48 | 0 | 75 | 406 | 0 |
| 2030 Build Heavy Vehicle % | 2% | 5% | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 5% | 2% | 4% | 9% | 2% |
| 2030 Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| | | | | | | | | | | | | | | | | _ |

PM PEAK HOUR

| Northbound Southbound Southbo | | - | Channe | II Pd NW | | r | Driv | oway | | GA-8 Dona | ld Lee Holl | | NW (West) | GA-8 Don: | ald I aa Holl | owell Pkwy | NW (East) |
|--|---------------------------------------|---------|--------|----------|-------|---------|-------|---------------|-------|-----------|-------------|--------------|------------|-----------|---------------|------------|-------------|
| U-Tom Left Throage Right U-Tom Left Throage Right U-Tom Left Torm Left Torm Right U-Tom Left Torm | | | Mortk | abound | | | South | cway bound | | GA-0 DUIR | Footh | Jweii Fkwy i | www (west) | GA-0 DOIR | Most | bound | INVV (Last) |
| Desired 3204 Traffic Volumes O 0 | | 11 Turn | NOLL | Through | Diabt | 11 Turn | Loft | Through | Diabt | 11 Turn | Easu | Through | Diabt | 11 Turn | VVest | Through | Diabt |
| Dote of Balance Volumes D P D 111 D <th>Observed 2024 Troffic Volumes</th> <th>0-Tuill</th> <th>Leri</th> <th>niougn</th> <th>111</th> <th>0-Turri</th> <th></th> <th>nii ougii</th> <th>Right</th> <th>0-1011</th> <th>Leit</th> <th>E11</th> <th>47</th> <th>0-1011</th> <th>104</th> <th>072</th> <th>Right</th> | Observed 2024 Troffic Volumes | 0-Tuill | Leri | niougn | 111 | 0-Turri | | nii ougii | Right | 0-1011 | Leit | E11 | 47 | 0-1011 | 104 | 072 | Right |
| Data Balancing Conflicting Rejectations I | Count Balancing | 0 | 99 | U | 111 | U | U | U | U | U | U | 511 | 67 | U | 100 | 912 | U |
| Understands U <th< td=""><td>De destale es</td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<> | De destale es | - | | | | | | | | | | | | | | | |
| Continuing Vederitarias D <thd< th=""> <thd< th=""> D <thd< th=""></thd<></thd<></thd<> | Pedestrians | - | | 0 | | | | 0 | | | | 0 | | | | 0 | <u> </u> |
| Biological Section 1 D <thd< th=""> <thd< th=""> D <thd< th=""></thd<></thd<></thd<> | Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Conflicting Brydes Image: Second | Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |
| Heary Vehicles Heary Vehicles Heary Vehicles 0 3 0 3 0 3 0 0 0 0 0 0 3 0 3 57 0 Weak Heary Vehicles Mapstement Factor Adjustment Factor 0 0 <td>Conflicting Bicycles</td> <td>-</td> <td>1</td> <td></td> <td>0</td> <td></td> <td></td> <td></td> <td>0</td> <td></td> <td></td> <td></td> <td>3</td> <td></td> <td></td> <td></td> <td>0</td> | Conflicting Bicycles | - | 1 | | 0 | | | | 0 | | | | 3 | | | | 0 |
| Heary Vehicle % 2% 3% 2% 3% 2% 0% 0.95 </td <td>Heavy Vehicles</td> <td>0</td> <td>3</td> <td>0</td> <td>3</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>30</td> <td>6</td> <td>0</td> <td>3</td> <td>57</td> <td>0</td> | Heavy Vehicles | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 30 | 6 | 0 | 3 | 57 | 0 |
| Peak Hour Factor 0.95 | Heavy Vehicle % | 2% | 3% | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 6% | 9% | 2% | 2% | 6% | 2% |
| Adjustment Factor 1 | Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Adjusted 2024 Volumes 0 99 0 111 0 0 0 0 0 15%< | Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Annual Growth Rate Growth Factor 15% <th< td=""><td>Adjusted 2024 Volumes</td><td>0</td><td>99</td><td>0</td><td>111</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>511</td><td>67</td><td>0</td><td>186</td><td>972</td><td>0</td></th<> | Adjusted 2024 Volumes | 0 | 99 | 0 | 111 | 0 | 0 | 0 | 0 | 0 | 0 | 511 | 67 | 0 | 186 | 972 | 0 |
| Annual Covolth Rate (covolt Factor) 1.5% | | - | | | | - | | | | - | | | | - | | | |
| Growth Frigs 0 0 109 109 109 109 109 10 0 10 0 0 0 0 | Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Background Growth Trips 0 9 0 10 0 0 0 0 0 48 6 0 17 91 0 0050 DH Byt H4187 0 108 0 121 0 < | Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Use DL MR #187 Image: Constraint of the second | Background Growth Trips | 0 | 9 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 48 | 6 | 0 | 17 | 91 | 0 |
| 0 108 0 121 0 0 0 0 591 73 0 203 1,126 0 0 | 1060 DLH DRI #4187 | | | | | | | | | | | 32 | | | | 63 | |
| 2030 No-Build Heavy Vehicle % 0 | 2030 No-Build Traffic | 0 | 108 | 0 | 121 | 0 | 0 | 0 | 0 | 0 | 0 | 591 | 73 | 0 | 203 | 1,126 | 0 |
| 0 | 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2030 No-Build Conflicting Pedestrians 0 | 2030 No-Build Pedestrians | - | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Image: construction IN Image: construction IN< | 2030 No-Build Conflicting Pedestrians | - | 0 | 1 | 0 | | 0 | | 0 | | 0 | 1 | 0 | | 0 | | 0 |
| Trip Distribution IN Image: constraint of the second s | | | | | | | | | | | | | | | | | |
| Trip Distribution OUT Balancing Adjustment Residential Trips Image: constraint of the second se | Trip Distribution IN | | | | | | | | | | | | 10% | | 15% | | |
| Balancing Adjustment Residential Trips Image: Control of the second | Trip Distribution OUT | | (10%) | | (15%) | | | | | | | | | | | | |
| Residential Trips 0 14 0 21 0 0 0 0 0 0 0 0 21 0 32 0 0 Trip Distribution IN Trip Distribution OUT Balancing Adjustment (10%) (20%) 1 10% 20% 1 10% 20% 1 1 10% 20% 1 10% 20% 1 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 10% 10% 10% 10% 10% 10% 10% | Balancing Adjustment | | | | | | | | | | | | | | | | |
| Image: constraint of the second sec | Residential Trips | 0 | 14 | 0 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 21 | 0 | 32 | 0 | 0 |
| Trip Distribution IN Trip Distribution OUT Balancing Adjustment Image: Constraint of the second se | | | | 1 | | | | | | | | | | | | | |
| Impossibilition OUT Balancing Adjustment Restaurant Trips Impossibilities | Trip Distribution IN | | 1 | | | | | | | | | | 10% | | 20% | | |
| Balancing Adjustment Retail Trips I <thi< th=""> <thi< th=""> <thi< th=""> <th< td=""><td>Trip Distribution OUT</td><td></td><td>(10%)</td><td></td><td>(20%)</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<></thi<></thi<></thi<> | Trip Distribution OUT | | (10%) | | (20%) | | | | | | | | | | | | |
| Retail Trips 0 1 0 1 0 1 0 0 0 0 0 0 0 0 1 0 2 0 0 Trip Distribution IN Trip Distribution OUT Balancing Adjustment (10%) (20%) 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 1 10% 20% 10% 10% 10% 10% 20% 10% 10% 10% 10% 10% 10% 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | Balancing Adjustment | | | | | | | | | | | | | | | | |
| Image: construction IN Trip Distribution OUT Balancing Adjustment Image: construction OUT Balancing Adjustment Image: constructing Adjustment Image: construction OUT Balancin | Retail Trips | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 2 | 0 | 0 |
| Trip Distribution IN Trip Distribution OUT Balancing Adjustment Restaurant Trips Image: Constraint of the state of | | | | | | | | | | | | | | | | | |
| Trip Distribution OUT (10%) (20%) Image: Constraint of the second | Trip Distribution IN | | | | | | | | | | | | 10% | | 20% | | |
| Balancing Adjustment Restaurant Trips Image: Constraint of the second seco | Trip Distribution OUT | | (10%) | | (20%) | | | | | | | | | | | | |
| Restaurant Trips 0 | Balancing Adjustment | | | | | | | | | | | | | | | | |
| Total Primary Site Trips 0 15 0 22 0 0 0 0 0 0 22 0 34 0 0 Pass-By Trips 0< | Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Primary Site Trips 0 15 0 22 0 0 0 0 0 0 22 0 34 0 0 Pass-By Trips 0 | | | | | | | | | | | | | | | | | |
| Pass-By Trips 0 < | Total Primary Site Trips | 0 | 15 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 34 | 0 | 0 |
| Pass-By Trips 0 < | | | | | | | | | | | | | | | | | |
| O 123 O 143 O O O O O 220 O A O O 2030 Build Traffic 0 123 0 143 0 0 0 0 591 95 0 237 1,126 0 2030 Build Heavy Vehicle % 2% 3% 2% 2% 2% 2% 2% 6% 7% 2% 6% 2% 2% 2% 2% 2% 6% 7% 2% 6% 2% 2% 2% 2% 2% 6% 7% 2% 6% 2% 2% 2% 2% 2% 6% 7% 2% 6% 2% | Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips 15 0 22 0 0 0 0 0 0 22 0 34 0 0 2030 Build Traffic 0 123 0 143 0 0 0 0 0 591 95 0 237 1,126 0 2030 Build Heavy Vehice % 2% 3% 2% | | | | | | | | | | | | | | | | | |
| Dot Digital Traffic 0 123 0 143 0 0 0 0 591 95 0 237 1,126 0 2030 Build Heavy Vehicle % 2% 3% 2% 2% 2% 2% 2% 2% 6% 7% 2% 2% 6% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% </td <td>Total Vehicular Project Trips</td> <td></td> <td>15</td> <td>0</td> <td>22</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>22</td> <td>0</td> <td>34</td> <td>0</td> <td>0</td> | Total Vehicular Project Trips | | 15 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 34 | 0 | 0 |
| 0 123 0 143 0 0 0 0 591 95 0 237 1,126 0 2030 Build Traffic 2% 3% 2% 2% 2% 2% 2% 6% 7% 2% | | | | | | • | | | | | | | | | | | |
| 2% 3% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 6% 7% 2% 2% 6% 2% 2% 2% 2% 6% 2%< | 2030 Build Traffic | 0 | 123 | 0 | 143 | 0 | 0 | 0 | 0 | 0 | 0 | 591 | 95 | 0 | 237 | 1,126 | 0 |
| 0 | 2030 Build Heavy Vehicle % | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 6% | 7% | 2% | 2% | 6% | 2% |
| 2030 Build Conflicting Pedestrians 0 0 0 0 0 0 0 0 0 0 0 0 0 | 2030 Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| | 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |

INTERSECTION VOLUME DEVELOPMENT INTERSECTION #2 Chappell Rd NW (South)/Chappell Rd NW (North) at North Avenue NW (West)/North Avenue NW (East)

| | | | | | | AM PE | AK HOUR | 2 | | | | | | | | |
|---------------------------------------|--------|-------------|-------------|-------|--------|-------------|-------------|-------|--------|------------|-----------|-------|--------|-------------|------------|--------|
| | (| Chappell Ro | I NW (South | ı) | (| Chappell Ro | I NW (North | 1) | N | orth Avenu | e NW (Wes | st) | N | Iorth Avenu | ue NW (Eas | t) |
| | 1 | North | bound | | | South | nbound | | | Eastb | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 11 | 242 | 39 | 0 | 2 | 112 | 4 | 0 | 14 | 18 | 19 | 0 | 37 | 11 | 8 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 0 | | | | 0 | | | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 3 | 11 | 2 | 0 | 0 | 4 | 2 | 0 | 0 | 1 | 1 | 0 | 4 | 6 | 5 |
| Heavy Vehicle % | 2% | 27% | 5% | 5% | 2% | 2% | 4% | 50% | 2% | 2% | 6% | 5% | 2% | 11% | 55% | 63% |
| Peak Hour Factor | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 | 0.86 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 11 | 242 | 39 | 0 | 2 | 112 | 4 | 0 | 14 | 18 | 19 | 0 | 37 | 11 | 8 |
| | | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 1 | 23 | 4 | 0 | 0 | 10 | 0 | 0 | 1 | 2 | 2 | 0 | 3 | 1 | 1 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 0 | 12 | 265 | 43 | 0 | 2 | 122 | 4 | 0 | 15 | 20 | 21 | 0 | 40 | 12 | 9 |
| 2030 No-Build Heavy Vehicle % | 2% | 27% | 5% | 5% | 2% | 2% | 4% | 50% | 2% | 2% | 6% | 5% | 2% | 11% | 55% | 63% |
| 2030 No-Build Pedestrians | | | 0 | | | | 0 | | | |) | | | | 0 | |
| 2030 No-Build Conflicting Pedestrians | | 0 | | 0 | | 0 | 1 | 0 | | 0 | | 0 | | 0 | | 0 |
| 0 | | | | | | | | | | | | | | | | |
| Trip Distribution IN | 1 | 1 | | | 1 | 10% | 15% | | 1 | | 5% | | 1 | | | |
| Trip Distribution OUT | | | (15%) | | | | | | | | | | | | (5%) | (10%) |
| Balancing Adjustment | | | | | | | | | | | | | | | . , | |
| Residential Trips | 0 | 0 | 36 | 0 | 0 | 8 | 11 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 12 | 24 |
| | | | | | | | | | | | | | | | 1 | |
| Trip Distribution IN | | | | | | | | | | 1 | | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Hotel Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | - | | | | - | | | - | - | | | 1 | - |
| Trip Distribution IN | | 1 | | | | 10% | 20% | | | | 5% | | 1 | | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | (5%) | (10%) |
| Balancing Adjustment | | | (2010) | | | | | | | | | | | | (2.2) | (1215) |
| Retail Trips | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | 1 | | | | 1 | 10% | 20% | | | | 5% | | 1 | ľ | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | (5%) | (10%) |
| Balancing Adjustment | | 1 | () | | | | 1 | | | | | | 1 | | (/ | () |
| Restaurant Trins | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| listudiant mps | | | Ū | Ŭ | Ů | Ŭ | Ū | ů, | Ū | , v | Ŭ | Ŭ | Ŭ | | , i | |
| Total Primary Site Trips | 0 | 0 | 37 | 0 | 0 | 8 | 12 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 12 | 24 |
| fordar i finitar y once i nipo | Ů | 0 | 07 | 0 | | 0 | 12 | 0 | 0 | 0 | | 0 | Ŭ | | 12 | 21 |
| Pass-Ry Trins | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1055 by 111p5 | | | , v | , , | Ů | , v | | , i | Ū | , v | , , | Ŭ | , v | | | |
| Total Vehicular Project Trips | 0 | 0 | 37 | 0 | 0 | 8 | 12 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 12 | 24 |
| | + ~ | | | | • · · | | 1 12 | | | | | | - × | | | 2.1 |
| 2030 Build Traffic | 0 | 12 | 302 | 43 | 0 | 10 | 134 | 4 | 0 | 15 | 25 | 21 | 0 | 40 | 24 | 33 |
| 2030 Build Heavy Vehicle % | 2% | 27% | 4% | 5% | 2% | 2% | 3% | 55% | 2% | 2% | 4% | 5% | 2% | 11% | 27% | 17% |
| 2030 Build Pedestrians | 270 | 2170 | 0 | 370 | 270 | 2.70 | 0 | 3370 | 270 | 270 | 1 | 570 | 2.70 | 1170 | 0 | 1770 |
| 2030 Build Conflicting Pedestrians | | 0 | Ĭ | 0 | | 0 | Ĭ | 0 | | 0 | Í | 0 | | 0 | Ĭ | 0 |
| 2000 band commenny redescribits | | U | | v | | 0 | | U | | U | | 0 | | | | 0 |

| | | | | | | PM PE | ak hour | | | | | | | | | |
|---------------------------------------|--------|-------------|-------------|-------|--------|-------------|-----------|-------|--------|------------|-----------|-------|--------|------------|------------|-------|
| | (| Chappell Ro | I NW (South | 1) | (| Chappell Rd | NW (North | i) | N | orth Avenu | e NW (Wes | st) | N | Jorth Aven | ue NW (Eas | t) |
| | | North | nbound | | | South | bound | | | East | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 9 | 234 | 41 | 0 | 4 | 284 | 19 | 0 | 7 | 15 | 19 | 0 | 203 | 94 | 13 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | - | | | 0 | | i | | 0 | | i | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 2 | 5 | 4 | 0 | 0 | 8 | 1 | 0 | 0 | 0 | 2 | 0 | 2 | 2 | 1 |
| Heavy Vehicle % | 2% | 22% | 2% | 10% | 2% | 2% | 3% | 5% | 2% | 2% | 2% | 11% | 2% | 2% | 2% | 8% |
| Peak Hour Factor | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 | 0.89 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 9 | 234 | 41 | 0 | 4 | 284 | 19 | 0 | 7 | 15 | 19 | 0 | 203 | 94 | 13 |
| | 1 | | | | I | | | | 1 | | L | 1 | I | I | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 1 | 22 | 4 | 0 | 0 | 27 | 2 | 0 | 1 | 1 | 2 | 0 | 19 | 9 | 1 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 0 | 10 | 256 | 45 | 0 | 4 | 311 | 21 | 0 | 8 | 16 | 21 | 0 | 222 | 103 | 14 |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2030 No-Build Conflicting Redestrians | | 0 | 0 | 0 | | 0 | 0 | 0 | | 0 | 0 | 0 | | 0 | 1 | 0 |
| 2030 NO-Build Connecting Fedestrians | | 0 | | 0 | | 0 | | U | | 0 | | 0 | | 0 | | 0 |
| Trip Distribution IN | | | | | | 10% | 15% | | | | 5% | 1 | | | | |
| Trip Distribution OUT | | | (15%) | | | | | | | | | | | | (5%) | (10%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 21 | 0 | 0 | 21 | 32 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 7 | 14 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | 10% | 20% | | | | 5% | | | | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | (5%) | (10%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 1 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 1 |
| Trip Distribution IN | 1 | 1 | 1 | 1 | r | 100/ | 200/ | | r | | E0/ | 1 | r | 1 | 1 | |
| Trip Distribution IN | - | | (20%) | | | 10% | 20% | | | | 376 | | | | (5%) | (10%) |
| Palancing Adjustment | - | | (20%) | | | | | | | | | | | | (3%) | (10%) |
| Restaurant Trins | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Restaurant mps | 0 | Ū | Ŭ | Ū | | 0 | Ū | 0 | , o | 0 | | Ū | | 0 | | Ŭ |
| Total Primary Site Trips | 0 | 0 | 22 | 0 | 0 | 22 | 34 | 0 | 0 | 0 | 11 | 0 | 0 | 0 | 7 | 15 |
| Pass By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Fass-by Tips | 0 | U | 0 | U | 0 | 0 | U | U | 0 | 0 | U | 0 | 0 | U | U | U |
| Total Vehicular Project Trips | 1 | 0 | 22 | 0 | 0 | 22 | 34 | 0 | 0 | 0 | 11 | 0 | 0 | 0 | 7 | 15 |
| , , , , , , , , , , , , , , , , , , , | | | | | | | | | | | | | | | | |
| 2030 Build Traffic | 0 | 10 | 278 | 45 | 0 | 26 | 345 | 21 | 0 | 8 | 27 | 21 | 0 | 222 | 110 | 29 |
| 2030 Build Heavy Vehicle % | 2% | 22% | 2% | 10% | 2% | 2% | 3% | 5% | 2% | 2% | 2% | 10% | 2% | 2% | 2% | 4% |
| 2030 Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| 2030 Build Conflicting Pedestrians | L | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |

INTERSECTION VOLUME DEVELOPMENT INTERSECTION #3 Chappell Rd NW (South)/Chappell Rd NW (North) at West Ave NW/Mayson Turner Rd NW

| | | | | | | AM PE | ak hour | 2 | | | | | | | | |
|---------------------------------------|--------|-------------|-------------|-------|--------|-------------|-------------|-------|--------|--------|---------|-------|--------|-----------|------------|-------|
| | (| Chappell Ro | I NW (South | ר) | (| Chappell Ro | I NW (North | 1) | | West / | Ave NW | | | Mayson Tu | rner Rd NV | / |
| | | North | bound | | | South | bound | | | East | oound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 0 | 199 | 5 | 0 | 79 | 106 | 2 | 0 | 1 | 0 | 2 | 0 | 3 | 0 | 55 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 1 | | | | 0 | | | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 0 | 14 | 1 | 0 | 2 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |
| Heavy Vehicle % | 2% | 2% | 7% | 20% | 2% | 3% | 9% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% |
| Peak Hour Factor | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 0 | 199 | 5 | 0 | 79 | 106 | 2 | 0 | 1 | 0 | 2 | 0 | 3 | 0 | 55 |
| Appual Crowth Pate | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% | 1.6% |
| Growth Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.0/0 | 1.00 | 1.00 | 1.00 | 1.00 | 1.0% | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.0/0 |
| Background Growth Trips | 0 | 0 | 10 | 0 | 0 | 7 | 1.07 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| 1060 DI H DPI #4187 | 0 | U | 17 | | 0 | | 10 | 0 | 0 | U | 0 | 0 | 0 | 0 | 0 | 5 |
| 2030 No-Build Traffic | 0 | 0 | 218 | 5 | 0 | 86 | 116 | 2 | 0 | 1 | 0 | 2 | 0 | 3 | 0 | 60 |
| 2030 No-Build Heavy Vehicle % | 2% | 2% | 7% | 20% | 2% | 3% | 9% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% |
| 2030 No-Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| 2030 No-Build Conflicting Pedestrians | | 0 | Ī | 0 | | 0 | Ī | 0 | | 0 | | 0 | | 0 | Ī | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | 25% | 5% | | | | | | | | | | | | |
| Trip Distribution OUT | | | | | | | (25%) | | | | | | | (5%) | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 19 | 4 | 0 | 0 | 59 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 |
| | | | | | | | | | | | - | | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Hotel Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | 1 | 1 | 30% | | 1 | | | | | 1 | 1 | | | 1 | | 10% |
| Trip Distribution OUT | | | 3070 | | | (10%) | (30%) | | | | | | | | | 10% |
| Balancing Adjustment | | | | | | (1070) | (30%) | | | | | | | | | |
| Retail Trips | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | 1 | | | | | | | | 1 | | | | | | |
| Trip Distribution IN | | | 30% | | | | | | | | | | | | | 10% |
| Trip Distribution OUT | | | | | | (10%) | (30%) | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | 1 | | | | | | | | | |
| Total Primary Site Trips | 0 | 0 | 20 | 4 | 0 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 |
| Deep Du Teler | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Pass-By Trips | U | 0 | U | U | U | U | U | U | 0 | 0 | U | U | 0 | U | U | U |
| Total Vehicular Project Trips | 0 | 0 | 20 | 4 | 0 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 |
| i i i i i i i i i i i i i i i i i i i | | | | | . ~ | | | . ~ | | | | | | | ۰ × | |
| 2030 Build Traffic | 0 | 0 | 238 | 9 | 0 | 86 | 176 | 2 | 0 | 1 | 0 | 2 | 0 | 15 | 0 | 60 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 6% | 12% | 2% | 3% | 6% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% |
| 2030 Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |

| | | | | | | PM PE | ak hour | | | | | | | | | |
|---------------------------------------|--------|-------------|-------------|-------|--------|-------------|-----------|-------|--------|--------|---------|-------|--------|-----------|------------|-------|
| | (| Chappell Ro | I NW (South | ı) | (| Chappell Ro | NW (North | 1) | | West / | Ave NW | | | Mayson Tu | rner Rd NV | / |
| | | North | bound | | | South | bound | | | East | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 1 | 1 | 177 | 8 | 1 | 128 | 330 | 0 | 0 | 2 | 0 | 2 | 0 | 5 | 1 | 104 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 2 | | | | 0 | | | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 0 | 4 | 0 | 0 | 4 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 1 | 1 | 177 | 8 | 1 | 128 | 330 | 0 | 0 | 2 | 0 | 2 | 0 | 5 | 1 | 104 |
| | | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 0 | 17 | 1 | 0 | 12 | 31 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 1 | 1 | 194 | 9 | 1 | 140 | 361 | 0 | 0 | 2 | 0 | 2 | 0 | 5 | 1 | 114 |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2030 No-Build Pedestrians | - | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| 2030 No-Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Trip Distribution IN | 1 | 1 | 259/ | E9/ | r | 1 | 1 | | r | | 1 | | r | 1 | 1 | 1 |
| Trip Distribution IN | - | | 23% | 3% | | | (050()) | | | | | | | (50() | | |
| Trip Distribution OUT | - | | | | | | (25%) | | | | | | | (5%) | | |
| Dalahuny Aujustment | 0 | 0 | E A | 11 | 0 | 0 | 25 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 |
| Residential Hips | U | U | 34 | 11 | U | U | 30 | U | U | U | U | U | U | 1 | U | U |
| Trip Distribution IN | | 1 | 30% | | | 1 | 1 | | | | 1 | | | 1 | | 10% |
| Trip Distribution OUT | - | | 3070 | | | (10%) | (30%) | | | | | | | | | 1070 |
| Balancing Adjustment | - | | | | | (1070) | (0010) | | | | | | | | | |
| Retail Trips | 0 | 0 | 3 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| | | | - | _ | - | | - | - | - | - | - | - | - | - | | |
| Trip Distribution IN | | | 30% | | | | | | | | | | | | | 10% |
| Trip Distribution OUT | | | | | | (10%) | (30%) | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Drimony Cito Trino | 0 | 0 | 67 | 11 | 0 | 1 | 27 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 1 |
| Total Primary site Trips | U | U | 57 | | U | | 37 | U | U | U | U | U | U | 1 | U | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Total vehicular Project Trips | I | 0 | 57 | 11 | 0 | 1 | 37 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 1 |
| 2020 Puild Troffic | 1 | 1 | 261 | 20 | 1 | 141 | 200 | 0 | 0 | 2 | 0 | 2 | 0 | 12 | 1 | 115 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 201 | 20 | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% |
| 2030 Build Pedestrians | 2.70 | 270 | 0 | 270 | 2.70 | 370 | 0 | 270 | 2.70 | 270 | 2.0 | 270 | 2.70 | 270 | 0 | 370 |
| 2030 Build Conflicting Pedestrians | - | 0 | ĭ | 0 | | 0 | Ĭ | 0 | | 0 | Ĭ | 0 | | 0 | Ĭ | 0 |
| | | | | | | | | | | | | | | | | |

| INTERSECTION VOLUME DEVELOPMENT |
|--|
| INTERSECTION #4 |
| Joseph E. Boone Blvd NW (West)/Joseph E. Boone Blvd NW (East) at Chappell Rd NW (South)/Chappell Rd NW (North) |
| |

| | <u>.</u> | | | | | AM PE | ak houf | 2 | | | | | | | | |
|---|--|--|----------|------|------|-------|---------|----------------|------|----------|------|------|------|------|-------|----------|
| | (| Chappell Rd NW (South) Chappell Rd NW (North) Joseph E. Boone Blvd NW (West) Joseph E. Boone Blvd NW (West) Joseph E. Boone Blvd NW (East) Southbound U-Turn Left Through Right U-Turn Left Through Righ | | | | | | | | | | | | | | (East) |
| | | North | bound | | | South | bound | D 1 1 / | | East | ound | | | West | bound | |
| | U-Turn Left Through Right U-Turn Left Throug | | | | | | | | | | | | | | | |
| Observed 2024 Traffic Volumes | 0 | 6 | 172 | 93 | 0 | 11 | 83 | 16 | 0 | 56 | 219 | 10 | 0 | 63 | 124 | 11 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | 1 | 0 | | | | 0 | | | 1 | 0 | | | 1 | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 0 | | | | 0 | | | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 0 | 9 | 1 | 0 | 0 | 0 | 6 | 0 | 5 | 6 | 0 | 0 | 1 | 6 | 0 |
| Heavy Vehicle % | 2% | 2% | 5% | 2% | 2% | 2% | 2% | 38% | 2% | 9% | 3% | 2% | 2% | 2% | 5% | 2% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 6 | 172 | 93 | 0 | 11 | 83 | 16 | 0 | 56 | 219 | 10 | 0 | 63 | 124 | 11 |
| | | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 1 | 16 | 9 | 0 | 1 | 8 | 1 | 0 | 5 | 20 | 1 | 0 | 6 | 12 | 1 |
| 1060 DLH DRI #4187 | <u> </u> | 1 | | | - | | - | <u> </u> | - | - | | | - | - | | <u> </u> |
| 2030 No-Build Traffic | 0 | 7 | 188 | 102 | 0 | 12 | 91 | 17 | 0 | 61 | 239 | 11 | 0 | 69 | 136 | 12 |
| 2030 No-Build Heavy Vehicle % | 2% | 2% | 5% | 2% | 2% | 2% | 2% | 38% | 2% | 9% | 3% | 2% | 2% | 2% | 5% | 2% |
| 2030 No-Build Pedestrians | 270 | 210 | 0 | 270 | 270 | 210 | 0 | 0070 | 2.10 | 770 | 0 | 2.70 | 2.10 | 2.70 | 0 | 270 |
| 2030 No-Build Conflicting Pedestrians | | 0 | Ĭ | 0 | | 0 | Ĭ | 0 | | 0 | 1 | 0 | | 0 | Î | 0 |
| 2030 No-Baild Connicting redestrians | | 0 | | 0 | 1 | 0 | | 0 | | 0 | 1 | U | | 0 | | 0 |
| Trip Distribution IN | 1 | 1 | 15% | | 1 | | | 1 | 1 | 16% | 1 | 1 | 1 | 1 | 1 | |
| Trip Distribution OUT | | | 1370 | | | | (15%) | (15%) | | 1376 | | | | | | |
| Palaasias Adverteent | | | | | | | (15%) | (15%) | | | | | | | | |
| Balancing Adjustment | 0 | 0 | 11 | 0 | 0 | 0 | 27 | 24 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 |
| Residential Trips | U | U | | U | U | U | 30 | 30 | U | | U | U | U | U | U | U |
| Trip Distribution IN | 1 | 1 | 1 | | 1 | | | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| Trip Distribution OUT | | | | | | | | | | | | | | | | |
| The distribution out | | | | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Hotel Trips | U | 0 | U | U | 0 | 0 | U | U | 0 | 0 | U | 0 | 0 | 0 | U | U |
| Tala Distally Alan IN | 1 | 1 | 1.00/ | | 1 | | | 1 | 1 | 150/ | 1 | 1 | 1 | 1 | 1 | 50/ |
| The Distribution IN | | | 10% | | | (50() | (100/) | (150) | - | 15% | | | | | | 5% |
| Trip Distribution OUT | L | | | | I | (5%) | (10%) | (15%) | | | | | | | | |
| Balancing Adjustment | - | - | - | 0 | | 0 | 0 | 0 | 0 | | 0 | 0 | 0 | 0 | 0 | 0 |
| ketali irips | U | U | U | U | U | U | U | U | U | 1 | U | U | U | U | U | U |
| Tele Distribution (A) | 1 | 1 | 100/ | | 1 | 1 | 1 | 1 | 1 | 150/ | 1 | | i | | 1 | 50/ |
| Trip Distribution IN | <u> </u> | | 10% | | l | (50/) | (100/) | (150) | | 15% | | | | | | 5% |
| Trip Distribution OUT | <u> </u> | | <u> </u> | | l | (5%) | (10%) | (15%) | | | | | | | | |
| Balancing Adjustment | - | | | | | | - | | | - · | | | | | | - |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Total Primary Site Trips | 0 | 0 | 11 | 0 | 0 | 0 | 36 | 36 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | <u>^</u> | | | | | | |
| Pass-By Trips 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | |
| Total vehicular Project Trips | 0 | 0 | 11 | 0 | 0 | 0 | 36 | 36 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0000 D 11 LT 17 | 1 | | 100 | 400 | | 10 | 107 | 50 | | | 000 | | | 10 | 101 | 10 |
| 2030 Build Traffic | 0 | 7 | 199 | 102 | 0 | 12 | 127 | 53 | 0 | 74 | 239 | 11 | 0 | 69 | 136 | 12 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 5% | 2% | 2% | 2% | 2% | 12% | 2% | 7% | 3% | 2% | 2% | 2% | 5% | 2% |
| 2030 Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | U | |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |

| | | | | | | PM PE | AK HOUR | 2 | | | | | | | | |
|---------------------------------------|--------|------------|-------------|-------|--------|-------------|-------------|-------|--------|------------|-----------|-------|--------|------------|-------------|-------|
| | C | happell Ro | i NW (South | 1) | (| Chappell Ro | I NW (North | 1) | Josep | h E. Boone | Blvd NW (| Nest) | Jose | oh E. Boon | e Blvd NW (| East) |
| | | North | hound | | | South | bound | | | Easti | bound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 14 | 154 | 43 | 0 | 8 | 310 | 53 | 0 | 15 | 125 | 10 | 0 | 101 | 208 | 12 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | 1 | 0 | 1 | | | 0 | | | | 0 | | | 1 | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 |
| Conflicting Bicycles | - | | _ | 0 | | | _ | 0 | - | | _ | 0 | | | | 1 |
| Heavy Vehicles | 0 | 1 | 2 | 0 | 0 | 0 | 5 | 0 | 0 | 2 | 7 | 1 | 0 | 3 | 8 | 0 |
| Heavy Vehicle % | 2% | 7% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 13% | 6% | 10% | 2% | 3% | 4% | 2% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 1/ | 154 | /13 | 0 | 8 | 310 | 53 | 0 | 15 | 125 | 10 | 0 | 101 | 208 | 12 |
| Adjusted 2024 Volumes | 0 | 14 | 134 | 45 | U | 0 | 510 | - 55 | Ū | 15 | 125 | 10 | 0 | 101 | 200 | 12 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 1 | 14 | 4 | 0 | 1 | 29 | 5 | 0 | 1 | 12 | 1 | 0 | 9 | 19 | 1 |
| 1060 DI H DRI #4187 | | | | | | · · · | | | Ŭ | · · · | | | , v | | | |
| 2030 No-Build Traffic | 0 | 15 | 168 | 47 | 0 | 9 | 330 | 58 | 0 | 16 | 137 | 11 | 0 | 110 | 227 | 13 |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2030 No-Build Pedestrians | | | 0 | | | - | 0 | | | - | 0 | | | | 0 | - |
| 2030 No-Build Conflicting Pedestrians | - | 0 | 1 | 0 | | 0 | 1 | 0 | | 0 | 1 | 0 | | 0 | 1 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | 15% | | | 1 | | 1 | | 15% | | | | | | |
| Trip Distribution OUT | | | | | | | (15%) | (15%) | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 32 | 0 | 0 | 0 | 21 | 21 | 0 | 32 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | 10% | | | | | | | 15% | | | | | | 5% |
| Trip Distribution OUT | | | | | | (5%) | (10%) | (15%) | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 1 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | 10% | | | (500) | (4.000) | (450) | | 15% | | | | | | 5% |
| Trip Distribution OUT | | | 1 | | | (5%) | (10%) | (15%) | | | 1 | | | | | |
| Balancing Adjustment | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Restaurant mps | 0 | U | U | U | U | 0 | U | U | 0 | 0 | U | U | U | U | U | U |
| Total Brimany Sito Tring | 0 | 0 | 22 | 0 | 0 | 0 | 22 | 22 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 1 |
| Total Philliary Site Trips | 0 | U | 33 | U | U | 0 | 22 | 22 | 0 | 34 | U | U | U | U | U | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1 435 63 11165 | Ū | U | Ŭ | 0 | | U | Ŭ | Ū | | U | Ŭ | 5 | 0 | 0 | Ū | 0 |
| Total Vehicular Project Trips | | 0 | 33 | 0 | 0 | 0 | 22 | 22 | 0 | 34 | 0 | 0 | 0 | 0 | 0 | 1 |
| | | | | | | | | | | | , - | - | | | , - | |
| 2030 Build Traffic | 0 | 15 | 201 | 47 | 0 | 9 | 361 | 80 | 0 | 50 | 137 | 11 | 0 | 110 | 227 | 14 |
| 2030 Build Heavy Vehicle % | 2% | 7% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 4% | 6% | 10% | 2% | 3% | 4% | 2% |
| 2030 Build Pedestrians | | | Ö | | | | Ö | | | | Ó | | | | Ó | |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| | | | | | | | | | | | | | | | | |

| INTERSECTION VOL | UME DEVELOPMENT |
|--|--|
| INTERS | CTION #5 |
| Joseph E. Boone Blvd NW (West)/Joseph E. Boone Blv | d NW (East) at Burbank Dr NW/Mayson Turner Rd NW |

| Image: state interview | | - | | | | | AM PE | ak houf | 2 | | | | | | | | |
|--|---------------------------------------|------------|--------|---------|-------|----------|-----------|------------|-------|---|------------|--------------|-------|--------|-------------|-------------|---------|
| Ulum Ulum Ulum Number of the source of | | | Burban | k Dr NW | | | Mayson Tu | rner Rd NV | 1 | Josep | h E. Boone | e Blvd NW () | Nest) | Jose | oh E. Boone | e Blvd NW (| East) |
| U-Turn Left Invarge Right U-Turn Left Inv | | | North | bound | | | South | bound | | | East | bound | | | West | bound | |
| bischer 2021 11 43 1 42 7 10 0 5 273 6 4 25 11 45 opticity Restrictions Optics 0 | | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Durit Balancing opticiting Pectatrians Opticiting Pectatrians Optici | Observed 2024 Traffic Volumes | 0 | 20 | 11 | 43 | 1 | 62 | 7 | 10 | 0 | 5 | 273 | 6 | 4 | 25 | 191 | 50 |
| odd odd <td>Count Balancing</td> <td></td> | Count Balancing | | | | | | | | | | | | | | | | |
| officies productions (spices support of the spice spice support of the spice support of the spice support of the | Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Opeles O <td>Conflicting Pedestrians</td> <td></td> <td>0</td> | Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| cmlicing laycles c c 0 - 0 - 0 - 0 0 - 0 0 - 0 | Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0 2 0 0 2 0 0 0 0 5 2 1 1 5 2 2% 10% 2% 15% 15% 15% 15% 15% 15% 15% 15% 15% 15% 15% 15% 15% 15% 15%< | Conflicting Bicycles | | | | 0 | | | | 0 | | | | 0 | | | | 0 |
| base yes 10% 2% 10% 2% 2% 3% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 3% 2% 2% 3% 2% 3% 2% 2% 2% 3% 2% 2% 3% 2% 2% 3% 2% 3% 4% 3% 4% 3% 4% 3% 4% 3% 1% 1 | Heavy Vehicles | 0 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 5 | 2 | 1 | 1 | 5 | 2 |
| Dip Dip <thdip< th=""> <thdip< th=""> <thdip< th=""></thdip<></thdip<></thdip<> | Heavy Vehicle % | 2% | 10% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 33% | 25% | 4% | 3% | 4% |
| instant Pactor 0.1 | Peak Hour Factor | 0.70 | 0.70 | 0.70 | 0.79 | 0.70 | 0.70 | 0.79 | 0.79 | 0.70 | 0.70 | 0.79 | 0.70 | 0.70 | 0.70 | 0.70 | 0.79 |
| justed 2024 volumes 0 20 11 43 1 62 7 1 0 1 4 25 101 50 musi Growth Rate (orwth Factor step/and Growth Trips 1.5% 1.09 | Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| operation 2.0 2.0 1.1 0.2 7 1.0 0 5 2.3 0 4 2.3 1.5% | Adjusted 2024 Volumos | 0 | 20 | 11 | 42 | 1 | 62 | 7 | 10 | 0 | 5 | 272 | 6 | 4 | 25 | 101 | 50 |
| Institution IN possibilition IN po | Aujusted 2024 Volumes | 0 | 20 | | 43 | | 02 | / | 10 | 0 | 5 | 213 | U | 4 | 23 | 171 | 50 |
| 100 109 <td>Annual Growth Rate</td> <td>1.5%</td> | Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| D 2 1 4 0 6 1 1 0 0 26 1 0 2 18 5 300 No-Build Traffic 0 22 12 47 1 68 8 11 0 5 299 7 4 27 209 55 300 No-Build Perky Vehicle % 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 3% 4% 3% | Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Distribution NU possibility Distribution NU possibility <t< td=""><td>Background Growth Trips</td><td>0</td><td>2</td><td>1</td><td>4</td><td>0</td><td>6</td><td>1</td><td>1</td><td>0</td><td>0</td><td>26</td><td>1</td><td>0</td><td>2</td><td>18</td><td>5</td></t<> | Background Growth Trips | 0 | 2 | 1 | 4 | 0 | 6 | 1 | 1 | 0 | 0 | 26 | 1 | 0 | 2 | 18 | 5 |
| Bit No-Build Traffic 0 22 12 47 1 68 8 11 0 5 299 7 4 27 209 55 300 No-Build Pedestrians 0 2% | 1060 DI H DRI #4187 | | - | | | | | | | | - | | | - | - | 1 | - |
| 200 No-Build Heavy Vehicle % 200 M 200 M <th< td=""><td>2030 No-Build Traffic</td><td>0</td><td>22</td><td>12</td><td>47</td><td>1</td><td>68</td><td>8</td><td>11</td><td>0</td><td>5</td><td>299</td><td>7</td><td>4</td><td>27</td><td>209</td><td>55</td></th<> | 2030 No-Build Traffic | 0 | 22 | 12 | 47 | 1 | 68 | 8 | 11 | 0 | 5 | 299 | 7 | 4 | 27 | 209 | 55 |
| Data Loss Values Arg Loss Loss <thloss< th=""> <thloss< th=""> Loss<td>2030 No-Build Heavy Vehicle %</td><td>2%</td><td>10%</td><td>2%</td><td>2%</td><td>2%</td><td>3%</td><td>2%</td><td>2%</td><td>2%</td><td>2%</td><td>2%</td><td>33%</td><td>25%</td><td>4%</td><td>20/</td><td>4%</td></thloss<></thloss<> | 2030 No-Build Heavy Vehicle % | 2% | 10% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 2% | 2% | 33% | 25% | 4% | 20/ | 4% |
| Oor Mount Generation O | 2030 No-Build Pedestrians | 270 | 1070 | 0 | 270 | 2.70 | 370 | 0 | 270 | 2.70 | 270 | 0 | 3370 | 2070 | 470 | 0 | 470 |
| Construction N Constru | 2030 No-Build Conflicting Redestrippe | | 0 | Ĭ | 0 | | 0 | Ĭ | 0 | | 0 | Î | 0 | | 0 | Ĭ | 0 |
| Ip Distribution NU Image: Construction NU <thimage: construction="" nu<="" th=""> Image:</thimage:> | 2000 No-Build Commening readStrial15 | | U | | U | | U | | U | | 0 | | 0 | | U | | U |
| ip Distribution OUT Image: Construction of the second | Trip Distribution IN | 1 | | 1 | | 1 | | 1 | 1 | 1 | | 1 | | 1 | | 1 | 20% |
| Description Image: Constraint of the second se | Trip Distribution OUT | | | | | | (20%) | | | | | | | | | | 2070 |
| Data Data Muschinem O | Palancing Adjustment | | | | | | (2070) | | | | | | | | | | |
| Sadelina rings 0 | Desidential Trins | 0 | 0 | 0 | 0 | 0 | 46 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 |
| Image: polymetry of p | Residential mps | 0 | 0 | 0 | 0 | 0 | 40 | 0 | 0 | 0 | 0 | 0 | U | U | 0 | 0 | 15 |
| ip Distribution OUT alancing Adjustment old Trigs image: height of the second out of the second out of the second polisification IN ip Distribution OUT alancing Adjustment alancing Adjustment ip Distribution OUT alancing Adjustment etail Trips image: height out out out out out out out out out ou | Trip Distribution IN | | | | | | | | | | | | | | | | |
| Image: state of the s | Trip Distribution OUT | | | | | | | | | | | | | | | | |
| otel Trips 0 | Balancing Adjustment | | | | | | | | | | | | | | | | |
| Image Image <th< td=""><td>Hotel Trips</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></th<> | Hotel Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ip Distribution N Image: Stribution OT Image: Stribution OT <thimage: ot<="" stribution="" th=""> Image: Stribu</thimage:> | | | - | - | - | | - | - | | | - | - | | | - | | - |
| Image: split registring Image: split r | Trip Distribution IN | | | | | | | | | | | | | | | 5% | 25% |
| Jancing Adjustment teal Trips Image: Construction of the second sec | Trip Distribution OUT | | | | | | (25%) | | | | | (5%) | | | | | |
| etail Trips 0 <th< td=""><td>Balancing Adjustment</td><td></td><td></td><td></td><td></td><td></td><td>,,</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<> | Balancing Adjustment | | | | | | ,, | | | | | | | | | | |
| i i <td>Retail Trips</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>1</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>1</td> | Retail Trips | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| Image: pipe distribution N Image: pipe distribution N <th< td=""><td></td><td>+ <u> </u></td><td></td><td>+</td><td></td><td>•</td><td>· · ·</td><td>·</td><td>·</td><td><u>، </u></td><td></td><td>+</td><td></td><td></td><td></td><td>+</td><td>· · · ·</td></th<> | | + <u> </u> | | + | | • | · · · | · | · | <u>، </u> | | + | | | | + | · · · · |
| Implicitivition OUT Implicit (25%) Implicit (25%) Implicit (5%) Implic | Trip Distribution IN | 1 | | | | | | | | | | | | | | 5% | 25% |
| Image: staurant Trips | Trip Distribution OUT | | | | | | (25%) | | | | | (5%) | | | | | |
| 0 | Balancing Adjustment | | | | | | | | | | | | | | | | |
| Dtal Primary Site Trips 0 0 0 0 0 47 0 | Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Dtal Primary Site Trips 0 0 0 0 0 47 0 <td></td> | | | | | | | | | | | | | | | | | |
| ass-By Trips 0 <t< td=""><td>Total Primary Site Trips</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>47</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>16</td></t<> | Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 16 |
| ass-sy rinps 0 <t< td=""><td>Deer Du Teler</td><td><u>^</u></td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></t<> | Deer Du Teler | <u>^</u> | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Data Vehicular Project Trips 0 0 0 0 47 0 0 0 0 0 0 0 16 308 Build Traffic 0 22 12 47 1 115 8 11 0 5 299 7 4 27 209 71 308 Build Traffic 0 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 31% 27% 4% 3%< | Pass-ву тrips | U | U | U | U | U | U | U | U | U | U | U | U | U | U | U | U |
| Dailid Traffic 0 22 12 47 1 115 8 11 0 5 299 7 4 27 209 71 303 Build Traffic 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 31% 33% 3% <td< td=""><td>Total Vehicular Project Trips</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>47</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>16</td></td<> | Total Vehicular Project Trips | 0 | 0 | 0 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 16 |
| 330 Build Traffic 0 22 12 47 1 115 8 11 0 5 299 7 4 27 209 71 330 Build Traffic 2% 10% 2% 3% < | · · | | · | · | | <u> </u> | · | · | · | <u> </u> | | <u> </u> | | | · | · | · |
| 330 Build Heavy Vehicle % 2% 10% 2% 2% 2% 2% 2% 2% 2% 2% 3% <th< td=""><td>2030 Build Traffic</td><td>0</td><td>22</td><td>12</td><td>47</td><td>1</td><td>115</td><td>8</td><td>11</td><td>0</td><td>5</td><td>299</td><td>7</td><td>4</td><td>27</td><td>209</td><td>71</td></th<> | 2030 Build Traffic | 0 | 22 | 12 | 47 | 1 | 115 | 8 | 11 | 0 | 5 | 299 | 7 | 4 | 27 | 209 | 71 |
| 303 Build Pedestrians 0 | 2030 Build Heavy Vehicle % | 2% | 10% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 31% | 27% | 4% | 3% | 3% |
| 130 Build Conflicting Pedestrians 0 0 0 0 0 0 0 0 0 0 0 | 2030 Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| | 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |

| | | | | | | PM PE | AK HOUR | | | | | | | | | | | | | |
|---|--------|--------|---------|-------|---|-----------|------------|-------|--------|------------|------------|-------|--------|-------------|-------------|--|--|--|--|--|
| | | Burban | k Dr NW | | | Mayson Tu | rner Rd NW | | Josep | h E. Boone | Blvd NW (\ | Nest) | Jose | oh E. Boone | e Blvd NW (| East) | | | | |
| | | North | bound | | | South | bound | | | Easti | bound | | | West | bound | | | | | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | ast) Right 79 0 0 0 2 3% 0.90 1 79 1.5% 1.5% 1.5% 1.0% 7 7 7 86 0 0 20% 43 25% 3 25% 0 0 0 0 0 0 0 0 0 0 0 0 0 | | | | |
| Observed 2024 Traffic Volumes | 0 | 18 | 24 | 24 | 0 | 58 | 43 | 15 | 1 | 2 | 147 | 15 | 1 | 33 | 272 | 79 | | | | |
| Count Balancing | | | | | | | | | | | | | | | | | | | | |
| Pedestrians | | 1 | 0 | | | | 0 | | | 1 | 0 | | | |) | | | | | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | | | |
| Bicycles | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | | | | |
| Conflicting Bicycles | | Ŭ | | 1 | , i i i i i i i i i i i i i i i i i i i | Ŭ | 0 | 0 | | Ŭ | 0 | 0 | | | Ū | 0 | | | | |
| Heavy Vehicles | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 8 | 2 | | | | |
| Heavy Vehicle % | 2% | 296 | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 2% | 2% | 2% | 2% | 2% | | | | |
| Peak Hour Factor | 0.00 | 0.90 | 0.90 | 0.00 | 0.90 | 0.90 | 0.90 | 0.90 | 0.00 | 0.90 | 0.00 | 0.00 | 0.00 | 0.90 | 0.00 | 0.00 | | | | |
| Adjustment Easter | 0.70 | 1 | 1 | 1 | 0.70 | 0.70 | 1 | 1 | 1 | 1 | 0.70 | 1 | 0.70 | 1 | 0.70 | 1 | | | | |
| Adjusted 2024 Volumes | 0 | 10 | 24 | 24 | 0 | E0 | 42 | 16 | 1 | 2 | 147 | 16 | 1 | 22 | 1 | 70 | | | | |
| Adjusted 2024 Volumes | 0 | 10 | 24 | 24 | 0 | 30 | 43 | IJ | | 2 | 147 | 15 | | 33 | 212 | 17 | | | | |
| Appual Growth Pate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | | | | |
| Crowth Eactor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | | | | |
| Backaround Growth Trins | 0 | 1.07 | 1.07 | 2 | 0 | 5 | 1.07 | 1.07 | 0 | 0 | 1.07 | 1.07 | 0 | 3 | 25 | 7 | | | | |
| 1040 DLH DDL #4197 | 0 | 2 | 2 | 2 | 0 | 5 | 4 | | J | 0 | 14 | 1 | J | 3 | 20 | 1 | | | | |
| 1000 DLFI DRI #4167 2020 No. Duild Troffic | 0 | 20 | 26 | 24 | 0 | 40 | 47 | 14 | 1 | 2 | 141 | 14 | 1 | 24 | 207 | 04 | | | | |
| 2030 No-Dullu Hama Vahiala W | 0 | 20 | 20 | 20 | 0 | 03 | 4/ | 10 | 0 | 2 | 101 | 10 | 0 | 30 | 297 | 00 | | | | |
| 2030 No-Build Redestrians | 0 | U | 0 | U | 0 | 0 | 0 | U | 0 | U | 0 | U | 0 | 0 | | U | | | | |
| 2030 No-Build Conflicting Redestrians | - | 0 | 1 | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | 5 | 0 | | | | |
| 2030 No-Build Connicting Pedestrians | | U | 1 | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | | | |
| Trin Distribution IN | 1 | | | | 1 | | 1 | | | | | | | | | 20% | | | | |
| Trip Distribution OUT | - | | | | | (20%) | | | | | | | | | | 2070 | | | | |
| Palancing Adjustment | | | | | | (2070) | | | | | | | | | | | | | | |
| Desidential Tring | 0 | 0 | 0 | 0 | 0 | 27 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 42 | | | | |
| Residential mps | 0 | 0 | 0 | 0 | 0 | 21 | U | U | 0 | 0 | 0 | U | 0 | 0 | 0 | 43 | | | | |
| Trip Distribution IN | | | 1 | | | | | | | | | | | | 5% | 25% | | | | |
| Trip Distribution OUT | - | | | | | (25%) | | | | | (5%) | | | | 570 | 2370 | | | | |
| Balancing Adjustment | - | | | | | (2070) | | | | | (070) | | | | | | | | | |
| Potail Trins | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | | | | |
| | v | Ŭ | Ŭ | 0 | v | - | Ū | Ū | 0 | Ŭ | Ū | 0 | 0 | 0 | | 0 | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | 5% | 25% | | | | |
| Trip Distribution OUT | | | 1 | | l | (25%) | | | | | (5%) | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | | |
| • | | | 1 | | • | | | | | | | | | | · · · · · | | | | | |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 46 | | | | |
| | | | | | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | | | | |
| | | | | | | | | | | | | | | | | | | | | |
| Total Vehicular Project Trips | | 0 | 0 | 0 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 46 | | | | |
| | | | | | | | | | | | | | | | | | | | | |
| 2030 Build Traffic | 0 | 20 | 26 | 26 | 0 | 92 | 47 | 16 | 1 | 2 | 161 | 16 | 1 | 36 | 298 | 132 | | | | |
| 2030 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 2% | 2% | 2% | 3% | 2% | | | | |
| 2030 Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | | | | | |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | | | |
| | | | | | | | | | | | | | | | | | | | | |

INTERSECTION VOLUME DEVELOPMENT INTERSECTION #6 North Avenue NW (West)/Driveway (East) at Driveway (South)/North Avenue NW (North)

| | | | | | | AM PE | AK HOUR | | | | | | | | | |
|--|--------|---------|-----------|-------|--|------------|------------|-------|--------|------------|-----------|-------|--------|----------|-----------|-------|
| | | Drivewa | y (South) | | N | orth Avenu | e NW (Nort | h) | N | orth Avenu | e NW (Wes | t) | | Drivewa | ay (East) | |
| | | North | bound | | | South | bound | | | Eastb | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 9 | 4 | 1 | 0 | 7 | 2 | 17 | 0 | 24 | 7 | 7 | 0 | 0 | 14 | 16 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | 1 |) | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 0 | | | | 0 | | | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 7 | 0 | 0 | 1 | 0 | 0 | 0 | 9 | 12 |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 29% | 2% | 41% | 2% | 2% | 14% | 2% | 2% | 2% | 64% | 75% |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 9 | 4 | 1 | 0 | 7 | 2 | 17 | 0 | 24 | 7 | 7 | 0 | 0 | 14 | 16 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 2 | 0 | 2 | 1 | 1 | 0 | 0 | 1 | 1 |
| 1060 DI H DRI #4187 | | · · · | Ŭ | , v | , in the second se | · · · | | - | Ŭ | - | | | , v | | - · | |
| 2030 No-Build Traffic | 0 | 10 | 4 | 1 | 0 | 8 | 2 | 19 | 0 | 26 | 8 | 8 | 0 | 0 | 15 | 17 |
| 2030 No-Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 29% | 2% | 41% | 2% | 2% | 14% | 2% | 2% | 2% | 64% | 75% |
| 2030 No-Build Pedestrians | 270 | 270 | 0 | 270 | 270 | 2770 | 0 | 4170 | 270 | 270 | 1470 | 270 | 270 | 270 | 0 0 4 70 | 7370 |
| 2030 No-Build Conflicting Pedestrians | | 0 | Ĭ | 0 | | 0 | Ĭ | 0 | | 0 | | 0 | | 0 | Ĭ | 0 |
| coor no baile connecting recessions | | 0 | | 0 | | Ŭ | | 0 | | 0 | | 0 | | 0 | 1 | 0 |
| Frip Distribution IN | | | | | | | | 20% | | | | | | | | |
| Trip Distribution OUT | | | | | | | | | | (20%) | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | | |
| Frip Distribution OUT | | | | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Hotel Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| rip Distribution IN | | | | | | | | 10% | | | | | | | | |
| rip Distribution OUT | | | | | | | | | | (10%) | | | | | | |
| 3alancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| Frin Distribution IN | 1 | 1 | 1 | | 1 | r | , | 10% | 1 | 1 | - | | 1 | r | | |
| Trip Distribution OLIT | | | | | | | | 10% | | (10%) | | | | + | | |
| Palancing Adjustment | | | | | | | | | | (10%) | | | | <u> </u> | | |
| Datanung Aujustinent Doctouropt Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Restaurant mps | U | U | U | U | U | U | U | U | U | U | U | U | U | U | U | U |
| Fotal Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 48 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Total Vehicular Project Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 48 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2030 Build Traffic | 0 | 10 | 4 | 1 | 0 | 8 | 2 | 34 | 0 | 74 | 8 | 8 | 0 | 0 | 15 | 17 |
| 2030 Duild Hamu Vohielo % | 20/ | 20% | 4 | 20/ | 20% | 270/ | 2 | 220/ | 20% | 20/ | 1.40/ | 20% | 20% | 20/ | 440/ | 77% |
| 2030 Build Redoctriane | 270 | 270 | 270 | 270 | 270 | 2170 | 270 | 2370 | 270 | 270 | 1470 | 270 | 270 | 270 | 00% | 1170 |
| 2030 Build Conflicting Dodoctrians | | 0 | 1 | 0 | | 0 | | 0 | | 0 | , | 0 | | 0 | , | 0 |
| 2030 build connicting redestrials | | U | | U | | U | | U | | U | | U | | U | | U |

| | | | | | | PM PE | ak houf | | | | | | | | | |
|---------------------------------------|--------|---------|------------|-------|----------|------------|------------|-------|--------|------------|-----------|-------|--------|---------|-----------|-------|
| | | Drivewa | ıy (South) | | N | orth Avenu | e NW (Norl | h) | N | orth Avenu | ie NW (We | st) | | Drivewa | ay (East) | |
| | | North | nbound | | | South | bound | | | East | bound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 8 | 3 | 2 | 0 | 1 | 1 | 272 | 0 | 34 | 3 | 7 | 0 | 1 | 3 | 2 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | - | | | 0 | | | | 0 | | | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 0 |
| Heavy Vehicle % | 2% | 13% | 33% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 67% | 2% | 2% | 2% | 33% | 2% |
| Peak Hour Factor | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 8 | 3 | 2 | 0 | 1 | 1 | 272 | 0 | 34 | 3 | 7 | 0 | 1 | 3 | 2 |
| | | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 25 | 0 | 3 | 0 | 1 | 0 | 0 | 0 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 0 | 9 | 3 | 2 | 0 | 1 | 1 | 297 | 0 | 37 | 3 | 8 | 0 | 1 | 3 | 2 |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |
| 2030 No-Build Pedestrians | | 0 | 0 | 0 | | 0 | 0 | 0 | | 0 | 0 | 0 | | | 0 | 0 |
| 2030 No-Build Conflicting Pedestrians | | U | | U | | 0 | | U | | 0 | | U | | 0 | | U |
| Trip Distribution IN | 1 | 1 | 1 | 1 | I | 1 | 1 | 20% | I | 1 | 1 | 1 | I | 1 | 1 | 1 |
| Trip Distribution OLIT | - | | | | | | | | | (20%) | | | | | | |
| Balancing Adjustment | - | | | | | | | | | (2070) | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 42 | 0 | 28 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | 10% | | | | | | | | |
| Trip Distribution OUT | | | | | | | | | | (10%) | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | 10% | | | | | | | | |
| Trip Distribution OUT | - | | | | | | | | | (10%) | | | | | | |
| Balancing Adjustment | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Restaurant Trips | U | U | U | U | U | 0 | U | U | 0 | 0 | U | U | 0 | U | U | U |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 43 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 |
| Dava Du Teles | 0 | 0 | 0 | 0 | <u> </u> | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-By Trips | U | 0 | 0 | U | 0 | 0 | 0 | U | 0 | 0 | 0 | 0 | 0 | 0 | 0 | U |
| Total Vehicular Project Trips | | 0 | 0 | 0 | 0 | 0 | 0 | 43 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 |
| | • | | | | • | • | | | • | • | | • | • | • | | • |
| 2030 Build Traffic | 0 | 9 | 3 | 2 | 0 | 1 | 1 | 340 | 0 | 66 | 3 | 8 | 0 | 1 | 3 | 2 |
| 2030 Build Heavy Vehicle % | 2% | 12% | 36% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 73% | 2% | 2% | 2% | 36% | 2% |
| 2030 Build Pedestrians | | | Ó | | | | 0 | | | | Ó | | | | Ó | |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| | | | | | | | | | | | | | | | | |

| INTERSECTION VOLUME DEVELOPMENT |
|---|
| INTERSECTION #7 |
| GA-8 Donald Lee Hollowell Pkwy NW (West)/GA-8 Donald Lee Hollowell Pkwy NW (East) at Pierce Ave NW/Driveway |

| | | | | | | AM PE | AK HOUR | | | | | | | | | |
|---------------------------------------|--------|--------|---------|-------|--------|-------|---------|-------|--------|-------------|--------------|-------|-----------|-------------|---|-----------|
| | | Pierce | Ave NW | | | Driv | eway | | GA-8 D | onald Lee H | Hollowell Pk | wy NW | GA-8 Dona | Id Lee Holl | owell Pkwy | NW (East) |
| | | North | bound | | | South | bound | | | East | oound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 12 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 1,153 | 4 | 0 | 4 | 406 | 0 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | (| 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Conflicting Bicycles | | | | 0 | | | | 0 | | | | 1 | | | | 1 |
| Heavy Vehicles | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 58 | 0 | 0 | 0 | 36 | 0 |
| Heavy Vehicle % | 2% | 2% | 2% | 7% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 2% | 2% | 2% | 9% | 2% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 12 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 1,153 | 4 | 0 | 4 | 406 | 0 |
| Appual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.00 | 1.00 | 1.0% | 1.00 | 1.00 | 1.00 | 1.0/0 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Background Growth Trins | 0 | 1.07 | 0 | 1.07 | 0 | 0 | 0 | 0 | 0 | 0 | 108 | 0 | 0 | 0 | 38 | 0 |
| 1060 DLH DPL #4187 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | U | J | 0 | 64 | J | J | J | 30 | J |
| 2020 No Puild Traffic | 0 | 12 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 1 2 2 5 | 4 | 0 | 4 | 30 | 0 |
| 2030 No Build Lloove Vehicle 9/ | 20/ | 13 | 20/ | 10 | 20/ | 20/ | 20/ | 20/ | 20/ | 20% | 1,323 | 4 | 20/ | 4 | 4/4 | 20/ |
| 2030 No-Build Redoctrianc | 270 | 270 | 270 | 170 | 270 | 270 | 270 | 270 | 270 | 270 | 3% | 270 | 270 | 270 | 976 | 270 |
| 2030 No-Build Conflicting Redectrians | | 0 | | 0 | | 0 | | 0 | | 0 | 1 | 0 | | 0 | | 0 |
| 2030 No-Balla Conneting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | U | | U | | U |
| Trip Distribution IN | 1 | | | | 1 | | | | | 1 | | | 1 | | 15% | |
| Trip Distribution OUT | | | | | | | | | | | (15%) | | | | 1370 | |
| Palancing Adjustment | | | | | | | | | | | (1370) | | | | | |
| Decidential Trins | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 26 | 0 | 0 | 0 | 11 | 0 |
| Residential mps | 0 | 0 | 0 | U | 0 | 0 | 0 | 0 | U | 0 | 30 | U | U | U | | U |
| Trip Distribution IN | 1 | 1 | | 1 | 1 | 1 | | | | | 1 | | 1 | 1 | <u> </u> | |
| Trip Distribution OUT | | | | | | | | | | | | | | | | |
| Delensing Adjustment | | | | | | | | | | | | | | | | |
| Balancing Aujustment | • | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Hotel Trips | 0 | 0 | 0 | U | 0 | 0 | 0 | 0 | 0 | 0 | U | U | U | U | U | U |
| Trip Distribution IN | 1 | | | | | | | | | | | | | | 20% | |
| Trip Distribution OUT | | | | | | | | | | | (20%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | (==:=) | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| | | | | | | | | | | | | | | | | - |
| Trip Distribution IN | | | | | | | | | | | | | | | 20% | |
| Trip Distribution OUT | | | | | | | | | | | (20%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 37 | 0 | 0 | 0 | 12 | 0 |
| | | | | | | | | 2 | 5 | | | 5 | | | | 5 |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | , <u>, , , , , , , , , , , , , , , , , , </u> | |
| Total Vehicular Project Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 37 | 0 | 0 | 0 | 12 | 0 |
| 2020 Build Troffic | 0 | 12 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 1 242 | 4 | 0 | 4 | 494 | 0 |
| 2030 Build Hame Vahielo % | 20/ | 13 | 20/ | 10 | 20% | 20% | 20/ | 20/ | 20/ | 20% | 1,30Z | 4 | 200 | 4 | 400 | 20% |
| 2030 Build Redoctrians | 270 | 270 | 270 | 170 | 270 | 270 | 270 | 270 | 270 | 270 | 3% | 270 | 270 | 270 | 070 | 270 |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | 1 | 0 | | 0 | i i | 0 |
| 2000 Bana Commenny i Caestrialis | | 0 | | v | | 0 | | 0 | | 0 | | 0 | | v | I | 0 |

| | PM PEAK HOUR Pierce Ave NW Driveway GA-8 Donaid Lee Hollowell Pkwy NW GA-8 Donaid Lee Hollowell Pkwy Northbound Southbound Eastbound Westbound | | | | | | | | | | | | | | | |
|--|--|----------|---------|-------|--------|-------|---------|-------|--------|-------------|--------------|-------|-----------|-------------|------------|------------|
| | | Pierce / | Ave NW | | | Drive | eway | | GA-8 D | onald Lee H | Iollowell Pl | wy NW | GA-8 Dona | ld Lee Holl | owell Pkwy | /NW (East) |
| | | North | bound | | | South | bound | | | Easth | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 10 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 624 | 7 | 0 | 15 | 1,153 | 0 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 0 | | | | 0 | | | | 1 | | | | 0 |
| Heavy Vehicles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 33 | 0 | 0 | 0 | 60 | 0 |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 2% | 2% | 2% | 5% | 2% |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 10 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 624 | 7 | 0 | 15 | 1,153 | 0 |
| | | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 58 | 1 | 0 | 1 | 108 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | 32 | | | | 63 | |
| 2030 No-Build Traffic | 0 | 11 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 714 | 8 | 0 | 16 | 1,324 | 0 |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2030 No-Build Pedestrians 2020 No Build Conflicting Redestrians | | 0 | J | 0 | | 0 | J | 0 | | 0 | 0 | 0 | | 0 | 0 | 0 |
| 2030 No-Build Connicting Pedestrians | | U | | U | | U | | U | | U | | U | | U | | U |
| Trip Distribution IN | | | | | | | | | | 1 | | 1 | | | 15% | |
| Trip Distribution OUT | | | | | | | | | | | (15%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 21 | 0 | 0 | 0 | 32 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | 20% | |
| Trip Distribution OUT | | | | | | | | | | | (20%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 2 | 0 |
| Trip Distribution IN | | | | | | | | | r | 1 | 1 | 1 | r | | 20% | |
| Trip Distribution OUT | | | | | | | | | | | (20%) | | | | 2070 | |
| Balancing Adjustment | | | | | | | | | | | (2070) | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | 1 | | | | | |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 0 | 0 | 34 | 0 |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1 435 05 11105 | Ū | Ū | Ŭ | Ū | Ū | Ū | 0 | Ū | 0 | 0 | 0 | 0 | 0 | Ū | Ŭ | |
| Total Vehicular Project Trips | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 0 | 0 | 34 | 0 |
| | | | | | | | | | | | | | | | | |
| 2030 Build Traffic | 0 | 11 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 736 | 8 | 0 | 16 | 1,358 | 0 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 2% | 2% | 2% | 5% | 2% |
| 2030 Build Conflicting Redestrians | | 0 | J | 0 | | 0 | J | 0 | | 0 | U | 0 | | 0 | 0 | 0 |
| 2000 Bulla Committing Pedesti falls | | J | | J | | J | | J | | 0 | | 0 | | J | ! | 0 |

INTERSECTION VOLUME DEVELOPMENT INTERSECTION #8 Chappell Rd NW (South)/Chappell Rd NW (North) at Kennesaw Dr NW (West)/Site Driveway C

| | | | | | | AM PE | AK HOUR | | | | | | | | | |
|---------------------------------------|--------|-------------|-----------|-------|--------|-------------|-----------|-------|--------|------------|------------|-------|--------|----------|---------|-------|
| | (| Chappell Rd | NW (South |) | (| Chappell Rd | NW (North |) | K | Cennesaw D | r NW (West | .) | | Site Dri | veway C | |
| | | North | bound | | | South | bound | | | Eastb | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 5 | 296 | 0 | 0 | 0 | 167 | 2 | 0 | 5 | 0 | 6 | 0 | 0 | 0 | 0 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | (|) | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 0 | | | | 0 | | | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 1 | 14 | 0 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Heavy Vehicle % | 2% | 20% | 5% | 2% | 2% | 2% | 7% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% |
| Peak Hour Factor | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 5 | 296 | 0 | 0 | 0 | 167 | 2 | 0 | 5 | 0 | 6 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 0 | 28 | 0 | 0 | 0 | 16 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 0 | 5 | 324 | 0 | 0 | 0 | 183 | 2 | 0 | 5 | 0 | 7 | 0 | 0 | 0 | 0 |
| 2030 No-Build Heavy Vehicle % | 2% | 20% | 5% | 2% | 2% | 2% | 7% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% |
| 2030 No-Build Pedestrians | | | 0 | | | | 0 | | | (|) | | | | 0 | |
| 2030 No-Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| | | | | | _ | | | | _ | | | | _ | | | |
| Trip Distribution IN | | | | 25% | | 15% | | | | | | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (25%) | | (15%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 18 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 59 | 0 | 36 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | | |
| Frip Distribution OUT | | | | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Hotel Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | 40% | | 20% | | | | | | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (40%) | | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| | | | | | _ | | | | _ | | | | _ | | | |
| Trip Distribution IN | | | | 40% | | 20% | | | | | | | | | | |
| Frip Distribution OUT | | | | | | | | | | | | | | (40%) | | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Total Primary Site Trips | 0 | 0 | 0 | 20 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 60 | 0 | 37 |
| | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| · · · · · · · · · · · · · · · · · · · | • | | | | · | | | | | | · · · · | | · | | | · |
| Total Vehicular Project Trips | 0 | 0 | 0 | 20 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 60 | 0 | 37 |
| | | | | | | | | | | | | | | | | |
| 2030 Build Traffic | 0 | 5 | 324 | 20 | 0 | 12 | 183 | 2 | 0 | 5 | 0 | 7 | 0 | 60 | 0 | 37 |
| 2030 Build Heavy Vehicle % | 2% | 22% | 5% | 2% | 2% | 2% | 7% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% |
| 2030 Build Pedestrians | | | 0 | | | | 0 | | | (|) | | | | 0 | |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |

| | | | | | | PM PE | ak hour | | | | | | | | | |
|---------------------------------------|--------|-------------|-------------|-------|--------|-------------|-----------|-------|--------|------------|-----------|-------|--------|----------|---------|--------|
| | (| Chappell Ro | l NW (South | 1) | (| Chappell Rd | NW (North | i) | K | (ennesaw D | r NW (Wes | t) | | Site Dri | veway C | |
| | | North | nbound | | | South | bound | | | East | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 6 | 283 | 0 | 0 | 0 | 499 | 4 | 0 | 4 | 0 | 3 | 0 | 0 | 0 | 0 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | 0 | | | | 0 | | | | 0 | | | | 0 | |
| Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Bicycles | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | | | 1 | | | | 0 | | | | 0 | | | | 0 |
| Heavy Vehicles | 0 | 0 | 10 | 0 | 0 | 0 | 12 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |
| Heavy Vehicle % | 2% | 2% | 4% | 2% | 2% | 2% | 2% | 2% | 2% | 25% | 2% | 2% | 2% | 2% | 2% | 2% |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 6 | 283 | 0 | 0 | 0 | 499 | 4 | 0 | 4 | 0 | 3 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 1 | 26 | 0 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 0 | 7 | 309 | 0 | 0 | 0 | 546 | 4 | 0 | 4 | 0 | 3 | 0 | 0 | 0 | 0 |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2030 No-Build Pedestrians | | | 0 | | | | 0 | | | | 0 | | | 1 | 0 | |
| 2030 No-Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| Trip Distribution IN | 1 | 1 | 1 | 250/ | r | 159/ | 1 | | r | | 1 | | r | 1 | | 1 |
| Trip Distribution OUT | | | | 2370 | | 1370 | | | | | | | | (259/) | | (1E0/) |
| Palanaina Adjustment | | | | | | | | | | | | | | (23%) | | (15%) |
| Dalancing Augustment | 0 | 0 | 0 | 64 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 25 | 0 | 21 |
| Residential mps | 0 | 0 | 0 | 34 | 0 | 32 | 0 | U | 0 | 0 | 0 | 0 | 0 | 33 | 0 | 21 |
| Trip Distribution IN | 1 | 1 | | 40% | I | 20% | 1 | | I | | 1 | | | 1 | | 1 |
| Trip Distribution OUT | | | | 4070 | | 2070 | | | | | | | | (40%) | | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | () | | (2010) |
| Retail Trips | 0 | 0 | 0 | 4 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 1 |
| • | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | 40% | | 20% | | | | | | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (40%) | | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Primary Site Trins | 0 | 0 | 0 | 58 | 0 | 34 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 38 | 0 | 22 |
| ······ | | | | | | | - | | | | _ | - | | | | |
| Pass-By Trips | 0 | 0 | -3 | 3 | 0 | 3 | -3 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 |
| Total Vakinular Draiget Trins | | 0 | 2 | 61 | 0 | 27 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 41 | 0 | 25 |
| rotar venicular Project Trips | I | U | -3 | 01 | U | 57 | -3 | U | U | U | U | U | U | 41 | U | 20 |
| 2030 Build Traffic | 0 | 7 | 306 | 61 | 0 | 37 | 543 | 4 | 0 | 4 | 0 | 3 | 0 | 41 | 0 | 25 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 4% | 2% | 2% | 2% | 2% | 2% | 2% | 27% | 2% | 2% | 2% | 2% | 2% | 2% |
| 2030 Build Pedestrians | 270 | 270 | 0 | 270 | 270 | 270 | 0 | 270 | 270 | | 0 | | 270 | 270 | 0 | 270 |
| 2030 Build Conflicting Pedestrians | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 | | 0 |
| | | | | | | | | | | | | | | | | |

INTERSECTION VOLUME DEVELOPMENT INTERSECTION #9 North Avenue at Site Driveway A

AM PEAK HOUR

| | | | | | | , E | | | | | | | | | | |
|--------------------------------|--------|----------|---------|--------|--------|-------|---------|-------|--------|-------|---------|-------|--------|-------|---------|-------|
| | | Site Dri | veway A | | | | | | | North | Avenue | | | North | Avenue | |
| | | North | bound | | | South | bound | | | Eastb | ound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 38 | 0 | 0 | 0 | 40 | 0 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | | 1 | | | | | | 1 | | | | | 1 | |
| Conflicting Pedestrians | | | | | | | | | | | | | | | | |
| Bicycles | | | | | | | | | | | | | | | | |
| Conflicting Bicycles | | | | | | | | | | | | | | | | |
| Heavy Vehicles | | | | | | | | | | | 1 | | | | 16 | |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 40% | 2% |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 38 | 0 | 0 | 0 | 40 | 0 |
| Aujusteu 2024 Volumes | 0 | 0 | U | U | U | 0 | U | 0 | 0 | U | 50 | U | 0 | U | 40 | 0 |
| Appual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.00 | 1.0% | 1.09 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.09 | 1.00 | 1.00 | 1.00 | 1.00 | 1.0% | 1.00 | 1.00 |
| Background Growth Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1.07 | 0 | 0 | 0 | 1.07 | 0 |
| | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | - | 0 |
| 2020 No Build Traffic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 | 0 | 44 | 0 |
| 2020 No Build Hoavy Vobiclo % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 92 | 2% | 2% | 2% | 44 | 2% |
| 2030 No-Balla Heavy Venicle // | 2 /0 | 2 /0 | 2 /0 | 270 | 2 /0 | 270 | 2 /0 | 270 | 2 /0 | 2 /0 | 370 | 2 /0 | 270 | 270 | 4070 | 2 /0 |
| Trip Distribution IN | | | | | | | | | | | | 5% | | 10% | 10% | |
| Trip Distribution OUT | | (5%) | | (10%) | | | | | | | (10%) | 0.0 | | 1070 | 1070 | |
| Balancing Adjustment | | (370) | | (1070) | | | | | | | (10/0) | | | | | |
| Residential Trins | 0 | 12 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 24 | 4 | 0 | 7 | 8 | 0 |
| Residential mps | U | 12 | Ū | 27 | U | U | U | Ū | Ū | U | 27 | 7 | U | , | 0 | U |
| Trip Distribution IN | | | | | | | | | I | | | | | | 10% | |
| Trip Distribution OUT | | | | | | | | | | | (10%) | | | | 10/0 | |
| Balancing Adjustment | | | | | | | | | | | (10/0) | | | | | |
| Potail Trins | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| itetan mps | 0 | 0 | 0 | U | 0 | U | U | Ū | 0 | U | 0 | 0 | 0 | Ū | 0 | U |
| Trip Distribution IN | | | | | | | | | | | | | | | 10% | |
| Trip Distribution OUT | | | | | | | | | | | (10%) | | | | 1070 | |
| Balancing Adjustment | | | | | | | | | | | () | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | - | | | | | - | | - | | - | - | - | | - | - | - |
| Total Primary Site Trips | 0 | 12 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 24 | 4 | 0 | 7 | 8 | 0 |
| | - | | | | - | - | - | - | | - | | | Ţ | | | - |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Total Vehicular Project Trips | 0 | 12 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 24 | 4 | 0 | 7 | 8 | 0 |
| | | | | | • | | | | • | • | | • | • | • | | |
| 2030 Build Traffic | 0 | 12 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 66 | 4 | 0 | 7 | 52 | 0 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 34% | 2% |

PM PEAK HOUR

| | | Site Dr | iveway A | | | | | | | North | Avenue | | | North | Avenue | |
|-------------------------------|--------|---------|----------|-------|--------|-------|---------|-------|--------|-------|---------|-------|--------|-------|---------|-------|
| | | North | nbound | | | South | bound | | | East | oound | | | West | bound | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | | | | | | | | | | | 44 | | | | 283 | |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | | | | | | | | | | | | | | |
| Conflicting Pedestrians | | | | | | | | | | | | | | | | |
| Bicycles | | | | | | | | | | | | | | | | |
| Conflicting Bicycles | | | | | | | | | | | | | | | | |
| Heavy Vehicles | | | | | | | | | | | 2 | | | | 7 | |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 5% | 2% | 2% | 2% | 2% | 2% |
| Peak Hour Factor | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 44 | 0 | 0 | 0 | 283 | 0 |
| | • | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 26 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 48 | 0 | 0 | 0 | 309 | 0 |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | • | | | |
| Trip Distribution IN | | | | | | | | | | | | 5% | | 10% | 10% | |
| Trip Distribution OUT | | (5%) | | (10%) | | | | | | | (10%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 7 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 11 | 0 | 21 | 21 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | 10% | |
| Trip Distribution OUT | | | | | | | | | | | (10%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| | - | | - | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | (100) | | | | 10% | |
| Trip Distribution OUT | - | | | | | | | | | | (10%) | | | | | |
| Balancing Adjustment | 0 | 0 | <u> </u> | | 0 | • | 0 | • | 0 | 0 | 0 | 0 | 0 | 0 | | 0 |
| Restaurant Trips | 0 | U | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | U | 0 | 0 | 0 | 0 |
| Tatal Drimon, Cita Trina | 0 | 7 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 11 | 0 | 21 | 22 | 0 |
| Total Primary site Trips | U | / | U | 14 | U | U | U | U | U | U | 15 | 11 | U | 21 | 22 | U |
| Pass By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| rass-by mps | 0 | U | 0 | 0 | 0 | 0 | 0 | U | U | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 1 | 7 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 11 | 0 | 21 | 22 | 0 |
| rotal volitorial region mps | 1 | ı , | | .4 | | | | 5 | | | | | 0 | | 1 | |
| 2030 Build Traffic | 0 | 7 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 63 | 11 | 0 | 21 | 331 | 0 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 2% | 2% |

INTERSECTION VOLUME DEVELOPMENT INTERSECTION #10 North Avenue at Site Driveway B

AM PEAK HOUR

| | | Site Dri | iveway B | | | | | | | North | Avenue | | | North | Avenue | |
|-------------------------------|--------|----------|----------|-------|--------|-------|---------|-------|--------|-------|---------|-------|-----------|-------|---------|-------|
| | | North | bound | | | South | bound | | | Fasth | ound | | Westbound | | | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 38 | 0 | 0 | 0 | 40 | 0 |
| Count Balancing | | | | | | | | | | | | | | | | ĺ |
| Pedestrians | | | | | | | | | | | | | | | | - |
| Conflicting Pedestrians | | | | | | | | | | | | | | | 1 | í l |
| Bicycles | | | | | | | | | | | | | | | | í l |
| Conflicting Bicycles | | | | | | 1 | | | | | | | | 1 | | |
| Heavy Vehicles | | | | | | | | | | | 1 | | | | 16 | |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 40% | 2% |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 38 | 0 | 0 | 0 | 40 | 0 |
| | | | | | | | | | | | | | | | | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 42 | 0 | 0 | 0 | 44 | 0 |
| 2030 No-Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 40% | 2% |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | 5% | 10% | | 10% | | |
| Trip Distribution OUT | | (10%) | | (10%) | | | | | | | | | | | (5%) | |
| Balancing Adjustment | | | | | | | | | | | | | | | | ĺ |
| Residential Trips | 0 | 24 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 8 | 0 | 8 | 12 | 0 |
| | - | | | | - | | | | - | | | | - | | | |
| Trip Distribution IN | | | | | | | | | | | | 15% | | 10% | | |
| Trip Distribution OUT | | (15%) | | (10%) | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | 15% | | 10% | | |
| Trip Distribution OUT | | (15%) | | (10%) | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | | | | | | | | | | | |
| Total Primary Site Trips | 0 | 24 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 9 | 0 | 8 | 12 | 0 |
| | | | | | | | | | | | | | | | 1 | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vobicular Project Trips | 0 | 24 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 12 | 0 |
| rotar venitulai riojett mps | U | 24 | U | 24 | U | U | U | U | U | U | 4 | 7 | U | 0 | 12 | |
| 2030 Build Traffic | 0 | 24 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 0 | 46 | 9 | 0 | 8 | 56 | 0 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 31% | 2% |
| | 270 | 270 | 270 | 270 | 270 | 270 | 270 | 270 | 270 | 270 | 270 | 270 | 270 | 270 | 0170 | 270 |

PM PEAK HOUR

| | | | | | | 1 1 1 1 1 1 2 | 10001 | • | | | | | | | | |
|-------------------------------|--------|----------|----------|--------|---------------------------------------|---------------|---------|-------|--------|-------|---------|-------|-----------|-------|---------|-------|
| | | Site Dri | iveway B | | | | | | | North | Avenue | | | North | Avenue | |
| | | North | hound | | | South | bound | | | Fastl | ound | | Westbound | | | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | | | | g | | | | | | | 44 | | | | 283 | g. |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | | 1 | | | | | | | | | | | | |
| Conflicting Pedestrians | | | | | | | | | | | | | | | | |
| Biovelos | | | | | | | | | | | | | | | | |
| Conflicting Biovelos | | | | | | | | | | | | | | | | |
| Homa Vobiolog | | | | | | 1 | | | | 1 | 2 | | | 1 | 7 | |
| Heavy Vehicles | 20/ | 20/ | 20/ | 20/ | 20/ | 20/ | 20/ | 20/ | 20/ | 20/ | E0/ | 20/ | 20/ | 20/ | 20/ | 20/ |
| Deels Lieur Feeter | 2 /0 | 270 | 2 /0 | 270 | 2 /0 | 2 /0 | 2 /0 | 270 | 2 /0 | 2 /0 | 0.02 | 270 | 2 /0 | 270 | 2 /0 | 2 /0 |
| Peak Hour Factor | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 | 0.83 |
| Adjustment Factor | | 1 | 1 | 0 | 1 | 1 | 1 | 1 | | 1 | | 1 | 1 | 1 | 000 | 1 |
| Adjusted 2024 Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 44 | 0 | 0 | 0 | 283 | 0 |
| Annual Quanth Data | 1.50/ | 4.50/ | 1.50/ | 4.50/ | 1.50/ | 4.50/ | 1.50/ | 1.50/ | 4 500 | 1.50/ | 1.50/ | 1.50/ | 4.50/ | 1 50/ | 4.50/ | 1.50/ |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 26 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | 10 | | | | | |
| 2030 No-Build Traffic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 48 | 0 | 0 | 0 | 309 | 0 |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | 1 | | | | | | | | 1 | | E0/ | 10% | | 10% | | |
| Trip Distribution IN | | (100() | | (100/) | | | | | | | 376 | 10% | | 10% | (50/) | |
| The Distribution OUT | | (10%) | | (10%) | | | | | | | | | | | (5%) | |
| Balancing Adjustment | 0 | | 0 | | 0 | 0 | 0 | 0 | 0 | 0 | 44 | 00 | 0 | 04 | 7 | 0 |
| Residential Irips | 0 | 14 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | | 20 | 0 | 21 | 1 | 0 |
| Trip Distribution IN | 1 | | | | | | | | 1 | | | 150/ | | 100/ | | |
| Trip Distribution IN | | (1E0/) | | (10%) | | | | | | | | 15% | | 10% | | |
| Palapsing Adjustment | | (1376) | | (10%) | | | | | | | | | | | | |
| Datail Trips | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 1 | 0 | 0 |
| Retail mps | 0 | | U | | 0 | U | U | U | U | U | U | Z | 0 | | U | U |
| Trip Distribution IN | 1 | | | | | | | | 1 | | | 15% | | 10% | | |
| Trip Distribution OUT | | (15%) | | (10%) | | | | | | | | 1370 | | 1070 | | |
| Balancing Adjustment | | (1070) | | (1070) | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| nostadram mps | | Ů | Ű | | , , , , , , , , , , , , , , , , , , , | | | Ū | ů | | | 0 | Ů | Ű | | |
| Total Primary Site Trips | 0 | 15 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 22 | 0 | 22 | 7 | 0 |
| | - | | | | - | - | - | - | - | - | | | - | | | - |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | 1 | 1 | | | | 1 | 1 | | | 1 | | | | 1 | |
| Total Vehicular Project Trips | | 15 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 22 | 0 | 22 | 7 | 0 |
| · · · | | | | | | | | | | | | | | | | |
| 2030 Build Traffic | 0 | 15 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 59 | 22 | 0 | 22 | 316 | 0 |
| 2030 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 4% | 2% | 2% | 2% | 2% | 2% |

INTERSECTION VOLUME DEVELOPMENT INTERSECTION #11 Mayson Turner Road at Site Driveway D

| | | | | | | AM PE | ak houf | 2 | | | | | | | | |
|-------------------------------|--------|-------|---------|-------|------------|-----------|---------|-----------|----------|----------|------------|-----------|--------|----------|------------|-------|
| | | | | | | Site Driv | veway D | | | Mayson T | urner Road | | | Mayson T | urner Road | |
| | | North | nbound | | Southbound | | | Eastbound | | | | Westbound | | | | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2024 Traffic Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 84 | 0 | 0 | 0 | 58 | 0 |
| Count Balancing | | | | | | | | | | | | | | | | |
| Pedestrians | | | | | | | | | | | | | | | | |
| Conflicting Pedestrians | | | | | | | | | | | | | | | | |
| Bicycles | | | | | | | | | | | | | | | | |
| Conflicting Bicycles | | | | | | | | | | | | | | | | |
| Heavy Vehicles | | | | | | | | | | | 3 | | | | 3 | |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 4% | 2% | 2% | 2% | 5% | 2% |
| Peak Hour Factor | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjusted 2024 Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 84 | 0 | 0 | 0 | 58 | 0 |
| | | | | | | | | | | | | | | | .1 | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 |
| Background Growth Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 5 | 0 |
| 1060 DLH DRI #4187 | | | | | | | | | | | | | | | | |
| 2030 No-Build Traffic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 92 | 0 | 0 | 0 | 63 | 0 |
| 2030 No-Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 4% | 2% | 2% | 2% | 5% | 2% |
| | | | | | | | | | | | | | - | | | |
| Trip Distribution IN | | | | | | | | | | 5% | | | | | | 20% |
| Trip Distribution OUT | | | | | | (20%) | | (5%) | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 47 | 0 | 12 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 15 |
| | | | | | | | | | - | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | 10% | 15% |
| Trip Distribution OUT | | | | | | (15%) | | | | | (10%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | 10% | 15% |
| Trip Distribution OUT | | | | | | (15%) | | | | | (10%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | | | | | | 1 | | | | 1 | 1 | | | 1 | | |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 47 | 0 | 12 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 16 |
| | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Tabliful and David Antoin | 0 | 0 | 0 | 0 | 0 | 47 | 0 | 40 | <u>^</u> | | 0 | 0 | | | | 11 |
| Total venicular Project Trips | 0 | 0 | 0 | 0 | 0 | 47 | 0 | 12 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 16 |
| 2020 Duild Troffin | 0 | 0 | 0 | 0 | 0 | 47 | 0 | 10 | 0 | 4 | 00 | 0 | 0 | 0 | (2 | 1/ |
| 2030 Build Hoper Vehicle % | 0 | 20/ | 0 | 0 | 0 | 4/ | 0 | 12 | 0 | 4 | 92 | 0 | 0 | 0 | 63 E9/ | 10 |
| 2030 build Heavy vehicle % | 270 | 270 | 270 | 270 | 270 | 2% | 270 | 2% | 270 | 2% | 470 | 270 | 2% | 2% | 3% | 270 |

PM PFAK HOUR

| | | | | | | TIVITE | | ` | | | | | | | | | |
|-------------------------------|--------|-------|---------|-------|-----------------|--------|---------|----------|--------------------|------|---------|-------|--------------------|----------|----------|-------|--|
| | | | 0 | | Site Driveway D | | | | Mayson Turner Road | | | | Mayson Turner Road | | | | |
| | | North | hbound | | | South | bound | | | East | ound | | Westbound | | | | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | |
| Observed 2024 Traffic Volumes | | | | | | | | | | | 136 | | | | 110 | | |
| Count Balancing | | | | | | | | | | | | | | | | | |
| Pedestrians | | | 1 | | | | | 1 | | | | 1 | | | | | |
| Conflicting Pedestrians | | | | | | | | | | | 1 | | | 1 | | 1 | |
| Bicycles | | | | | | | | | | | | | | | | | |
| Conflicting Bicycles | | 1 | | | | 1 | | | | 1 | | | | | | | |
| Heavy Vehicles | | | | | | | | | | | 4 | | | | 5 | | |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 5% | 2% | |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| Adjusted 2024 Volumes | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 136 | 0 | 0 | 0 | 110 | 0 | |
| Aujusteu 2024 Volumes | 0 | Ū | 0 | U | Ū | Ū | Ū | Ū | Ū | Ū | 150 | Ū | Ū | U | 110 | Ū | |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | |
| Growth Eactor | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | 1.09 | |
| Background Growth Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 10 | 0 | |
| 1060 DI H DRI #4187 | _ | - | | - | - | - | - | - | - | - | | - | - | - | | | |
| 2030 No-Build Traffic | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 149 | 0 | 0 | 0 | 120 | 0 | |
| 2030 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| | - | - | 1 - | | | - | | - | | - | - | - | | - | | - | |
| Trip Distribution IN | | | | | | | | | | 5% | | | | | | 20% | |
| Trip Distribution OUT | | | | | | (20%) | | (5%) | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 27 | 0 | 7 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 43 | |
| · | | | | | | | | | | | | | | | | | |
| Trip Distribution IN | | | | | | | | | | | | | | | 10% | 15% | |
| Trip Distribution OUT | | | | | | (15%) | | | | | (10%) | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 2 | |
| | | | | | | | | | | | | | | r | | r | |
| Trip Distribution IN | | | | | | | | | | | | | | | 10% | 15% | |
| Trip Distribution OUT | | | | | | (15%) | | | | | (10%) | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| | 1 . | | | | | | | _ | | | | | | | | | |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 28 | 0 | 7 | 0 | 11 | 1 | 0 | 0 | 0 | 1 | 45 | |
| Daga Du Tring | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | U | 0 | 0 | |
| Total Vobicular Project Trips | 1 | 0 | 0 | 0 | 0 | 20 | 0 | 7 | 0 | 11 | 1 | 0 | 0 | 0 | 1 | 45 | |
| rotar venicular rioject mps | | U | 1 0 | | 0 | 20 | 0 | . / | 0 | L 11 | L ' | U | U | U | <u> </u> | 40 | |
| 2030 Build Traffic | 0 | 0 | 0 | 0 | 0 | 28 | 0 | 7 | 0 | 11 | 150 | 0 | 0 | 0 | 121 | 45 | |
| 2030 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 2% | 5% | 2% | |

Programmed and Planned Projects Fact Sheets



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SR 8/US 278 FROM SR 280 TO CS 6701/STIFF STREET

| Project ID: | 0017926 | Notice to Proceed Date: | 7/3/2023 |
|--------------------------------|--------------------|-----------------------------------|-----------------------|
| Project Manager: | Nakeeta Batson | Construction Percent Complete: | 59.36% |
| Office: | Program Delivery | Current Completion Date: | 3/31/2024 |
| County: | Fulton | Work Completion Date: | |
| Congressional District: | 005 | Construction Contract Amount: | |
| State Senate District.: | 006, 038 | Construction Contractor: | SMART ROAD TECHNOLOGY |
| State House District: | 055, 060 | Preconstruction Status R | leport |
| Project Type: | Safety | Construction Status Rep | ort |
| Project Status: | Under Construction | | |
| Right of Way Authorization: | | Contact Us | |

Project Description:

The project proposes to road diet DL Hollowell from 4 to 3 lanes by re-striping and relocate signal heads within project limits.

| Activity | Program Year | Cost Estimate | Date of Last Estimate |
|------------------------------|--------------|----------------|-----------------------|
| PE (Preliminary Engineering) | 2022 | \$680,908.08 | |
| CST (Construction) | 2023 | \$3,303,553.05 | |



Project Documents

There are no items to show in this view.



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| Activity | Progra | ım Year | Cost Estima | te | Date of Last Esti |
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| Grove Park | Evelyn Mar Nw Blanche St Nw Wadrona St NW | Flore | COLUMN TO CALL | Elinor Place Park 00103222 | Blyss Ave NW |
| arktw | 10 | 000 | STY PI NW | 9 | |
| Marktw | He Hon | ns Dr NW Ha | sty pi NW | Georgia D | epartment of Transportatio |

0010322_RevCR_JAN2024.pdf

Project Outreach Archive

DL Hollowell Parkway - Fact Sheet I.pdf





Home Projects Sidewalks DL Hollowell Sidewalks

DL Hollowell Sidewalks



PROJECT NUMBER **3060**

Scope

Includes sidewalk construction on DL Hollowell Pkwy NW from Proctor Creek to W Lake Ave NW and represents a scope reduction from ATLDOT's existing project No. 3003.

| PAID | \$0 |
|---------------|----------|
| | May 2022 |
| PROJECT START | May 2 |

Disclaimer: Project schedules and scopes are subject to change.

PHASE



Additional Project Information

ATLANTA DEPARTMENT OF TRANSPORTATION (ATLDOT)

Atlanta Department of Transportation (ATLDOT) Atlanta City Hall 55 Trinity Avenue SW, Suite 4400 Atlanta, GA 30303

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SR 8 FROM PROCTOR CREEK GREENWAY TO ATLANTA BELTLINE - VRU

| Project ID: | 0020200 | Notice to Proceed Date: |
|--------------------------|---------------------------|-----------------------------------|
| Project Manager: | Stenley K. Mack | Construction Percent % |
| Office: | Traffic Operations | Current Completion Date: |
| County: | Fulton | Work Completion Date: |
| Congressional District: | 005 | Construction Contract Amount: |
| State Senate District .: | 006, 039 | Construction Contractor: |
| State House District: | 055, 056 | Preconstruction Status Report |
| Project Type: | Safety | Construction Status Report |
| Project Status: | Construction Work Program | |
| Right of Way | | Contact Us |
| Authorization: | | |

Project Description:

| Activity | Program Year | Cost Estimate | Date of Last Estimate |
|------------------------------|--------------|----------------|-----------------------|
| UTL (Utilities) | | \$25,000.00 | |
| PE (Preliminary Engineering) | | \$20,000.00 | |
| CST (Construction) | | \$1,575,000.00 | |



Project Documents

There are no items to show in this view.



Georgia Department of Transportation

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Rider Alert:

Mon 4/29 - Fri 5/3: All rail lines will operate on 12-20 minute special single tracking schedules.

MARTA Airport Station closed for renovation April 8 - May 19.

Shuttle provided between College Park Station and North Terminal Lower Level. View rail schedules and additional details here.



Projects

Bankhead Station Platform Extension

Current Status: Design. Scheduled for completion in 2027.

Overview

This a More MARTA Atlanta project in partnership with the City of Atlanta to renovate the existing Bankhead Station and extend the platform to accommodate eight cars instead of the current two cars (matching the capacity of the other 37 rail stations). This location is also being considered for future Transit-Oriented Development (TOD). To learn more about TOD master planning for this location, visit <u>bankheadmasterplan.com</u>.



Project Renderings

Image description: This illustration shows what MARTA's Bankhead Station will look like after it is renovated and the platform is extended. The existing station is shown in blue and the new extended platform, canopy and station entrance is shown in orange.



Image description: This illustration shows the proposed new entrance plaza on the west side of MARTA's Bankhead Station. The new extended platform and glass walls are in the background. In the foreground, people are entering and exiting the station. On the right, people are sitting on a new bench.



Image description: This illustration shows the platform level of the proposed expansion of MARTA's Bankhead Station. A MARTA train extends along the left side. People are standing on the platform waiting to board the train. An escalator descends in the middle of the platform.

Project Features

- Extension of the existing platform to accommodate eight rail cars rather than the current two rail cars
- Public pedestrian plaza with enhanced entrances and exits for customers, pedestrians and motorists
- New canopy on the platform
- New elevator and escalator
- Designed for integration with future Transit-Oriented Development

Resources

Public Meetings



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CS 716/CS 3498/MARIETTA BLVD FROM CS 13/CORONET WAY TO SR 8 Notice to Decend Date

| Project ID: | 0017803 | Notice to Proceed Date: | |
|--------------------------|---------------------------|-------------------------------------|--|
| Project Manager: | Elliott Vernon Bryson III | Construction Percent % Complete: | |
| Office: | Program Delivery | Current Completion Date: | |
| County: | Fulton | Work Completion Date: | |
| Congressional District: | 005 | Construction Contract Amount: | |
| State Senate District .: | 038 | Construction Contractor: | |
| State House District: | 053, 055 | Preconstruction Status Report | |
| Project Type: | Other | Construction Status Report | |
| Project Status: | Construction Work Program | | |
| Right of Way | | Contact Us | |
| Authorization: | | | |

Project Description:

This project will reconstruct and resurface the corridor between Coronet Way NW and DL Hollowell Parkway NW. The Pavement Condition Index (PCI) score varies from 37 to 58. The corridor is on the Regional Freight Route and is a major North-South connection for the City of Atlanta.

| Activity | Program Year | Cost Estimate | Date of Last Estimate |
|---------------|--------------|---------------|-----------------------|
| SCP (Scoping) | 2021 | \$6,250.00 | |


Project Documents

There are no items to show in this view.



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Connectivity

The 425 Chappell Road site is uniquely situated to nearby neighborhood destinations. Within walking distance, the site has proximity to a MARTA rail station, Maddox Park, the Proctor Creek Greenway, and the future Atlanta BeltLine Westside Trail. Further out, both Grove Park and the Westside Reservoir Park lie to the northwest. to North Avenue and the proposed street connections through the Overall, the site has excellent access to transit and greenspace – an site. ideal site for added housing opportunities.

Despite the proximity, accessing these transit and trail assets in the site's current condition is challenging. The following framework of existing, planned, and proposed connectivity projects aims to improve access and connectivity.

Existing Connectivity Framework

Connectivity is important to the overall plan for this site as well as to the community. Safe bike and pedestrian accessibility was a chief community concern; thus, the first step to the development plan involved creating a connectivity framework. This framework maps existing and planned connections in and around the site, ensuring they all work together. Multiple entities – including the City • Valley of the Hawks Constructed Wetlands: constructed of Atlanta Watershed Department, MARTA, the Georgia Department of Transportation (GDOT), the Atlanta Department of Transportation (ATLDOT), and Friends of Maddox Park – have planned bike and pedestrian projects that impact the area's connectivity. Along with ABI's proposed connectivity improvements, the connectivity map on the facing page illustrates how these multiple projects will act cohesively to achieve the community's desire to have a more walkable and bikeable community.

Atlanta BeltLine Westside Trail and Spur Trail

Running north-to-south, the Atlanta BeltLine Westside Trail – also called "mainline"— follows the former rail line east of the site, directly adjacent to the City of Atlanta Public Works property. This part of the trail, specifically Westside Trail Segment 4, kicked off construction in March of 2023 and is expected to open in summer of 2025.

The development framework for 425 Chappell Road proposes an TOD development. These critical crossings will in turn, provide much Atlanta BeltLine spur trail to better connect the site and the broader neighborhood to the mainline trail. The route facilitates access from Bankhead Station, Maddox Park, and ultimately, Atlanta BeltLine the BeltLine to 425 Chappell Road but also, Maddox Park and its mainline trail. proposed trails connecting to the Bankhead MARTA station and the mixed use development opportunity on the existing City of Atlanta Public Works site. This spur trail appears as a sidepath running parallel

needed bike and pedestrian access between the future TOD at

Friends of Maddox Park

The civic group Friends of Maddox Park worked with Park Pride, a nonprofit that advocates for parks and park improvements, to develop proposed park improvements to Maddox Park. One key project includes a lap trail.

The connectivity plan integrates this lap trail within the broader trail network As shown, the lap trail connects to the BeltLine spur trail and provides direct access to MARTA via the new park entrance on Donald Lee Hollowell Parkway.

The City of Atlanta Watershed department has a number of

City of Atlanta Watershed Department Projects

forthcoming projects designed to restore Proctor Creek and its tributaries. Each of them include trail plans that the connectivity plan integrates into the broader trail network. All of these projects have finalized designs and await implementation or are being constructed. These projects include:

- Proctor Creek Wetlands at Hollowell: constructed wetlands featuring a pedestrian trails and boardwalks will collect and clean stormwater before it enters Proctor Creek. Property acquisition is underway.
- Mosquito Hole: restoration and bank stabilization of the Mosquito Hole Tributary and confluence with Proctor Creek.
- wetlands to capture and filter stormwater before releasing it into Proctor Creek, preventing flooding and improving water quality. The first phase of construction is complete.
- Proctor Creek Restoration at Hunter Hills: restoration of a portion of Proctor Creek that was routed into a concrete channel in the 1960's. The restored stream and reconnected floodplain will manage stormwater, improve water quality, and restore natural habitats. Construction is anticipated to begin in the summer of 2024.

MARTA, GDOT, and ATLDOT

GDOT and ATLDOT are currently implementing a road diet along Donald Lee Hollowell Parkway that will add a bike lane on the north side of the street. The road improvements also feature bike and pedestrian crossings to the Bankhead MARTA station and its planned

Creating a True Gateway

Another critical connectivity need was to create a true "front door," or gateway, from the main road that would draw people into the development. This development plan recommends creating a front door to the site via Maddox Park Drive. This connection point will allow for direct access to the entire site via Donald Lee Hollowell Parkway and will provide access to the Bankhead MARTA station. Because the topography at this connection point involves extreme grade changes of more than 30 feet, the focus of any development at this front door will be internal.

To accomplish this, ABI must enter into agreements with the City of Atlanta Parks and Recreation Department and Public Works Department to swap portions of Maddox Park and a public works site. The next chapter discusses this proposed land swap.

NEIGHBORHOOD CONNECTIVITY

LEGEND





| AR-491B | ARC MTP DRAFT PROJECT FACT SHEET | | | | | | | |
|---|---|--|--|--|--|--|--|--|
| Short Title | NORTH AVENUE CORRIDOR BUS RAPID TRANSIT FROM MARTA NORTH AVENUE RAIL STATION TO MARTA BANKHEAD RAIL STATION | TO MAN W Manietta Street | | | | | | |
| GDOT Project No. | N/A | Participant in the second seco | | | | | | |
| Federal ID No. | N/A | | | | | | | |
| Status | Long Range | de Joseph E Boone Blvd NW | | | | | | |
| Service Type | Transit / BRT Capital | the state of the s | | | | | | |
| Sponsor | MARTA | Martin Luther King Jr. Dr. NW | | | | | | |
| Jurisdiction | City of Atlanta | 0 0.5 1 Miles | | | | | | |
| Analysis Level | In the Region's Air Quality Conformity Analysis | Anna ava Str | | | | | | |
| Existing Thru Lane | N/A LCI | Network Year 2050 | | | | | | |
| Planned Thru Lane | N/A Flex | Corridor Length TBD miles | | | | | | |
| Detailed Description | Detailed Description and Justification | | | | | | | |
| This project will provide hig heavy rail stations. | yh capacity premium transit service along the North Avenue co | orridor between MARTA's North Avenue and Bankhead | | | | | | |

| Phase Status & Funding St | | Status | FISCAL | TOTAL PHASE | BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE | | | |
|---------------------------|--|--------|------------------|--------------|---|---------|---------|---------------|
| Information | | | YEAR | COST | FEDERAL | STATE | BONDS | LOCAL/PRIVATE |
| ALL | Local Jurisdiction/Municipality Funds | | LR 2041- 2050 | \$62,900,000 | \$0,000 | \$0,000 | \$0,000 | \$62,900,000 |
| | | | | \$62,900,000 | \$0,000 | \$0,000 | \$0,000 | \$62,900,000 |

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases

? For additional information about this project, please call (404) 463-3100 or email transportation@atlantaregional.com.

| AR-490F | ARC MTP DRAFT PROJECT FA | CT SHEET |
|---|---|--|
| Short Title | ATLANTA STREETCAR - NORTHWEST BELTLINE CORRIDOR FROM NEAR INTERSECTION OF WESTVIEW DRIVE AT LANGHORN STREET TO MARTA BANKHEAD RAIL STATION | Z78 MN e AV ey P Coseph C Boone Blyd NW |
| GDOT Project No. | N/A | bell h |
| Federal ID No. | N/A | |
| Status | Long Range | Martin Wher King Jr Dr NW |
| Service Type | Transit / Rail Capital | |
| Sponsor | MARTA | 2 402 Westview D Sh |
| Jurisdiction | Regional - Central | 0 0.5 1 Miles |
| Analysis Level | In the Region's Air Quality Conformity Analysis | CUCHE AVE SW |
| Existing Thru Lane | N/A LCI | Network Year 2050 |
| Planned Thru Lane | N/A Flex | Corridor Length TBD miles |
| Detailed Description | and Justification | |
| This project constructs a ne MARTA Bankhead heavy ra | ew streetcar line along the Beltline corridor between the inter il station | section of Westview Drive and Langhorn Street to the |

| Phase Status & Funding Status | | FISCAL | TOTAL PHASE | BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE | | | | |
|-------------------------------|------------|--------|------------------|---|--------------|---------|--------------|---------------|
| Information | | | YEAR | COST | FEDERAL | STATE | BONDS | LOCAL/PRIVATE |
| ALL | New Starts | | LR 2041- 2050 | \$96,900,000 | \$48,450,000 | \$0,000 | \$0,000 | \$48,450,000 |
| | | | \$96,900,000 | \$48,450,000 | \$0,000 | \$0,000 | \$48,450,000 | |

 SCP: Scoping
 PE: Preliminary engineering / engineering / design / planning
 PE-OV: GDOT oversight services for engineering
 ROW: Right-of-way Acquistion

 UTL: Utility relocation
 CST: Construction / Implementation
 ALL: Total estimated cost, inclusive of all phases
 ROW: Right-of-way Acquistion





Image: TSW

Land Use and Zoning Maps

