

Transportation Analysis - DRAFT

Aventon Park DRI #3734

Gwinnett County, Georgia

August 2022

Prepared for:

Aventon Companies

Prepared by:

Kimley-Horn and Associates, Inc.
817 West Peachtree Street NW, Suite 601
Atlanta, GA 30308
013543004

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Available Upon Request

Raw Traffic Count Data
Synchro Capacity Analyses

EXECUTIVE SUMMARY

This report presents the analysis of the anticipated traffic impacts of the proposed *Aventon Park* development located in Gwinnett County, Georgia. The approximate 57.76-acre site is located along the north side of Athens Highway (SR 10/US 78). The site is currently undeveloped.

The proposed development will consist of the following land uses and densities contained in **Table 1**. The project is expected to be completed by 2026 (approximately 4 years).

| Table 1: Proposed Land Use and Density | |
|--|--------------------|
| Multifamily Residential | 662 dwelling units |
| Office | 7,100 SF |
| Retail | 9,600 SF |
| Restaurant | 5,400 SF |

The DRI analysis includes an estimation of the overall vehicle trips projected to be generated by the development, also known as gross trips. Mixed-use and pass-by reductions to gross trips are included in the trip generation, as outlined in the Georgia Regional Transportation Authority (GRTA) Letter of Understanding (dated March 22, 2022).

Capacity analyses were performed for the study intersections under the Calibrated 2022 conditions, the Projected 2026 No-Build conditions, and the Projected 2026 Build conditions.

- Calibrated 2022 conditions represent current traffic volumes collected in April 2022 that were calibrated to account for COVID-19's impact on traffic.
- Projected 2026 No-Build conditions represent the Calibrated 2022 traffic volumes grown for four (4) years using a 1.5% per year growth rate.
- Projected 2026 Build conditions represent the Projected 2026 No-Build conditions plus the addition of the project trips that are anticipated to be generated by the *Aventon Park* development.

No-Build (System Improvements)

There are no recommended system improvements needed to serve background traffic (without the development) for the study network.

Build (Site Access Improvements)

The following should be considered to serve the Projected 2026 Build Conditions (needed to serve development traffic):

- Athens Highway (SR 10/US 78) at Midway Plaza Driveway/Driveway B (Intersection 3) – Full Movement
 - Construct Driveway B to operate as a full movement, side-street stop-controlled intersection
 - Construct one (1) ingress lane entering the site and one (1) egress lane exiting the site
 - Construct one (1) westbound right-turn lane along Athens Highway (SR 10/ US 78)
- Athens Highway (SR 10/US 78) at Driveway A (Intersection 6)
 - Construct Driveway A to operate as a full movement, side-street stop-controlled intersection
 - Construct one (1) ingress lane entering the site and one (1) egress lane exiting the site
 - Construct one (1) westbound right-turn lane along Athens Highway (SR 10/ US 78)

Athens Highway (SR 10/US 78) at Midway Plaza Driveway/ Driveway B (Intersection 3) LOS Summary

Overall LOS Standard: D
Approach LOS Standard: D

| Overall LOS Standard: D Approach LOS Standard: D | | Midway Plaza Driveway | | | Driveway B | | | Athens Highway (SR 10/US 78) | | | Athens Highway (SR 10/US 78) | | | |
|---|----|-----------------------|----------|----|------------|-----------|-----|------------------------------|---------|---|------------------------------|---------|---|--|
| | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | | |
| | | L | T | R | L | T | R | L | T | R | L | T | R | |
| 2026 Build (TWSC) | AM | Overall LOS | B (14.3) | | | | | | | | | | | |
| | | Approach LOS | F (55.2) | | | F (482.1) | | | A (0.6) | | | A (0.1) | | |
| | | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 40 | | | 230 | | 10 | | | 3 | | |
| | PM | Overall LOS | A (2.5) | | | | | | | | | | | |
| | | Approach LOS | E (46.2) | | | E (47.0) | | | A (0.4) | | | A (0.3) | | |
| | | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 80 | | | 40 | | 10 | | | 5 | | |

Athens Highway (SR 10/US 78) at Driveway A (Intersection 6) LOS Summary

Overall LOS Standard: D
Approach LOS Standard: D

| Overall LOS Standard: D Approach LOS Standard: D | | - | | | Driveway A | | | Athens Highway (SR 10/US 78) | | | Athens Highway (SR 10/US 78) | | | |
|---|----|--------------|---------|---|------------|-----------|---|---------------------------------|-------|---|---------------------------------|-------|--|--|
| | | | | | | | | | | | | | | |
| | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | | |
| L | T | R | L | T | R | L | T | R | L | T | R | | | |
| 2026 BUILD (TWSC) | AM | Overall LOS | A (9.8) | | | | | | | | | | | |
| | | Approach LOS | | | | F (204.9) | | | (1.3) | | | (0.0) | | |
| | | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | | | 233 | | | | | | 23 | | |
| | PM | Overall LOS | A (1.0) | | | | | | | | | | | |
| | | Approach LOS | | | | D (26.7) | | | (0.8) | | | (0.0) | | |
| | | Storage | | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | | | 33 | | | | | | 20 | | |

1.0 PROJECT DESCRIPTION

1.1 Introduction

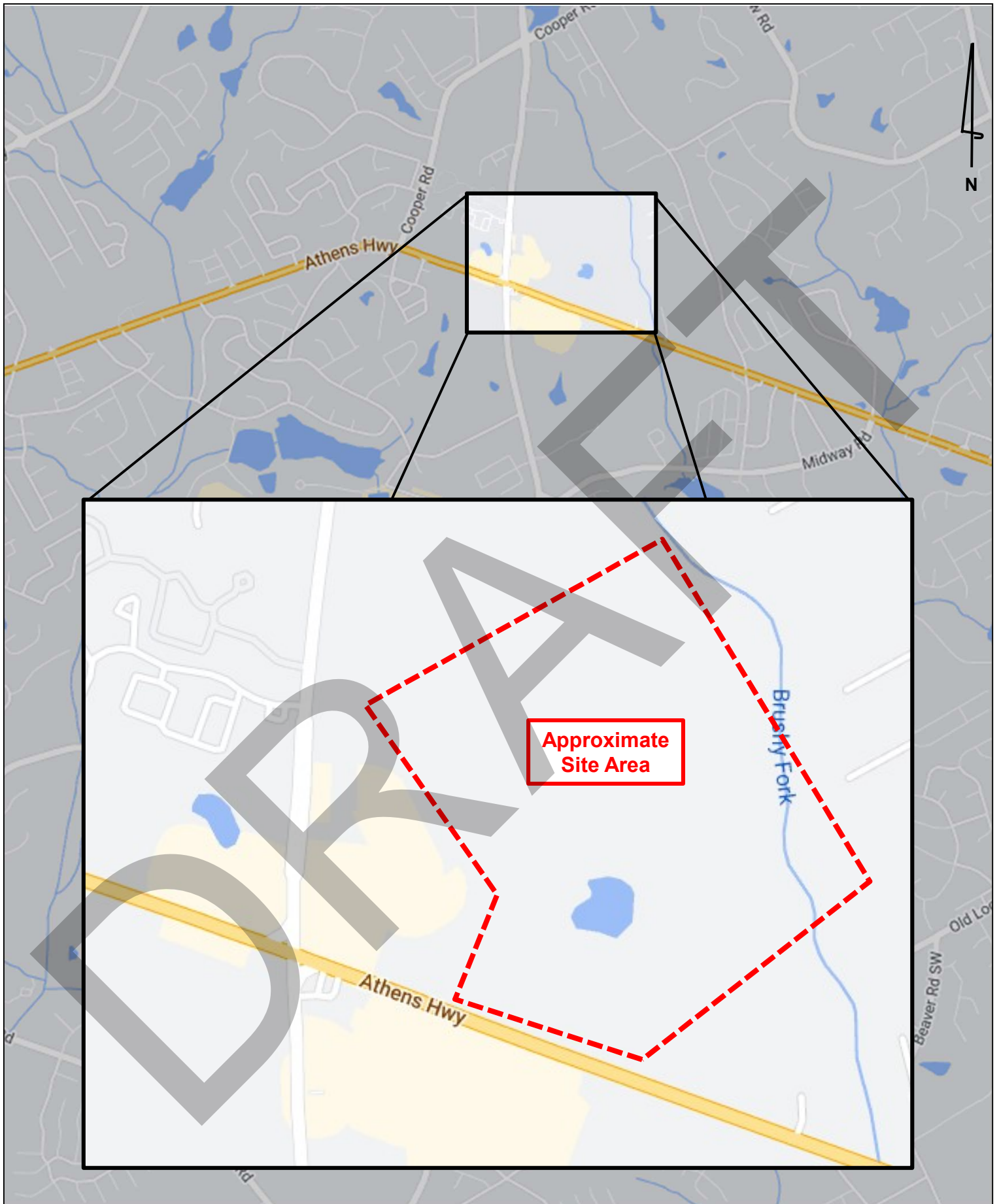
This report presents the analysis of the anticipated traffic impacts of the proposed *Aventon Park* development located in Gwinnett County, Georgia. The approximate 57.76-acre site is located along the north side of Athens Highway (SR 10/US 78). The project site is currently zoned R-100 (Single-Family Residence District). Permit #RZM2022-0035: RZC2022-00029) was filed on May 31, 2022. **Figure 1** provides a location map of the project site. **Figure 2** provides an aerial view of the project site and surrounding area.

The site is currently undeveloped. The proposed development will consist of the following land uses and densities contained in **Table 2**. The project is expected to be completed by 2026 (approximately 4 years).

| Table 2: Proposed Land Use and Density | |
|--|--------------------|
| Land Use | Proposed |
| Multifamily Residential | 662 dwelling units |
| Office | 7,100 SF |
| Retail | 9,600 SF |
| Restaurant | 5,400 SF |

A reference of the proposed site plan is provided in **Appendix A**. A full-sized site plan consistent with GRTA's Site Plan Guidelines is also being submitted as part of the review package.

The project is considered a Development of Regional Impact (DRI) and is subject to Georgia Regional Transportation Authority (GRTA) and Atlanta Regional Commission (ARC) review due to the project size exceeding 500 residential units (housing development) or exceeding 500,000 GSF (mixed-use development in a Maturing Developing Suburb (per UGPM). The DRI was formally triggered with the filing of the Initial DRI Information (Form 1) on June 15, 2022 by the City of Atlanta. This transportation analysis includes all inputs and methodologies discussed at the DRI Methodology Meeting with GRTA, ARC, and other stakeholders. The inputs and methodologies are outlined in the GRTA Letter of Understanding (LOU) dated June 20, 2022.





1.2 Site Access

As currently envisioned, the proposed development will be accessible via two (2) access points:

1. **Site Driveway A** – a proposed full-movement driveway located along Athens Highway (SR 10/US 78), approximately 850 feet east of Rosebud Road that is proposed to operate under side-street stop control.
2. **Site Driveway B** – a proposed full-movement driveway located along Athens Highway (SR 10/US 78), approximately 1,550 feet east of Rosebud Road that is proposed to operate under side-street stop control.

1.3 Internal Circulation Analysis

The proposed site is to be accessed via both Site Driveway A and Site Driveway B. Internal, private roadways throughout the site provide access to all residential and commercial buildings and parking facilities.

1.4 Parking

The current number of total site parking spaces to be provided are listed below in **Table 3**.

| Land Use | Parking Type | Minimum | Maximum | Proposed |
|---------------|--------------|------------------------------|--------------------------------|----------|
| Residential | Car | 993 (1.5 spaces per unit) | 1,986 (3.0 spaces per unit) | 1,115 |
| Restaurant | Car | 36 (1 space per 150 SF) | 72 (1 space per 75 SF) | 36 |
| Office/Retail | Car | 33 (1 space per 150 SF) | 84 (1 space per 75 SF) | 78 |
| Total | Car | Min: 1,062 | Max: 2,142 | 1,229 |

Additional parking details are provided on the proposed site plan in Appendix A. In addition to standard vehicle parking, electric vehicle charging stations will be provided in accordance with Gwinnett County Standards and will be coordinated with the County during the permitting process.

1.5 Alternative Transportation Facilities

Pedestrian facilities are currently provided throughout the study network, though sidewalk gaps do exist. Bicycle and transit facilities are currently not provided nearby. Internal pedestrian 5-foot sidewalk facilities are proposed to be included throughout the site.

1.6 Enhanced Focus Area for Dense Urban Environments

Per Section 3.2.4.2 of the GRTA *Development of Regional Impact Review Procedures*, the *Aventon Park* development does not qualify for this enhanced focus area review.

2.0 TRAFFIC ANALYSES, METHODOLOGY AND ASSUMPTIONS

2.1 Study Network Determination

The study area was determined at the methodology meeting with input from GRTA, ARC, and other local agency stakeholders. The study includes the following five (5) off-site intersections described in **Table 4** and shown in **Figure 3**.

Table 4: Intersection Control Summary

| Intersection | Jurisdiction | Control |
|--|----------------------|---------------------|
| 1. Athens Highway (SR 10/US 78) at Cooper Road | Gwinnett County/GDOT | Signalized |
| 2. Athens Highway (SR 10/US 78) at Rosebud Road | Gwinnett County/GDOT | Signalized |
| 3. Athens Highway (SR 10/US 78) at Midway Plaza Entrance | Gwinnett County/GDOT | Unsignalized (TWSC) |
| 4. Athens Highway (SR 10/US 78) at Langley Road | Gwinnett County/GDOT | Unsignalized (TWSC) |
| 5. Rosebud Road at Cooper Road | Gwinnett County | Signalized |

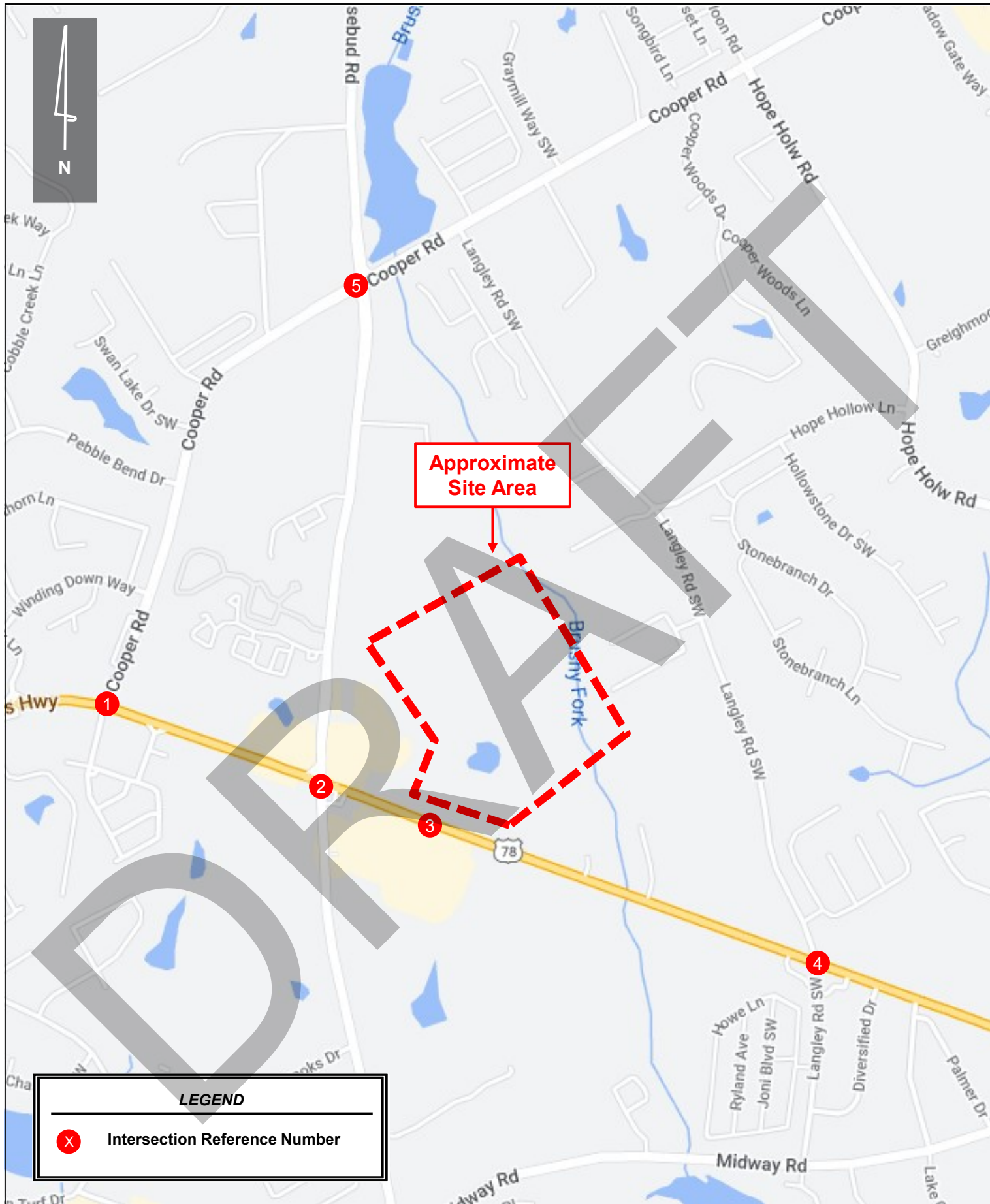
2.2 Existing Roadway Facilities

Roadway classification descriptions and estimated Annual Average Daily Traffic (AADT) for roadway segments within the study network are provided in **Table 5** (bolded roadways are adjacent to the site).

Table 5: Roadway Classifications

| Roadway | Lanes | Posted Speed Limit | AADT (GDOT, 2019) | GDOT Functional Classification |
|---------------------------------|----------|--------------------|-------------------|--------------------------------|
| Athens Hwy (SR 10/US 78) | 4 | 55 MPH | 38,800 | Principal Arterial |
| Cooper Road | 2 | 45 MPH | - | Local |
| Rosebud Road | 2 | 45 MPH | 15,700 | Minor Arterial |
| Langley Road | 2 | 30 /40 MPH* | - | Local |

*Langley Road is 30 MPH south of Athens Hwy (SR 10/US 78) and 40 MPH north of Athens Hwy (SR 10/US 78).



2.3 Traffic Data Collection and Calibration

Traffic counts were collected at all five (5) existing study intersections on Thursday, April 14, 2022. The collected counts were calibrated using adjustment factors to determine the potential impacts of COVID-19 to typical traffic volumes and patterns.

The peak hour adjustment factors were determined by comparing the AM and PM peak volumes at a newly collected average daily traffic (ADT) count to the AM and PM peak ADT volumes previously collected by GDOT in the same location at Station 135-0061. A comparison was conducted for vehicular volumes along Athens Highway (SR 10/US 78) east of Midway Road.

As a result of the volume comparison between the newly collected ADT and the GDOT Historical ADT, it was determined that an adjustment factor of 1.11 should be used for the existing AM turning movement counts, and an adjustment factor of 1.00 should be used for the existing PM turning movement counts. The methodologies used in this analysis for traffic count calibration were approved by GRTA.

The methodologies used in this analysis for traffic count calibration were approved by GRTA and ARC and included in the methodology meeting packet.

Traffic count peak hours for all the study intersections are shown in **Table 6**. The collected peak hour turning movement traffic counts are available upon request.

| Table 6: Traffic Count Summary | | | |
|--|------------|----------------|----------------|
| Intersection | Count Date | AM Peak Hour | PM Peak Hour |
| 1. Athens Highway (SR 10) at Cooper Road | 4/2022 | 7:30 – 8:30 AM | 5:00 – 6:00 PM |
| 2. Athens Highway (SR 10) at Rosebud Road | 4/2022 | 7:15 – 8:15 AM | 5:00 – 6:00 PM |
| 3. Athens Highway (SR 10) at Midway Plaza Entrance | 4/2022 | 7:15 – 8:15 AM | 5:00 – 6:00 PM |
| 4. Athens Highway (SR 10) at Langley Road | 4/2022 | 7:15 – 8:15 AM | 5:00 – 6:00 PM |
| 5. Rosebud Road at Cooper Road | 4/2022 | 7:15 – 8:15 AM | 5:00 – 6:00 PM |

2.4 Background Growth

Background traffic is defined as expected traffic on the roadway network in future year(s) absent the construction and opening of the proposed *Aventon Park* development. Background traffic can include a base growth rate, which is based on historical count data and population growth data as well as trips anticipated from nearby or adjacent other projects.

Based on methodology outlined in the GRTA Letter of Understanding (LOU), a 1.5% per year background traffic growth rate from 2022 to 2026 (4 years) was used for all roadways.

The Projected 2026 No-Build conditions represent the Calibrated 2022 traffic volumes grown for four (4) years at 1.5% per year throughout the study network.

The Projected 2026 Build conditions represent the project trips generated by the *Aventon Park* development (discussed in Section 3.0 and 4.0) added to the Projected 2026 No-Build Conditions.

2.5 Programmed and Planned Projects

Programmed and planned projects near the project site were researched to account for any improvements or modifications within the study network before or by the build-out year of the development. No programmed or planned projects were identified in the methodology meeting with GRTA, ARC, and other local stakeholders.

2.6 Level-of-Service Overview

Level-of-service (LOS) is used to describe the operating characteristics of a road segment or intersection in relation to its capacity. LOS is defined as a qualitative measure that describes operational conditions and motorists' perceptions within a traffic stream. The *Highway Capacity Manual* defines six levels-of-service, LOS A through LOS F, with A being the best and F being the worst. LOS analyses were conducted at all intersections within the study network using *Synchro 11*. Existing traffic signal phasing and timing data were retrieved for available intersections.

LOS for signalized intersections and all-way stop controlled intersections are reported for the overall intersection. One or more movements at an intersection may experience a low LOS while the overall intersection may operate acceptably.

LOS for unsignalized intersections with stop control on the minor street only is reported for the side street approaches and the major street left-turn movements. Low LOS for side street approaches is not uncommon, as vehicles may experience delays in turning onto a major roadway.

2.7 Level-of-Service Standards

For the purposes of this traffic analysis, a LOS standard of D was assumed for all study intersections per section 3.2.2.1 of the GRTA *Development of Regional Impact Review Procedures* as specified in the LOU.

3.0 TRIP GENERATION

Gross trips associated with the proposed development were estimated using the *Institute of Transportation Engineers' (ITE) Trip Generation Manual, 11th Edition*, using equations where available. Reductions to gross trips including mixed-use reductions and alternative transportation mode reductions are considered in the analysis based on methodology outlined in the GRTA Letter of Understanding (LOU).

Mixed-use reductions occur when a site has a combination of different land uses that interact with one another. For example, people living in a residential development may walk to the restaurants and retail instead of driving off-site or to the site. This reduces the number of vehicle trips that will be made on the roadway, thus reducing traffic congestion. Mixed-use reductions were taken in this analysis per the LOU.

Alternative modes reductions are taken when a site can be accessed by modes other than vehicles (walking, bicycling, transit, etc.). Alternative modes reductions were not taken in this analysis per the LOU.

Pass-by reductions are taken for a site when traffic normally traveling along a roadway may choose to visit a retail or restaurant establishment that is along the vehicle's path. These trips were already on the road and would therefore only be new trips on the driveways. Pass-by trips were taken for this analysis per the LOU.

Table 7 summarizes the gross trip generation, reductions, net trip generation, and driveway volumes for the proposed *Aventon Park* development.

| Table 7: Trip Generation | | | | | | | | |
|---|-------------|---------------|-------|-------|--------------|------|--------------|------|
| Land Use | Density | Daily Traffic | | | AM Peak Hour | | PM Peak Hour | |
| | | Total | Enter | Exit | Enter | Exit | Enter | Exit |
| Proposed Project Trips | | | | | | | | |
| 221 – Multi-family Housing (Mid-Rise) | 662 units | 3,112 | 1,556 | 1,556 | 64 | 216 | 158 | 101 |
| 712 – Small Office Building | 7,100 units | 102 | 51 | 51 | 10 | 2 | 5 | 10 |
| 822 – Strip Retail Plaza | 9,600 SF | 636 | 318 | 318 | 17 | 11 | 38 | 38 |
| 932 – High-Turnover (Sit-Down) Restaurant | 5,400 SF | 580 | 290 | 290 | 28 | 24 | 30 | 19 |
| Gross Project Trips | | 4,430 | 2,215 | 2,215 | 119 | 253 | 231 | 168 |
| Mixed-Use Reductions | | -286 | -144 | -142 | -15 | -15 | -45 | -45 |
| Alternative Mode Reductions | | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-By Reductions | | -412 | -207 | -205 | 0 | 0 | -12 | -12 |
| New Trips | | 3,732 | 1,864 | 1,868 | 104 | 238 | 174 | 111 |

A more detailed trip generation analysis summary table is provided in **Appendix B**.

4.0 TRIP DISTRIBUTION AND ASSIGNMENT

The distribution of new project trips was based on the project land uses, a review of land use densities and road facilities in the area, engineering judgement, and methodology discussions with GRTA, ARC, and other local stakeholders.

The anticipated assignment of the site project trips throughout the study roadway network is shown in **Figure 4**. These trip assignment percentages were applied to the net project trips expected to be generated by the development, and the volumes were assigned to the roadway network. The peak hour project trips are shown by turning movement throughout the study network in **Figure 5**.

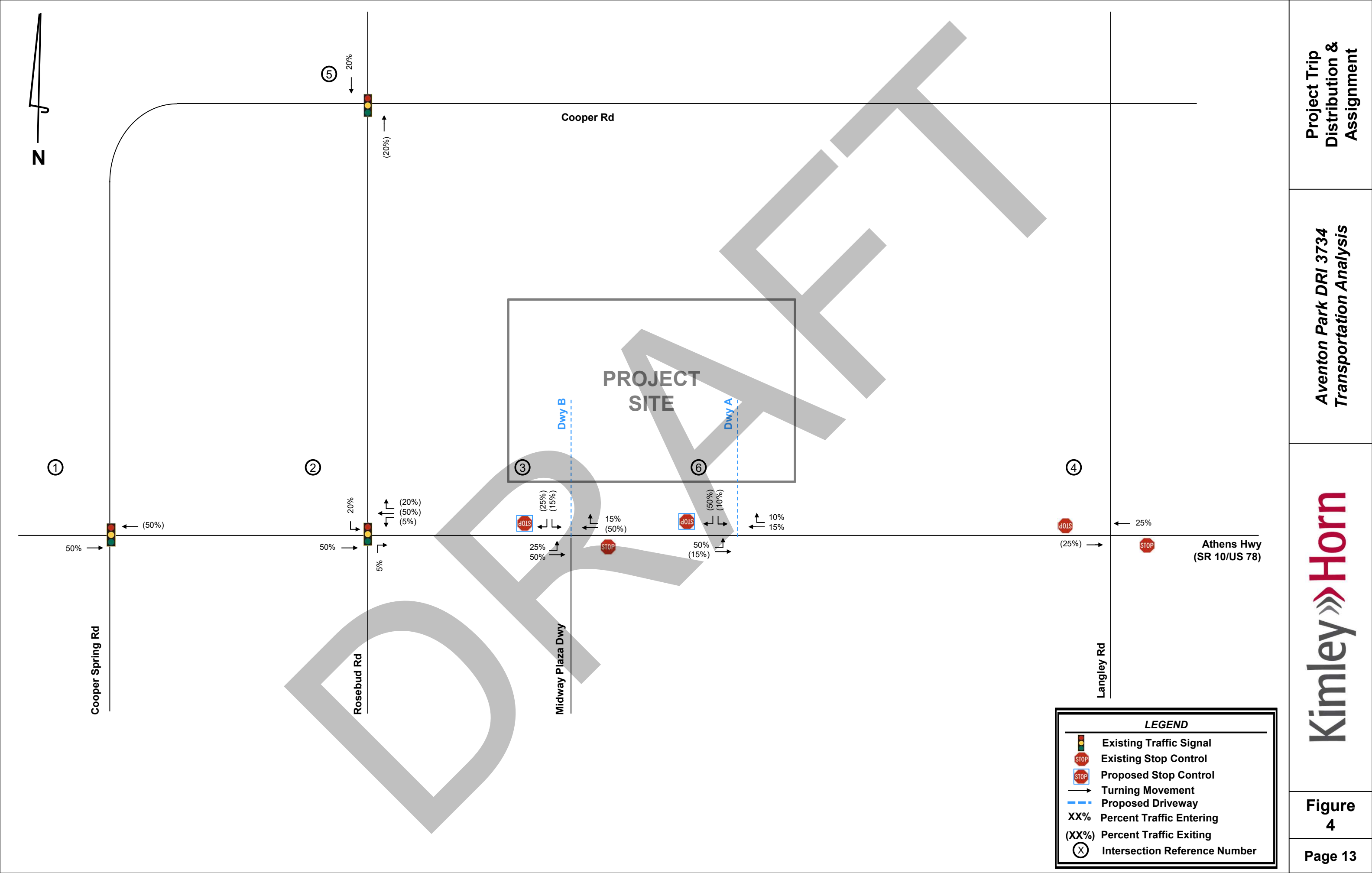
Detailed intersection volume worksheets are provided in **Appendix C**.

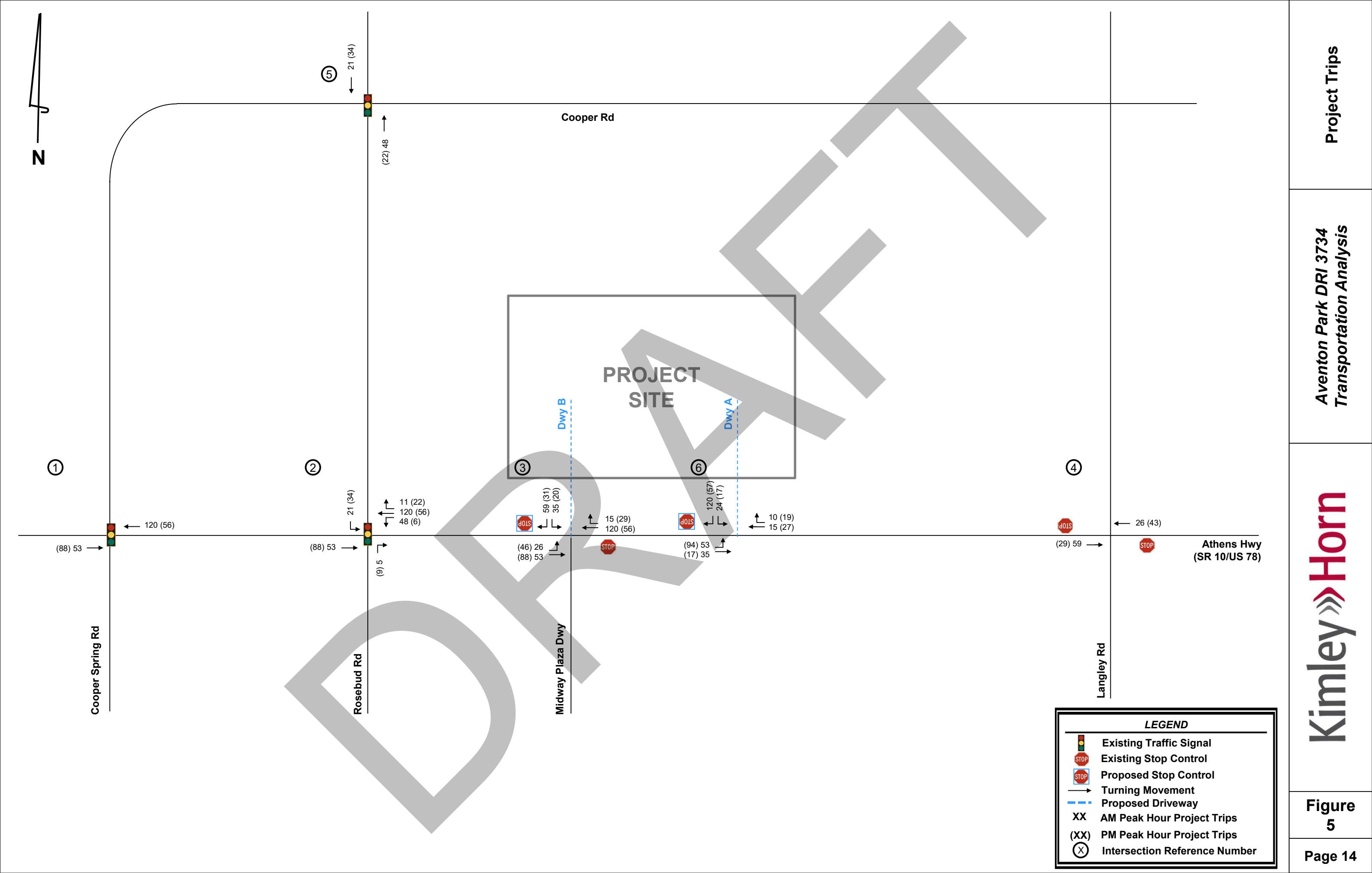
5.0 TRAFFIC ANALYSIS

Capacity analyses were performed using *Synchro 11* for the AM and PM peak hours under the Calibrated 2022 conditions, Projected 2026 No-Build conditions, and Projected 2026 Build conditions. The capacity analyses were performed using methodologies from the *Highway Capacity Manual (HCM)*, 6th Edition unless otherwise noted.

These analyses included existing roadway laneage for each of the scenarios. The traffic volumes and roadway laneage used for each scenario are shown in Error! Reference source not found. **Figure 6** for Calibrated 2022 conditions, **Figure 7** for Projected 2026 No-Build conditions, and **Figure 8** for Projected 2026 Build conditions.

Sections 5.1 – 5.6 provide the results of the capacity analyses are presented for each study intersection and include projected LOS, delay, and queue lengths.





5.1 Athens Highway (SR 10/US 78) at Cooper Road/Cooper Springs Road (Intersection 1)

| | | | | | | | | | | | | | |
|---|----|---------------------|----------|-----|-------------|----------|-----|-----------------------------|----------|-----|-----------------------------|----------|-----|
| Overall LOS Standard: D Approach LOS Standard: D | | Cooper Springs Road | | | Cooper Road | | | Athens Highway (SR10/US 78) | | | Athens Highway (SR10/US 78) | | |
| | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | |
| | | L | T | R | L | T | R | L | T | R | L | T | R |
| 2022 CALIBRATED (SIGNAL) | AM | Overall LOS | A (8.3) | | | | | | | | | | |
| | | Approach LOS | D (49.6) | | | D (53.0) | | | A (8.5) | | | A (1.0) | |
| | | Storage | | | | 140 | | | 100 | | 250 | 115 | 115 |
| | | 50th Queue | 17 | 13 | | 126 | 146 | | 138 | 132 | 0 | 2 | 133 |
| | | 95th Queue | 49 | 41 | | 205 | 279 | | 270 | 162 | 1 | 3 | 169 |
| | PM | Overall LOS | C (28.0) | | | | | | | | | | |
| | | Approach LOS | D (51.8) | | | D (52.4) | | | C (27.0) | | | C (24.7) | |
| | | Storage | | | | 140 | | | 100 | | 250 | 115 | 115 |
| | | 50th Queue | 35 | 45 | | 56 | 59 | | 427 | 485 | 2 | 14 | 308 |
| | | 95th Queue | 77 | 96 | | 102 | 169 | | 653 | 556 | 17 | 29 | 356 |
| 2026 NO-BUILD (SIGNAL) | AM | Overall LOS | A (8.5) | | | | | | | | | | |
| | | Approach LOS | D (49.8) | | | D (54.1) | | | A (9.1) | | | A (0.9) | |
| | | Storage | | | | 140 | | | 100 | | 250 | 115 | 115 |
| | | 50th Queue | 18 | 14 | | 135 | 174 | | 151 | 142 | 0 | 3 | 170 |
| | | 95th Queue | 54 | 42 | | 216 | 344 | | 297 | 174 | 1 | 3 | 170 |
| | PM | Overall LOS | C (33.1) | | | | | | | | | | |
| | | Approach LOS | D (52.1) | | | D (52.8) | | | C (33.8) | | | C (28.0) | |
| | | Storage | | | | 140 | | | 100 | | 250 | 115 | 115 |
| | | 50th Queue | 37 | 48 | | 60 | 80 | | 515 | 542 | 3 | 16 | 328 |
| | | 95th Queue | 85 | 101 | | 109 | 199 | | 746 | 620 | 18 | 31 | 402 |
| 2026 BUILD (SIGNAL) | AM | Overall LOS | A (8.5) | | | | | | | | | | |
| | | Approach LOS | D (49.8) | | | D (54.1) | | | B (10.1) | | | A (0.8) | |
| | | Storage | | | | 140 | | | 100 | | 250 | 115 | 115 |
| | | 50th Queue | 18 | 14 | | 135 | 176 | | 151 | 155 | 0 | 3 | 196 |
| | | 95th Queue | 54 | 42 | | 216 | 347 | | 297 | 188 | 1 | 2 | 164 |
| | PM | Overall LOS | D (35.3) | | | | | | | | | | |
| | | Approach LOS | D (52.1) | | | D (52.8) | | | D (36.8) | | | C (29.4) | |
| | | Storage | | | | 140 | | | 100 | | 250 | 115 | 115 |
| | | 50th Queue | 38 | 52 | | 60 | 80 | | 540 | 604 | 4 | 14 | 313 |
| | | 95th Queue | 86 | 106 | | 109 | 199 | | 770 | 690 | 19 | 28 | 423 |

The signalized intersection of Athens Highway (SR 10/ US 78) at Cooper Road/Cooper Springs Road (Intersection 1) is projected to operate at an acceptable overall LOS under the Calibrated 2022, No-Build 2025, and Build 2025 conditions. Each approach of the intersection is projected to operate acceptably under all studied scenarios. No improvements are recommended to be conditioned.

5.2 Athens Highway (SR 10/US 78) at Rosebud Road (Intersection 2)

Overall LOS Standard: D
Approach LOS Standard: D

| | | Rosebud Road | | | Rosebud Road | | | Athens Highway (SR 10/US 78) | | | Athens Highway (SR 10/US 78) | | |
|-----------------------------|----|--------------|----------|-----|--------------|----------|-----|---------------------------------|----------|------|---------------------------------|----------|-----|
| | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | |
| | | L | T | R | L | T | R | L | T | R | L | T | R |
| 2022 CALIBRATED (SIGNAL) | AM | Overall LOS | D (39.4) | | | | | | | | | | |
| | | Approach LOS | D (53.0) | | | D (49.2) | | | B (13.3) | | | D (43.1) | |
| | | Storage | 300 | | 425 | 240 | | 320 | | 75 | 280 | | 300 |
| | | 50th Queue | 126 | 426 | 0 | 79 | 307 | 0 | 11 | 341 | 50 | 737 | 25 |
| | | 95th Queue | 189 | 616 | 46 | 144 | 471 | 0 | 30 | 410 | 85 | 916 | 74 |
| | PM | Overall LOS | C (28.0) | | | | | | | | | | |
| | | Approach LOS | E (69.8) | | | E (68.1) | | | A (6.3) | | | C (20.1) | |
| | | Storage | 300 | | 425 | 240 | | 320 | | 75 | 280 | | 300 |
| | | 50th Queue | 104 | 352 | 0 | 126 | 376 | 0 | 33 | 844 | 91 | 343 | 0 |
| | | 95th Queue | 161 | 519 | 53 | 190 | 588 | 0 | 55 | 963 | 231 | 402 | 35 |
| 2026 NO-BUILD (SIGNAL) | AM | Overall LOS | D (46.6) | | | | | | | | | | |
| | | Approach LOS | E (58.3) | | | D (53.5) | | | B (14.6) | | | D (54.5) | |
| | | Storage | 300 | | 425 | 240 | | 320 | | 75 | 280 | | 300 |
| | | 50th Queue | 134 | 461 | 0 | 84 | 334 | 0 | 12 | 368 | 54 | 848 | 32 |
| | | 95th Queue | 200 | 677 | 48 | 200 | 516 | 0 | 33 | 441 | 90 | 1,016 | 84 |
| | PM | Overall LOS | C (31.2) | | | | | | | | | | |
| | | Approach LOS | E (76.0) | | | E (73.6) | | | A (9.6) | | | C (21.2) | |
| | | Storage | 300 | | 425 | 240 | | 320 | | 75 | 280 | | 300 |
| | | 50th Queue | 110 | 380 | 0 | 134 | 407 | 0 | 37 | 940 | 100 | 374 | 0 |
| | | 95th Queue | 175 | 571 | 54 | 243 | 642 | 0 | 55 | 1070 | 253 | 438 | 36 |
| 2026 BUILD (SIGNAL) | AM | Overall LOS | D (51.2) | | | | | | | | | | |
| | | Approach LOS | E (72.2) | | | E (75.1) | | | B (11.3) | | | D (54.9) | |
| | | Storage | 300 | | 425 | 240 | | 320 | | 75 | 280 | | 300 |
| | | 50th Queue | 141 | 461 | 0 | 102 | 334 | 0 | 12 | 379 | 56 | 954 | 38 |
| | | 95th Queue | 308 | 677 | 49 | 258 | 516 | 0 | 30 | 452 | 92 | 1091 | 95 |
| | PM | Overall LOS | C (34.3) | | | | | | | | | | |
| | | Approach LOS | E (75.5) | | | E (75.6) | | | B (15.7) | | | C (23.1) | |
| | | Storage | 300 | | 425 | 240 | | 320 | | 75 | 280 | | 300 |
| | | 50th Queue | 110 | 380 | 0 | 164 | 407 | 0 | 36 | 1029 | 113 | 402 | 0 |
| | | 95th Queue | 174 | 571 | 56 | 345 | 642 | 0 | 49 | 1175 | 269 | 470 | 37 |

The intersection of Athens Highway (SR10/US 78) at Rosebud Road (Intersection 2) is projected to operate at an acceptable overall LOS under the Calibrated 2022, 2025 No-Build, and 2025 Build conditions. The northbound and southbound approaches are projected to operate at LOS D and LOS E during the AM and PM peak hours, respectively, under the Calibrated 2022 conditions. Additionally, the northbound approach is projected to continue operating at LOS E during the AM and PM peak hours under all future scenarios. Similarly, the southbound approach is projected to operate at LOS E during the PM peak hour under the 2025 No-Build conditions and during the AM and PM peak hours under the 2025 Build conditions.

It should be noted that per GRTA's DRI guidelines, an improvement should be considered if an approach operates at a failing LOS, even if the overall intersection operates acceptably. The intersection operates at an acceptable overall LOS, and existing signal timings and cycle lengths prioritize vehicular progression on the mainline (Athens Highway / SR 10 / US 78) at the expense of side street operations. In order for all approaches of the intersection of Athens Highway (SR 10/US 78) at Rosebud Road to operate at LOS D or better, Rosebud Road must be widened to provide an additional travel lane. Although the northbound and southbound approaches are projected to operate at LOS E, no feasible improvements exist within the scope of this project. No improvements are recommended to be conditioned.

5.3 Athens Highway (SR 10/US 78) at Midway Plaza Driveway/Driveway B (Intersection 3)

Overall LOS Standard: D
Approach LOS Standard: D

| Overall LOS Standard: D Approach LOS Standard: D | | | Midway Plaza Driveway | | | Driveway B | | | Athens Highway (SR 10/US 78) | | | Athens Highway (SR 10/US 78) | | |
|---|----|--------------|-----------------------|----|----|------------|-----|---|------------------------------|-----|---|------------------------------|---|---|
| | | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | |
| | | | L | T | R | L | T | R | L | T | R | L | T | R |
| 2022 CALIBRATED (TWSC) | AM | Overall LOS | A (0.3) | | | | | | | | | | | |
| | | Approach LOS | C (20.8) | | | | | | A (0.0) | | | A (0.1) | | |
| | | Storage | | | | | | | | 200 | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 13 | | | | | | | 3 | | | |
| | PM | Overall LOS | A (1.0) | | | | | | | | | | | |
| | | Approach LOS | C (23.5) | | | | | | A (0.0) | | | A (0.3) | | |
| | | Storage | | | | | | | | 200 | | | | |
| 50th Queue | | | | | | | | | | | | | | |
| 95th Queue | | 40 | | | | | | | | 5 | | | | |
| 2026 NO-BUILD (TWSC) | AM | Overall LOS | A (0.4) | | | | | | | | | | | |
| | | Approach LOS | C (22.7) | | | | | | A (0.0) | | | A (0.1) | | |
| | | Storage | | | | | | | | 200 | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 15 | | | | | | | 3 | | | |
| | PM | Overall LOS | A (1.1) | | | | | | | | | | | |
| | | Approach LOS | D (27.5) | | | | | | A (0.0) | | | A (0.3) | | |
| | | Storage | | | | | | | | 200 | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 50 | | | | | | | | 5 | | |
| 2026 BUILD (TWSC) | AM | Overall LOS | B (14.3) | | | | | | | | | | | |
| | | Approach LOS | F (55.2) | | | F (482.1) | | | A (0.6) | | | A (0.1) | | |
| | | Storage | | | | | | | | 200 | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 40 | | | 230 | | 10 | | | 3 | | |
| | PM | Overall LOS | A (2.5) | | | | | | | | | | | |
| | | Approach LOS | E (46.2) | | | E (47.0) | | | A (0.4) | | | A (0.3) | | |
| | | Storage | | | | | | | | 200 | | | | |
| | | 50th Queue | | | | | | | | | | | | |
| 95th Queue | | 80 | | | 40 | | 10 | | | 5 | | | | |

The intersection of Athens Highway (SR 10/US 78) at Midway Plaza Driveway/Driveway B (Intersection 3) is projected to operate at an acceptable overall LOS under the Calibrated 2022, No-Build 2026, and Build 2026 conditions. The northbound and southbound approaches are projected to operate at LOS F and LOS E during the AM and PM peak hour, respectively. It should be noted that it is not uncommon to have long delays for minor street stop-controlled approaches when there is heavy major street volume.

The following should be considered to serve the Projected 2026 Build Conditions (needed to serve the development traffic):

- Athens Highway (SR 10/US 78) at Midway Plaza Driveway/Driveway B (Intersection 3) – Full Movement
 - Construct Driveway B to operate as a full movement, side-street stop-controlled intersection
 - Construct one (1) ingress lane entering the site and one (1) egress lane exiting the site
 - Construct one (1) westbound right-turn lane along Athens Highway (SR 10/ US 78)

The recommended build improvements are shown in **Figure 8**.

5.4 Athens Highway (SR 10/US 78) at Langley Road (Intersection 4)

Overall LOS Standard: D
Approach LOS Standard: D

| Overall LOS Standard: D Approach LOS Standard: D | | | Langley Road | | | Langley Road | | | Athens Highway (SR 10/US 78) | | | Athens Highway (SR 10/US 78) | | |
|---|----|--------------|--------------|----|---|--------------|-----|----|---------------------------------|---|-----|---------------------------------|---|---|
| | | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | |
| | | | L | T | R | L | T | R | L | T | R | L | T | R |
| 2022 CALIBRATED (TWSC) | AM | Overall LOS | A (7.2) | | | | | | | | | | | |
| | | Approach LOS | F (59.1) | | | F (126.4) | | | (0.6) | | | (0.1) | | |
| | | Storage | | | | | | | | | 160 | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 58 | | | 170 | | 8 | | | 3 | | |
| | PM | Overall LOS | A (7.8) | | | | | | | | | | | |
| | | Approach LOS | F (86.9) | | | F (139.4) | | | (0.5) | | | (0.4) | | |
| | | Storage | | | | | | | | | 160 | | | |
| 95th Queue | | | 65 | | | 185 | | 13 | | | 8 | | | |
| 2026 NO-BUILD (TWSC) | AM | Overall LOS | B (11.2) | | | | | | | | | | | |
| | | Approach LOS | F (91.3) | | | F (200.7) | | | (0.6) | | | (0.1) | | |
| | | Storage | | | | | | | | | 160 | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 80 | | | 218 | | 10 | | | 3 | | |
| | PM | Overall LOS | B (11.8) | | | | | | | | | | | |
| | | Approach LOS | F (127.9) | | | F (215.8) | | | (0.6) | | | (0.4) | | |
| | | Storage | | | | | | | | | 160 | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 85 | | | 235 | | 13 | | | 8 | | |
| 2026 BUILD (TWSC) | AM | Overall LOS | B (12.0) | | | | | | | | | | | |
| | | Approach LOS | F (102.9) | | | F (221.6) | | | (0.6) | | | (0.1) | | |
| | | Storage | | | | | | | | | 160 | | | |
| | | 50th Queue | | | | | | | | | | | | |
| | | 95th Queue | | 88 | | | 228 | | 10 | | | 3 | | |
| | PM | Overall LOS | B (13.5) | | | | | | | | | | | |
| | | Approach LOS | F (145.8) | | | F (256.7) | | | (0.6) | | | (0.4) | | |
| | | Storage | | | | | | | | | 160 | | | |
| | | 95th Queue | | 93 | | | 253 | | 15 | | | 8 | | |

The intersection of Athens Highway (SR 10/US 78) at Langley Road (Intersection 4) is projected to operate at an acceptable overall LOS under the Calibrated 2022, No-Build 2026, and Build 2026 conditions during the AM and PM peak hours. The northbound and southbound approaches are projected to operate at LOS F under the Calibrated 2022, No-Build 2026, and Build 2026 conditions during the AM and PM peak hours. It should be noted that it is not uncommon to have long delays for minor street stop-controlled approaches when there is heavy major street volume. No improvements are recommended to be conditioned.

5.5 Cooper Road at Rosebud Road (Intersection 5)

Overall LOS Standard: D
Approach LOS Standard: D

| Overall LOS Standard: D Approach LOS Standard: D | | Rosebud Road | | | Rosebud Road | | | Cooper Road | | | Cooper Road | | | |
|---|----|--------------|----------|------|--------------|----------|-----|-------------|----------|-----|-------------|----------|-----|--|
| | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | | |
| | | L | T | R | L | T | R | L | T | R | L | T | R | |
| 2022 CALIBRATED (Signal) | AM | Overall LOS | D (40.3) | | | | | | | | | | | |
| | | Approach LOS | D (43.0) | | | C (32.4) | | | D (37.4) | | | D (45.5) | | |
| | | Storage | 200 | | 600 | 575 | | 600 | 600 | | | 600 | | |
| | | 50th Queue | 51 | 628 | 0 | 55 | 323 | 0 | 104 | 206 | | 74 | 643 | |
| | | 95th Queue | 88 | 806 | 31 | 101 | 432 | 49 | 249 | 289 | | 117 | 874 | |
| | PM | Overall LOS | C (32.8) | | | | | | | | | | | |
| | | Approach LOS | D (37.4) | | | C (27.7) | | | D (36.1) | | | C (30.3) | | |
| | | Storage | 200 | | 600 | 575 | | 600 | 600 | | | 600 | | |
| | | 50th Queue | 13 | 355 | 16 | 72 | 302 | 8 | 68 | 378 | | 86 | 354 | |
| | | 95th Queue | 34 | 567 | 78 | 133 | 473 | 59 | 120 | 577 | | 147 | 543 | |
| 2026 NO-BUILD (Signal) | AM | Overall LOS | D (45.1) | | | | | | | | | | | |
| | | Approach LOS | D (47.6) | | | D (35.1) | | | D (45.0) | | | D (50.2) | | |
| | | Storage | 200 | | 600 | 575 | | 600 | 600 | | | 600 | | |
| | | 50th Queue | 54 | 686 | 0 | 58 | 348 | 0 | 148 | 221 | | 80 | 704 | |
| | | 95th Queue | 92 | 921 | 37 | 141 | 462 | 49 | 317 | 307 | | 124 | 964 | |
| | PM | Overall LOS | D (35.9) | | | | | | | | | | | |
| | | Approach LOS | D (41.4) | | | C (30.0) | | | D (40.0) | | | C (32.5) | | |
| | | Storage | 200 | | 600 | 575 | | 600 | 600 | | | 600 | | |
| | | 50th Queue | 14 | 419 | 25 | 84 | 355 | 15 | 76 | 431 | | 97 | 402 | |
| | | 95th Queue | 36 | 673 | 92 | 145 | 527 | 68 | 127 | 685 | | 157 | 601 | |
| 2026 BUILD (Signal) | AM | Overall LOS | D (48.9) | | | | | | | | | | | |
| | | Approach LOS | D (51.8) | | | D (36.4) | | | D (51.5) | | | D (53.8) | | |
| | | Storage | 200 | | 600 | 575 | | 600 | 600 | | | 600 | | |
| | | 50th Queue | 54 | 774 | 0 | 63 | 373 | 0 | 156 | 222 | | 80 | 704 | |
| | | 95th Queue | 92 | 1048 | 37 | 168 | 494 | 49 | 325 | 309 | | 124 | 964 | |
| | PM | Overall LOS | D (37.2) | | | | | | | | | | | |
| | | Approach LOS | D (42.7) | | | C (31.4) | | | D (41.7) | | | C (33.6) | | |
| | | Storage | 200 | | 600 | 575 | | 600 | 600 | | | 600 | | |
| | | 50th Queue | 14 | 450 | 30 | 85 | 397 | 21 | 76 | 431 | | 97 | 402 | |
| | | 95th Queue | 36 | 748 | 100 | 145 | 584 | 75 | 131 | 705 | | 162 | 613 | |

The intersection of Cooper Road at Rosebud Road (Intersection 5) is projected to operate at an acceptable overall LOS under the Calibrated 2022, No-Build 2025, and Build 2025 conditions. Each approach of the intersection is projected to operate acceptably under all studied scenarios. No improvements are recommended to be conditioned.

5.6 Athens Highway (SR 10/US 78) at Driveway A (Intersection 6)

| | | | | | | | | | | | | | |
|--------------------------|----|--------------|---------|---|------------|-----------|---|------------------------------|-------|---|------------------------------|-------|---|
| Overall LOS Standard: D | | | | | | | | | | | | | |
| Approach LOS Standard: D | | | | | | | | | | | | | |
| | | - | | | Driveway A | | | Athens Highway (SR 10/US 78) | | | Athens Highway (SR 10/US 78) | | |
| | | Northbound | | | Southbound | | | Eastbound | | | Westbound | | |
| | | L | T | R | L | T | R | L | T | R | L | T | R |
| 2026 BUILD (TWSC) | AM | Overall LOS | A (9.8) | | | | | | | | | | |
| | | Approach LOS | | | | F (204.9) | | | (1.3) | | | (0.0) | |
| | | Storage | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | |
| | | 95th Queue | | | | 233 | | | | | | 23 | |
| | PM | Overall LOS | A (1.0) | | | | | | | | | | |
| | | Approach LOS | | | | D (26.7) | | | (0.8) | | | (0.0) | |
| | | Storage | | | | | | | | | | | |
| | | 50th Queue | | | | | | | | | | | |
| | | 95th Queue | | | | 33 | | | | | | 20 | |

The intersection of Athens Highway (SR 10/US 78) at Driveway A (Intersection 6) is projected to operate at an acceptable overall LOS under the Build 2025 scenario during the AM and PM peak hours. The southbound approach of the intersection is projected to operate at LOS F during the AM peak hour. It should be noted that it is not uncommon to have long delays for minor street stop-controlled approaches when there is heavy major street volume.

The following should be considered to serve the Projected 2026 Build Conditions (needed to serve the development traffic):

- Athens Highway (SR 10/US 78) at Driveway A (Intersection 6)
 - Construct Driveway A to operate as a full movement, side-street stop-controlled intersection
 - Construct one (1) ingress lane entering the site and one (1) egress lane exiting the site
 - Construct one (1) westbound right-turn lane along Athens Highway (SR 10/ US 78)

The recommended build improvements are shown in **Figure 8**.

6.0 INTERSECTION CONTROL EVALUATION (ICE)

Per GDOT's Policy, Intersection Control Evaluation (ICE) was performed at the following locations:

- Athens Highway (SR 10/US 78) at Midway Plaza Driveway/Driveway B (Intersection 3)
- Athens Highway (SR 10/US 78) at Driveway A (Intersection 6)

The intent of ICE is to determine the most effective intersection design/traffic control at a given intersection.

6.1 ICE Stage 1

Stage 1 is conducted early in the project development process and is intended to inform which alternatives are worthy of further evaluation in Stage 2. Stage 1 serves as a screening effort meant to eliminate non-competitive options and identify which alternatives merit further considerations based on their practical feasibility.

6.2 ICE Stage 2

Stage 2 involves a more detailed evaluation of the alternatives identified in Stage 1 in order to support the selection of a preferred alternative that may be advanced to detailed design. Stage 2 considers the construction cost, operational efficiency, safety considerations, and public opinion.

The intersection delays and v/c (volume-capacity) ratios were calculated at the study intersections during the AM and PM peak hour using Synchro Professional, Version 11.0, which uses methodologies contained in the 6th Edition Highway Capacity Manual to determine the operating characteristics of an intersection.

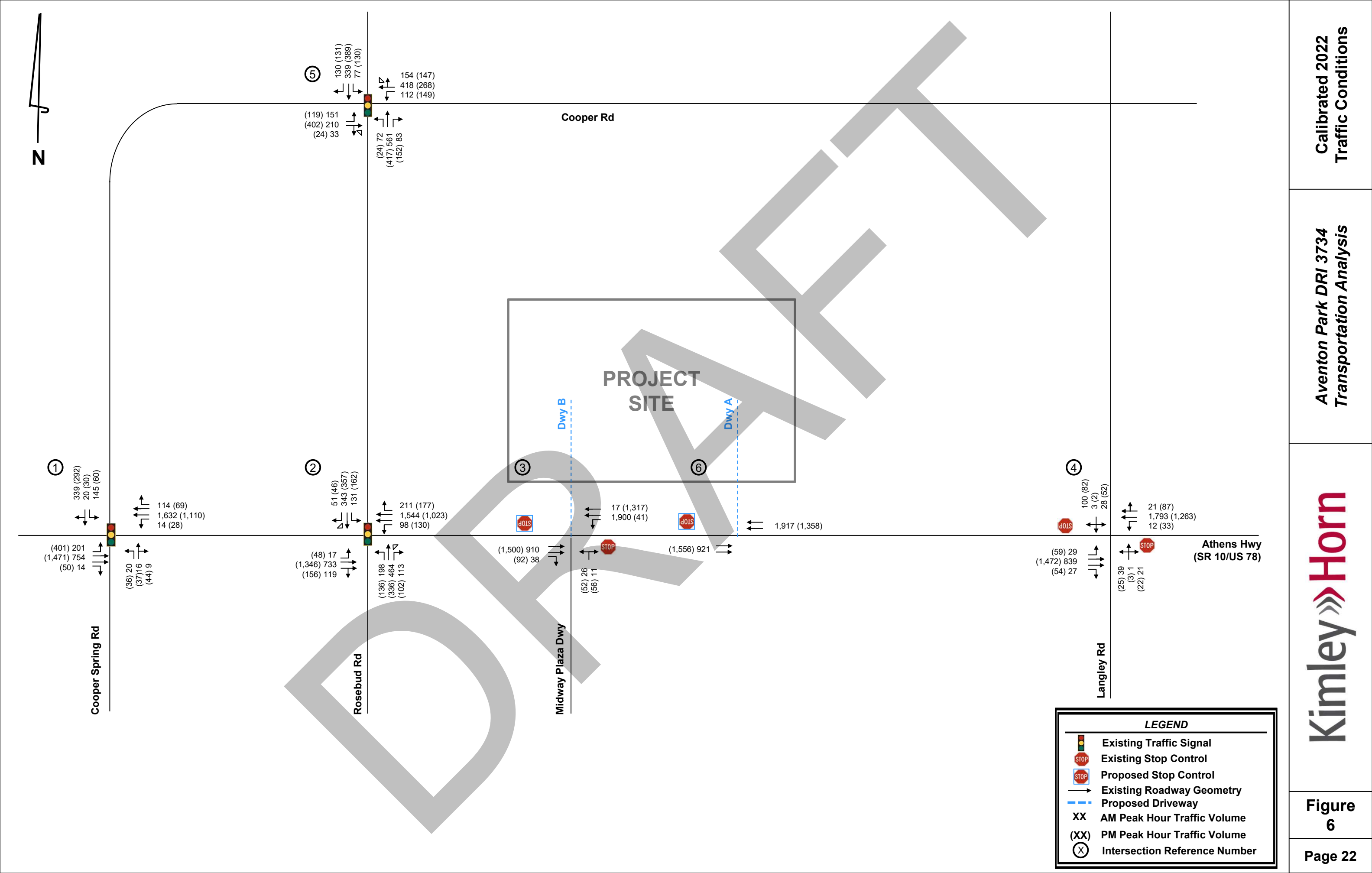
Per ICE Stage 1, the following alternatives were compared, and the ICE Stage 2 scores are shown in Table 8.

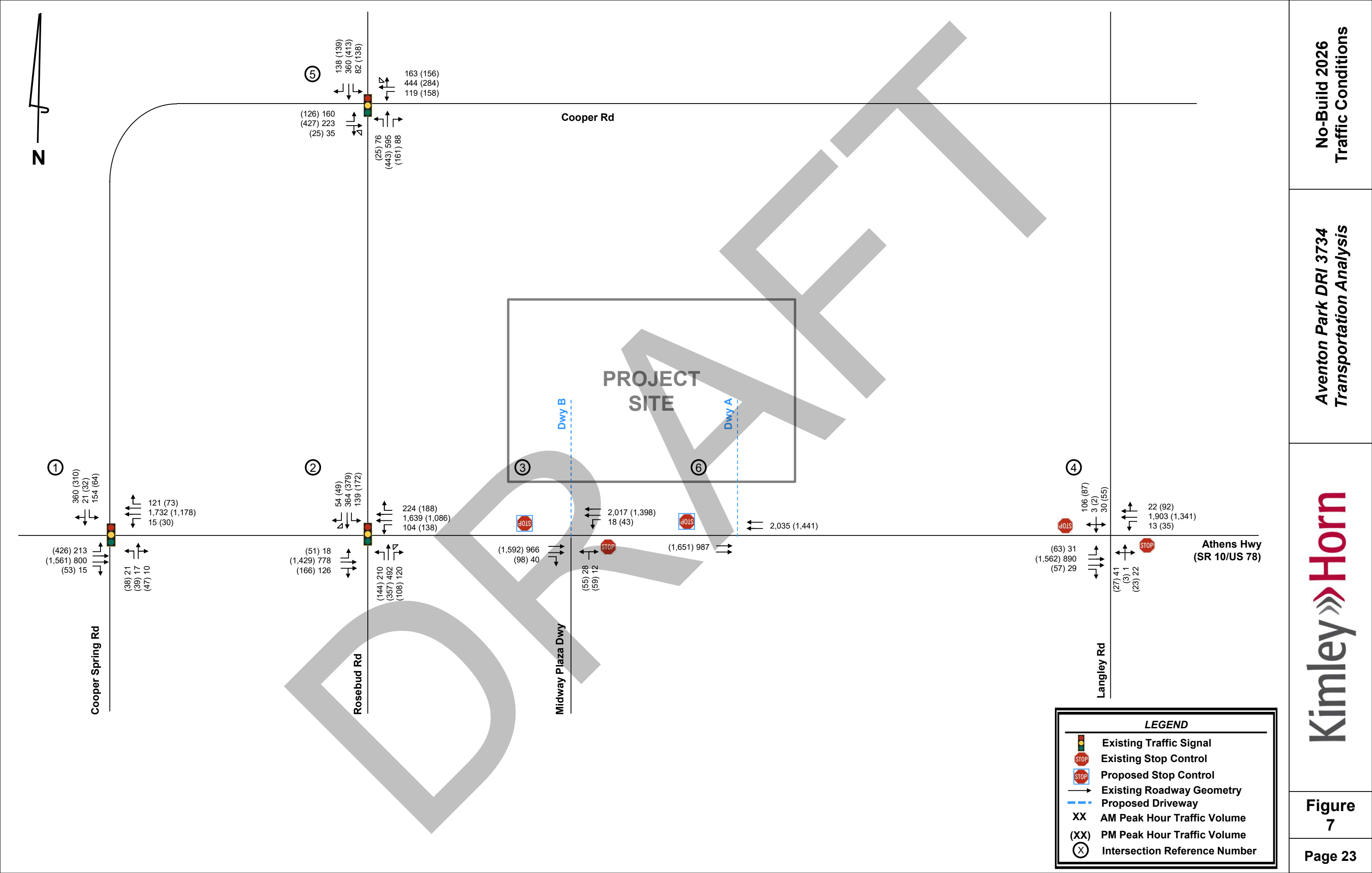
| Table 8: ICE Alternative Selection Decision | | | |
|---|--------------------------------------|--------------------------------|--------------------------------------|
| <i>Athens Highway (SR10/US 78) at Midway Plaza Driveway/Driveway B – Intersection 3</i> | | | |
| ICE Stage 2 | Conventional (Minor Stop) | RCUT (Stop Control) | RIRO w/Down Stream U-Turn |
| Score | 3.2 | 4.5 | 3.7 |
| Rank | 3 | 1 | 2 |
| <i>Athens Highway (SR10/US 78) at Driveway A – Intersection 6</i> | | | |
| ICE Stage 2 | Conventional (Minor Stop) | RCUT (Stop Control) | RIRO w/Down Stream U-Turn |
| Score | 3.8 | 2.6 | 1.7 |
| Rank | 1 | 2 | 3 |

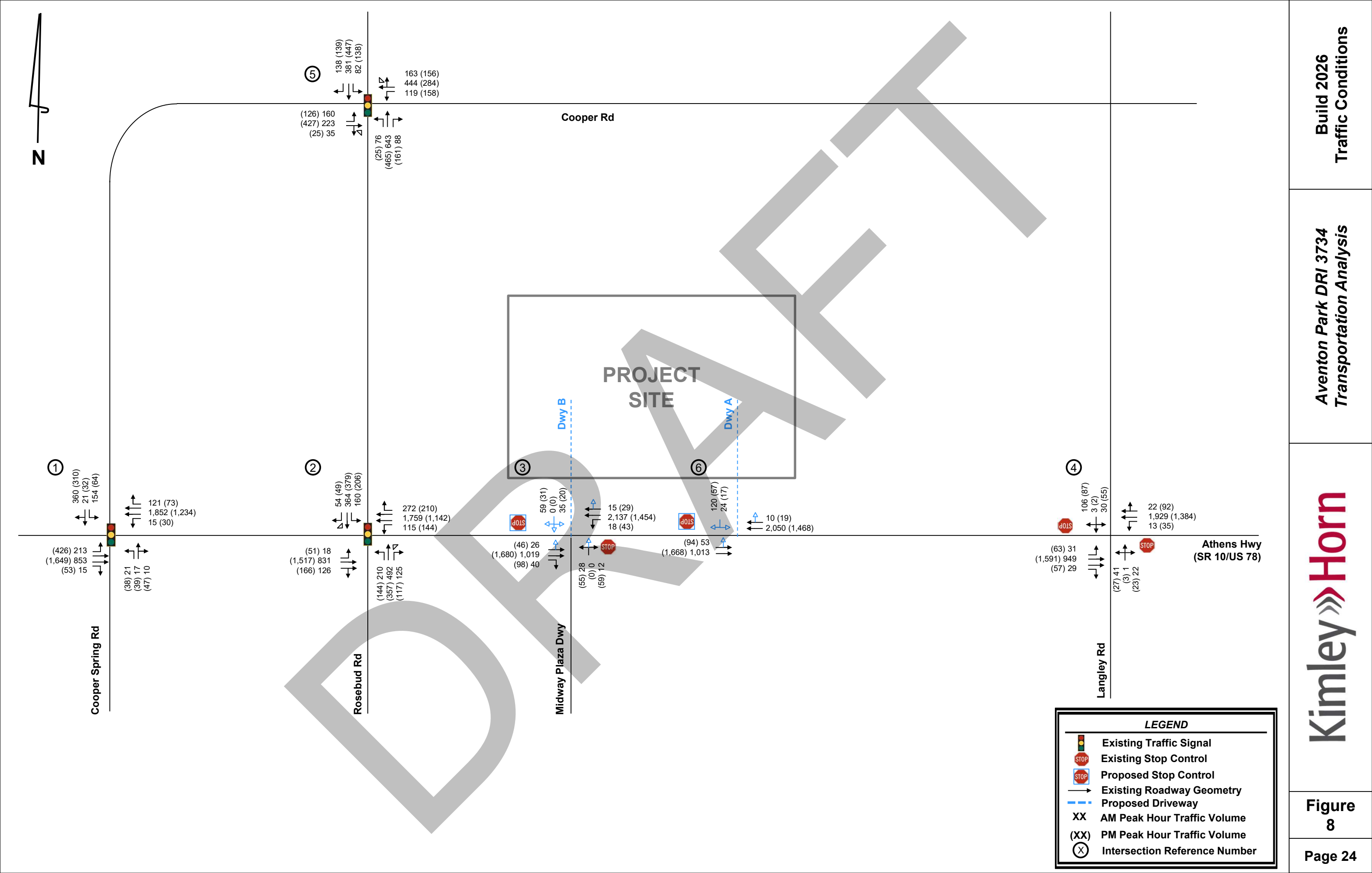
For Athens Highway (SR 10/US 78) at Midway Plaza Driveway/Driveway B (Intersection 3), a waiver is requested to select an alternative that did not rank #1 in ICE Stage 2. A conventional (minor stop) intersection is requested at the proposed site driveway to maintain existing full movement access as well as to match the overall characteristics of the corridor to avoid negatively impacting driver expectancy. A center two-way left-turn lane currently existing along Athens Hwy and no median break is required.

For Athens Highway (SR 10/US 78) at Driveway A (Intersection 6), the conventional (minor stop) ranks #1 and is requested.

GDOT's ICE Stage 1 and Stage 2 are provided in **Appendix D**.







Proposed Site Plan

DRAFT

- * A LAND DISTURBING PERMIT FOR PHASE 2 WILL BE ISSUED A MINIMUM OF 12 MONTHS AFTER THE LAND DISTURBING PERMIT FOR PHASE 1
- * IF THE ALTERNATE PARK LAYOUT IS APPROVED, IT SHALL BE BUILT CONCURRENT WITH THE PHASE I PROJECT USING PRIVATE FUNDS AND SHALL BE PRIVATELY MAINTAINED. THIS PARK SHALL BE OPEN FOR PUBLIC ACCESS DURING DAYLIGHT HOURS.

OPEN SPACE SUMMARY:
TOTAL PROJECT AREA: 57.76 AC
IMPERVIOUS COVERAGE: ±38%
PROPOSED OPEN SPACE/RESERVED AREA: ±26.97 AC

NOW OR FORMERLY
ROSEBUD 78 LAND, L
D.B. 59236, PG. 44
PARCEL ID: R5124 3
ZONED: RTH

PHASE 2:
GROSS AREA: 23.12 AC
NET AREA: 22.63 AC
13 UNITS/AC: 294 UNITS
PROPOSED ZONING: RM-13

NOW OR FORMERLY
 POWER J. & MARGARET JEFFREY
 D.B. 20796, PG. 242
 PARCEL ID: R5124 034

NOW OR FORMERLY
GLEYS FARMS HOMEOWN
ASSOCIATION, INC.
D.B. 47009, PG. 167
PARCEL ID: R5124 234
ZONED: R100CS0

NOW OR FORMERLY
FELTON PROPERTIES, LLC.
D.B. 59518, PG. 503
PARCEL ID: R5124 013
ZONED: RA200

DEVELOPER INFORMATION

AVENTON COMPANIES
1201 EDWARDS MILL ROAD, SUITE 120
RALEIGH, NC 27607
CONTACT: RON PERERA
TEL. 919-451-2093
rperera@aventoncompanies.com

AS-SURVEYED LEGAL DESCRIPTION

ALL THAT TRACT OR PARCEL OF LAND LYING AND BEING IN THE 5TH DISTRICT, LAND LOTS 100, 125 AND 124 OF GWINNETT COUNTY, GEORGIA AND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT A POINT ON THE NORTHWESTERLY SIDE OF U.S. HIGHWAY NO. 78, SAID POINT BEING 752 FEET SOUTHEASTERLY FROM THE INTERSECTION OF U.S. HIGHWAY 78 AND ROSEBUD ROAD, AS MEASURED ALONG THE NORTHWESTERLY SIDE OF U.S. HIGHWAY NO. 78, RUNNING THENCE NORTH 16°39' EAST, 307.7 FEET TO A 1/4" REBAR FOUND; RUNNING THENCE NORTH 30°38' 51" WEST, 800.30 FEET TO A 1/4" REBAR WITH CAP ("EARTHPO 0994") FOUND; RUNNING THENCE NORTH 59°23'24" EAST, 1468.26 FEET TO A PAINTED ROCK FOUND; RUNNING THENCE SOUTH 30°02'00" EAST, 1739.48 FEET TO A 1" SQUARE SPIKE FOUND; THENCE SOUTH 58°31'00" WEST, 1062.00 FEET TO A MAGNETIC NAIL SET, SAID POINT BEING 45 FEET NORTHEAST OF THE MID-LINE OF U.S. HIGHWAY NO. 78 AND BEING LOCATED ON THE NORTHEASTERLY SIDE OF SAID ROADWAY; RUNNING THENCE NORTH 70°03' 55" WEST, 966.75 FEET, AS MEASURED ALONG THE NORTHERLY SIDE OF U.S. HIGHWAY NO. 78, TO AN IRON PIN SET AND THE POINT OF BEGINNING.

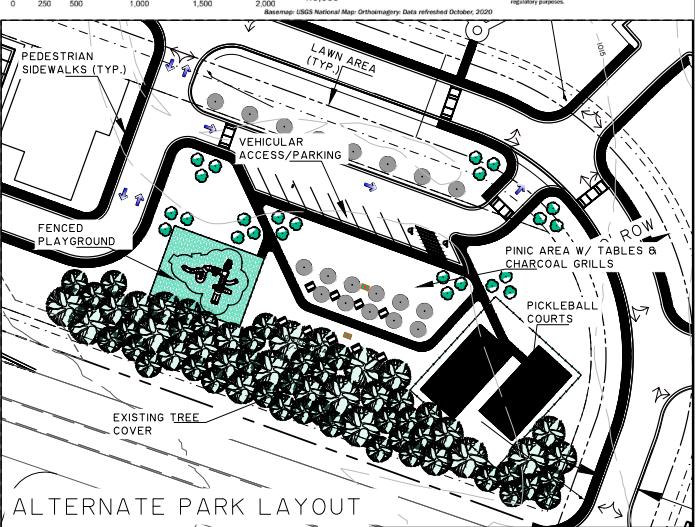
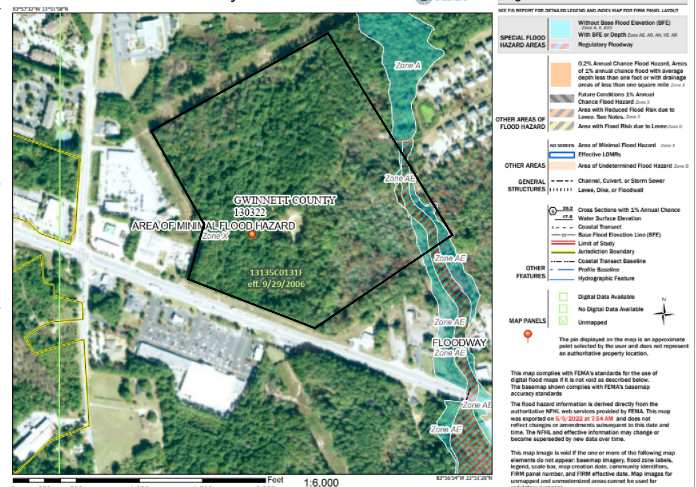
SAID TRACT OF LAND CONTAINING 57.7557 ACRES
OR 2,515,839.02 SQUARE FEET.

PROHIBITED USES:

No portion of the C-2 zoned land shall be utilized for a dry cleaning plant, coin laundry, billboards, drug-related paraphernalia shop, pool room, adult entertainment facility, massage parlor (except to the extent included as part of a day spa), adult book or video store, funeral parlor, flea market, bingo parlor, gaming, arcade, gambling, betting or game of chance business (exclusive of the sale of lottery tickets); sale, rental or lease of automobiles, trucks, other motorized vehicles or trailers; auto retail or repair business, gun or weapon shop, pawn shop, medical and/or recreational marijuana dispensing facility, plant nursery (but a florist or other establishment selling cut flowers or potted plants shall be allowed), or a tattoo parlor.

COMMERCIAL AREA: 3.34 AC
PROPOSED ZONING: C-2
FAR: 0.15
NOTE: COMMERCIAL LAYOUT
IS CONCEPTUAL. FINAL
PROGRAMMING SUBJECT TO
CHANGE

National Flood Hazard Layer FIRMette



TRAFFIC ENGINEER:
KIMLEY-HORN & ASSOCIATES, INC.
817 WEST PEACHTREE STREET
THE BILMORE, SUITE 601 ATLANTA, GA 30308
PHONE: (404) 419-8700
CONTACT: MATT FLYNN, P.E.

Zoning Dimensional Standards

| | | |
|------------------------------|--|-------|
| ZONING DISTRICT | RM-13 | C-2 |
| MIN. LOT AREA | 15,000 SF | NONE |
| MIN. LOT WIDTH | 100 FT | NONE |
| MIN. FRONT SETBACK | 15 FT | 15 FT |
| MIN. SIDE SETBACK | 10 FT | 10 FT |
| MIN. REAR SETBACK | 30 FT | 30 FT |
| MIN. HEATED FLOOR AREA | 750 SF | -- |
| MAXIMUM 3 BEDROOMS OR LARGER | PHASE 1: 36 UNITS PHASE 2: 29 UNITS | -- |
| MIN. % COM. AREA | 15% | N/A |
| MAX. NET DENSITY | 13 UNITS PER ACRE | NONE |
| MAX. BLDG. HEIGHT | 50 FT | 45 FT |
| PROPOSED BLDG. HEIGHT | 49'-6" | -- |

NOTE: ALL PROPOSED
MULTIFAMILY BUILDINGS
SHOWN ON PLAN SHALL
BE 4 STORIES

RM-13 PARKING REQUIREMENTS/PROPOSAL

| PHASE | PROPOSED UNITS | MIN. (1.5 X UNITS) | MAX. (3 X UNITS) | PROPOSED PARKING |
|-------|-------------------|-----------------------|---------------------|---------------------|
| 1 | 368 | 552 | 1,104 | 623 |
| 2 | 294 | 441 | 882 | 492 |

C-2 PARKING REQUIREMENTS/PROPOSAL

| USE/DEVELOPMENT | AREA | MIN. | MAX. | PROPOSED |
|-----------------|-----------|---------------------|---------------------|----------|
| RESTAURANT | 5,400 SF | 1 PER 150 SF= 36 PL | 1 PER 75 SF= 72 PL | 36 |
| OFFICE/RETAIL | 16,700 SF | 1 PER 500 SF= 33 PL | 1 PER 200 SF =84 PL | 78 |
| TOTAL PARKING | | 69 PL | 156 PL | 114 PL |

REZONING SITE PLAN

DRI #3437

GWINNETT COUNTY, GEORGIA

PREPARED FOR:

AVENTON COMPANIES

PREPARED BY:



5553 Peachtree Road • Suite 175
Chamblee, GA 30341 • 470.893.1700

www.thomasandhutton.com

| | |
|-----------|---------|
| JOB NO: | J-29887 |
| DRAWN: | NWP |
| REVIEWED: | GP |

| | |
|--------|-----------|
| DATE: | 08/18/22 |
| SCALE: | 1" = 100' |
| SHEET: | RZ-1 |

SITE DATA

| | |
|--------------------------|--------------------|
| SITE | Athens Highway SW |
| PARCEL NUMBERS..... | R5125-003 |
| TOTAL SITE AREA..... | 57.7557 ACRES |
| ZONING: | |
| CURRENT ZONING..... | R-100 |
| PROPOSED ZONING..... | RM-13, C-2 |
| ZONING JURISDICTION..... | WINNETT COUNTY, GA |

GRAPHIC SCALE



(IN FEET)
1 inch = 100 ft.

- * A LAND DISTURBING PERMIT FOR PHASE 2 WILL BE ISSUED A MINIMUM OF 12 MONTHS AFTER THE LAND DISTURBING PERMIT FOR PHASE 1
- * IF THE ALTERNATE PARK LAYOUT IS APPROVED, IT SHALL BE BUILT CONCURRENT WITH THE PHASE I PROJECT USING PRIVATE FUNDS AND SHALL BE PRIVATELY MAINTAINED. THIS PARK SHALL BE OPEN FOR PUBLIC ACCESS DURING DAYLIGHT HOURS.

OPEN SPACE SUMMARY:
TOTAL PROJECT AREA: 57.76 AC
IMPERVIOUS COVERAGE: ±38%
PROPOSED OPEN SPACE/RESERVED AREA: ±26.97 AC

NOW OR FORMERLY
ROSEBUD 78 LAND, LLC.
D.B. 59236, PG. 448
PARCEL ID: R5124 366
ZONED: RTH

HEAR SETBACK

NOW OR FORMERLY
POWER J. & MARGARET JEFFREY
D.B. 20796, PG. 242
PARCEL ID: R5124 034

PHASE 2:
GROSS AREA: 23.12 AC
NET AREA: 22.63 AC
13 UNITS/AC: 294 UNITS
PROPOSED ZONING: RM-13

NOW OR FORMERLY
GRACEPOINTE NAZARENE CHURCH
D.B. 25073, PG. 266
PARCEL ID: R5124 032
ZONED: R100

NOW OR FORMERLY
SAFeway PLAZA, INC.
D.B. 38579, PG. 174
PARCEL ID: R5101 179
ZONED: C2

DEVELOPER INFORMATION

AVENTON COMPANIES
1201 EDWARDS MILL ROAD, SUITE 120
RALEIGH, NC 27607
CONTACT: RON PERERA
TEL. 919-451-2093
rperera@aventoncompanies.com

AS-SURVEYED LEGAL DESCRIPTION

ALL THAT TRACT OR PARCEL OF LAND LYING AND
BEING IN THE 5TH DISTRICT, LAND LOTS 100, 125 AND
124 OF GWINNETT COUNTY, GEORGIA AND BEING
MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT A POINT ON THE NORTHWESTERLY SIDE OF U.S. HIGHWAY NO. 78, SAID POINT BEING 752 FEET SOUTHEASTERLY FROM THE INTERSECTION OF U.S. HIGHWAY 78 AND ROSEBUD ROAD, AS MEASURED ALONG THE NORTHWESTERLY SIDE OF U.S. HIGHWAY NO. 78, RUNNING THENCE NORTH 16°39' EAST, 307.7 FEET TO A 1/4" REBAR FOUND; RUNNING THENCE NORTH 30°38' 51" WEST, 800.30 FEET TO A 1/4" REBAR WITH CAP ("EARTHPO 0994") FOUND; RUNNING THENCE NORTH 59°23' 24" EAST, 1468.26 FEET TO A PAINTED ROCK FOUND; RUNNING THENCE SOUTH 30°02'00" EAST, 1739.48 FEET TO A 1" SQUARE SPIKE FOUND; THENCE SOUTH 58°31'00" WEST, 1062.00 FEET TO A MAGNETIC NAIL SET, SAID POINT BEING 45 FEET NORTHEAST OF THE MID-LINE OF U.S. HIGHWAY NO. 78 AND BEING LOCATED ON THE NORTHEASTERLY SIDE OF SAID ROADWAY; RUNNING THENCE NORTH 70°03' 55" WEST, 966.75 FEET, AS MEASURED ALONG THE NORTHERLY SIDE OF U.S. HIGHWAY NO. 78, TO AN IRON PIN SET AND THE POINT OF BEGINNING.

SAID TRACT OF LAND CONTAINING 57.7557 ACRES
OR 2,515,839.02 SQUARE FEET.

PROHIBITED USES:

No portion of the C-2 zoned land shall be utilized for a dry cleaning plant, coin laundry, billboards, drug-related paraphernalia shop, pool room, adult entertainment facility, massage parlor (except to the extent included as part of a day spa), adult book or video store, funeral parlor, flea market, bingo parlor; gaming, arcade, gambling, betting or game of chance business (exclusive of the sale of lottery tickets); sale, rental or lease of automobiles, trucks, other motorized vehicles or trailers; auto retail or repair business, gun or weapon shop, pawn shop, medical and/or recreational marijuana dispensing facility, plant nursery (but a florist or other establishment selling cut flowers or potted plants shall be allowed), or a tattoo parlor.

GEORGIA POWER COMPANY
RIGHT-OF-WAY EASEMENT
PER D.B. 401, PG. 34

NOW OR FORMERLY
2970 ROSEBUD PROPERTIES, LLC
D.B. 56168, PG. 10
PARCEL ID: R5100 115
ZONED: C2

NOW OR FORMERLY
GEORGIA TRANSMISSION CORP.
D.B. 13924, PG. 13
PARCEL ID: R5100 020
ZONED: R100

EXISTING SIGNALIZED INTERSECTION
AT ATHENS HIGHWAY (HWY 78) AND
ROSEBUD ROAD 811' TO THE WEST
FROM THE CENTERLINE OF
"DRIVEWAY A"



0' LANDSCAPE STRIP

GEORGIA POWER COMPANY
EASEMENT PER
D.B. 1079, PG. 111


COMMERCIAL AREA: 3.34 AC
PROPOSED ZONING: C-2
FAR: 0.15
NOTE: COMMERCIAL LAYOUT
IS CONCEPTUAL. FINAL
PROGRAMMING SUBJECT TO
CHANGE

NOW OR FORMERLY
LANGLEY FARMS HOMEOWNERS
ASSOCIATION, INC.
D.B. 47009, PG. 167
PARCEL ID: R5124 234

NOW OR FORMERLY
FELTON PROPERTIES, LL
D.B. 59518, PG. 503
PARCEL ID: R5124 013
ZONED: RA200

TRAFFIC ENGINEER:
KIMLEY-HORN & ASSOCIATES, INC.
817 WEST PEACHTREE STREET
THE BILMORE, SUITE 601 ATLANTA, GA 30308
PHONE: (404) 419-8700
CONTACT: MATT FLYNN, P.E.

NOTE: ALL PROPOSED
MULTIFAMILY BUILDINGS
SHOWN ON PLAN SHALL
BE 4 STORIES

| Zoning Dimensional Standards | | |
|------------------------------|--|-------|
| ZONING DISTRICT | RM-13 | C-2 |
| MIN. LOT SIZE | 18,000 SF | NONE |
| MIN. LOT WIDTH | 100 FT | NONE |
| MIN. FRONT SETBACK | 15 FT | 15 FT |
| MIN. SIDE SETBACK | 10 FT | 10 FT |
| MIN. REAR SETBACK | 30 FT | 30 FT |
| MIN. HEATED FLOOR AREA | 750 SF | -- |
| MAXIMUM 3 BEDROOMS OR LARGER | PHASE 1: 36 UNITS PHASE 2: 29 UNITS | -- |
| MIN. % COM. AREA | 15% | N/A |
| MAX. NET DENSITY | 13 UNITS PER ACRE | NONE |
| MAX. BLDG. HEIGHT | 50 FT | 45 FT |
| PROPOSED BLDG. HEIGHT | 49'-6"  | -- |

| RM-13 PARKING REQUIREMENTS/PROPOSAL | | | | |
|-------------------------------------|----------------|-----------------------|----------------------|------------------|
| PHASE | PROPOSED UNITS | MIN. (1.5 X UNITS) | MAX. (3 X UNITS) | PROPOSED PARKING |
| 1 | 368 | 552 | 1,104 | 623 |
| 2 | 294 | 441 | 882 | 492 |
| C-2 PARKING REQUIREMENTS/PROPOSAL | | | | |
| USE/DEVELOPMENT | AREA | MIN. | MAX. | PROPOSED |
| RESTAURANT | 5,400 SF | 1 PER 150 SF = 36 PL | 1 PER 75 SF = 72 PL | 36 |
| OFFICE/RETAIL | 16,700 SF | 1 PER 500 SF = 33 PL | 1 PER 200 SF = 84 PL | 78 |
| TOTAL PARKING | | 69 PL | 156 PL | 114 PL |

REZONING SITE PLAN

DRI #3437

GWINNETT COUNTY, GEORGIA

PREPARED FOR:

AVENTON COMPANIES

PREPARED BY:

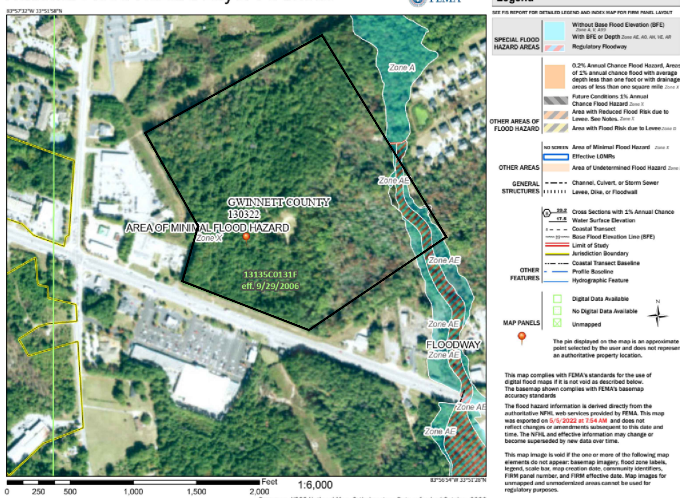


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|--------|-----------|
| DATE: | 08/18/22 |
| SCALE: | 1" = 100' |
| SHEET: | RZ-1 |

National Flood Hazard Layer FIRMette



ALTERNATE PARK LAYOUT

TRAFFIC ENGINEER:
KIMLEY-HORN & ASSOCIATES, INC.
817 WEST PEACHTREE STREET
THE BILMORE, SUITE 601 ATLANTA, GA 30308
PHONE: (404) 419-8700
CONTACT: MATT FLYNN, P.E.

NOTE: ALL PROPOSED
MULTIFAMILY BUILDINGS
SHOWN ON PLAN SHALL
BE 4 STORIES

SITE DATA

SITEAthens Highway SW
PARCEL NUMBERS.....R5125-005
TOTAL SITE AREA.....57.7557 ACRES
ZONING:
CURRENT ZONING.....R-100
PROPOSED ZONING.....RM-13, C-2
ZONING JURISDICTION.....GWINNETT COUNTY, GA

GRAPHIC SCALE



© 2022 Microsoft Corporation © 2022 Maxar © CNES (2022) (IN FEET)
1 inch = 100 ft.

Trip Generation Analysis

DRAFT

| Trip Generation Analysis (10th Ed. With 2nd Edition Handbook: Daily IC & 3rd Edition AM/PM IC) Aventon Park DRI Gwinnett Co. | | | | | | | | | | | | | |
|--|-------------------------------------|------------------------|-------------|----------------|-------|--------------|-------|-----|--------------|-----|-----|-----|-----|
| Land Use | Setting | Density | Daily Trips | | | AM Peak Hour | | | PM Peak Hour | | | | |
| | | | Total | In | Out | Total | In | Out | Total | In | Out | | |
| Proposed Project Trips | | | | | | | | | | | | | |
| 221 | Multifamily Housing (Mid-Rise) | General Urban/Suburban | 662 | dwelling units | 3,112 | 1,556 | 1,556 | 280 | 64 | 216 | 259 | 158 | 101 |
| 712 | Small Office Building | General Urban/Suburban | 7,100 | Sq. Ft. GFA | 102 | 51 | 51 | 12 | 10 | 2 | 15 | 5 | 10 |
| 822 | Strip Retail Plaza (<40k) | General Urban/Suburban | 9,600 | Sq. Ft. GFA | 636 | 318 | 318 | 28 | 17 | 11 | 76 | 38 | 38 |
| 932 | High-Turnover (Sit-Down) Restaurant | General Urban/Suburban | 5,400 | Sq. Ft. GFA | 580 | 290 | 290 | 52 | 28 | 24 | 49 | 30 | 19 |
| Gross Project Trips | | | | | 4,430 | 2,215 | 2,215 | 372 | 119 | 253 | 399 | 231 | 168 |
| Residential Trips | | | | | 3,112 | 1,556 | 1,556 | 280 | 64 | 216 | 259 | 158 | 101 |
| Mixed-Use Reductions | | | | | -123 | -62 | -61 | -10 | -2 | -8 | -24 | -13 | -11 |
| Alternative Mode Reductions | | | | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Adjusted Residential Trips | | | | | 2,989 | 1,494 | 1,495 | 270 | 62 | 208 | 235 | 145 | 90 |
| Office Trips | | | | | 102 | 51 | 51 | 12 | 10 | 2 | 15 | 5 | 10 |
| Mixed-Use Reductions | | | | | -22 | -11 | -11 | -3 | -1 | -2 | -7 | -5 | -2 |
| Alternative Mode Reductions | | | | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Adjusted Office Trips | | | | | 80 | 40 | 40 | 9 | 9 | 0 | 8 | 0 | 8 |
| Retail Trips | | | | | 636 | 318 | 318 | 28 | 17 | 11 | 76 | 38 | 38 |
| Mixed-Use Reductions | | | | | -74 | -37 | -37 | -6 | -4 | -2 | -34 | -14 | -20 |
| Alternative Mode Reductions | | | | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass By Reductions (Based on ITE Rates) | | | | | -191 | -96 | -95 | 0 | 0 | 0 | -14 | -7 | -7 |
| Adjusted Retail Trips | | | | | 371 | 185 | 186 | 22 | 13 | 9 | 28 | 17 | 11 |
| Restaurant Trips | | | | | 580 | 290 | 290 | 52 | 28 | 24 | 49 | 30 | 19 |
| Mixed-Use Reductions | | | | | -67 | -34 | -33 | -11 | -8 | -3 | -25 | -13 | -12 |
| Alternative Mode Reductions | | | | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass By Reductions (Based on ITE Rates) | | | | | -221 | -111 | -110 | 0 | 0 | 0 | -10 | -5 | -5 |
| Adjusted Restaurant Trips | | | | | 292 | 145 | 147 | 41 | 20 | 21 | 14 | 12 | 2 |
| Mixed-Use Reductions - TOTAL | | | | | -286 | -144 | -142 | -30 | -15 | -15 | -90 | -45 | -45 |
| Alternative Mode Reductions - TOTAL | | | | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-By Reductions - TOTAL | | | | | -412 | -207 | -205 | 0 | 0 | 0 | -24 | -12 | -12 |
| New Trips | | | | | 3,732 | 1,864 | 1,868 | 342 | 104 | 238 | 285 | 174 | 111 |
| Driveway Volumes | | | | | 4,144 | 2,071 | 2,073 | 342 | 104 | 238 | 309 | 186 | 123 |

Intersection Volume Worksheets

DRAFT

GA-10 Athens Hwy (West) at Cooper Spring Rd/Cooper Rd

| | Cooper Spring Rd | | | | Cooper Rd | | | | GA-10 Athens Hwy (West) | | | | GA-10 Athens Hwy (West) | | | |
|-------------------------------|------------------|------|---------|-------|------------|------|---------|-------|-------------------------|------|---------|-------|-------------------------|------|---------|-------|
| | Northbound | | | | Southbound | | | | Eastbound | | | | Westbound | | | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2022 Traffic Volumes | 0 | 18 | 14 | 8 | 0 | 131 | 18 | 305 | 0 | 181 | 679 | 13 | 0 | 13 | 1,470 | 103 |
| Pedestrians | 0 | 0 | | | 0 | 0 | | | 0 | 0 | | | 0 | 0 | | |
| Conflicting Pedestrians | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | 0 | 0 | | | 0 | 0 | | | 0 | 0 | | | 0 | 0 | | |
| Heavy Vehicles | 0 | 0 | 2 | 1 | 0 | 6 | 0 | 7 | 0 | 10 | 30 | 2 | 0 | 0 | 51 | 7 |
| Heavy Vehicle % | 2% | 2% | 14% | 13% | 2% | 5% | 2% | 2% | 2% | 6% | 4% | 15% | 2% | 2% | 3% | 7% |
| Peak Hour Factor | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 | 0.96 |
| Adjustment Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Calibrated 2022 Volumes | 0 | 20 | 16 | 9 | 0 | 145 | 20 | 339 | 0 | 201 | 754 | 14 | 0 | 14 | 1,632 | 114 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 |
| 2026 No-Build Traffic | 0 | 21 | 17 | 10 | 0 | 154 | 21 | 360 | 0 | 213 | 800 | 15 | 0 | 15 | 1,732 | 121 |
| Trip Distribution IN | | | | | | | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | | (50%) | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 31 | 0 | 0 | 0 | 104 | 0 |
| Trip Distribution IN | | | | | | | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | | (50%) | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 5 | 0 |
| Trip Distribution IN | | | | | | | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | | (50%) | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 11 | 0 |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 53 | 0 | 0 | 0 | 120 | 0 |
| Pass-By Distribution IN | | | | | | | | | | | | | | | | |
| Pass-By Distribution OUT | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 53 | 0 | 0 | 0 | 120 | 0 |
| 2026 Build Traffic | 0 | 21 | 17 | 10 | 0 | 154 | 21 | 360 | 0 | 213 | 853 | 15 | 0 | 15 | 1,852 | 121 |
| 2026 Build Heavy Vehicle % | 2% | 2% | 14% | 13% | 2% | 5% | 2% | 2% | 2% | 6% | 4% | 15% | 2% | 2% | 3% | 7% |

[illegible]

INTERSECTION VOLUME DEVELOPMENT
INTERSECTION #2
GA-10 Athens Hwy (West) at Rosebud Rd

AM PEAK HOUR

| | Rosebud Rd Northbound | | | | Rosebud Rd Southbound | | | | GA-10 Athens Hwy (West) Eastbound | | | | GA-10 Athens Hwy (West) Westbound | | | |
|-------------------------------|--------------------------|------|---------|-------|--------------------------|------|---------|-------|--------------------------------------|------|---------|-------|--------------------------------------|------|---------|-------|
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2022 Traffic Volumes | 0 | 178 | 418 | 102 | 0 | 118 | 309 | 46 | 0 | 15 | 660 | 107 | 0 | 88 | 1,391 | 190 |
| Pedestrians | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Conflicting Pedestrians | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Heavy Vehicles | 0 | 10 | 8 | 2 | 0 | 4 | 10 | 4 | 0 | 1 | 20 | 12 | 0 | 1 | 45 | 17 |
| Heavy Vehicle % | 2% | 6% | 2% | 2% | 2% | 3% | 3% | 9% | 2% | 7% | 3% | 11% | 2% | 2% | 3% | 9% |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Adjustment Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Calibrated 2022 Volumes | 0 | 198 | 464 | 113 | 0 | 131 | 343 | 51 | 0 | 17 | 733 | 119 | 0 | 98 | 1,544 | 211 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 |
| 2026 No-Build Traffic | 0 | 210 | 492 | 120 | 0 | 139 | 364 | 54 | 0 | 18 | 778 | 126 | 0 | 104 | 1,639 | 224 |
| Trip Distribution IN | | | | 5% | | 20% | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (5%) | (50%) | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 3 | 0 | 12 | 0 | 0 | 0 | 0 | 31 | 0 | 0 | 10 | 104 | 42 |
| Trip Distribution IN | | | | 5% | | 20% | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (5%) | (50%) | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 5 | 2 |
| Trip Distribution IN | | | | 5% | | 20% | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (5%) | (50%) | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 1 | 11 | 4 |
| Total Primary Site Trips | 0 | 0 | 0 | 5 | 0 | 21 | 0 | 0 | 0 | 0 | 53 | 0 | 0 | 11 | 120 | 48 |
| Pass-By Distribution IN | | | | | | | | | | | | | | | | |
| Pass-By Distribution OUT | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 0 | 0 | 0 | 5 | 0 | 21 | 0 | 0 | 0 | 0 | 53 | 0 | 0 | 11 | 120 | 48 |
| 2026 Build Traffic | 0 | 210 | 492 | 125 | 0 | 160 | 364 | 54 | 0 | 18 | 831 | 126 | 0 | 115 | 1,759 | 272 |
| 2026 Build Heavy Vehicle % | 2% | 6% | 2% | 2% | 2% | 3% | 3% | 9% | 2% | 7% | 3% | 11% | 2% | 2% | 3% | 9% |

PM PEAK HOUR

| | Rosebud Rd Northbound | | | | Rosebud Rd Southbound | | | | GA-10 Athens Hwy (West) Eastbound | | | | GA-10 Athens Hwy (West) Westbound | | | |
|-------------------------------|--------------------------|------|---------|-------|--------------------------|------|---------|-------|--------------------------------------|------|---------|-------|--------------------------------------|------|---------|-------|
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2022 Traffic Volumes | 0 | 136 | 336 | 102 | 0 | 162 | 357 | 46 | 0 | 48 | 1,346 | 156 | 0 | 130 | 1,023 | 177 |
| Pedestrians | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Conflicting Pedestrians | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Heavy Vehicles | 0 | 2 | 4 | 2 | 0 | 0 | 11 | 0 | 0 | 4 | 29 | 2 | 0 | 3 | 29 | 8 |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 8% | 2% | 2% | 2% | 2% | 3% | 5% |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Calibrated 2022 Volumes | 0 | 136 | 336 | 102 | 0 | 162 | 357 | 46 | 0 | 48 | 1,346 | 156 | 0 | 130 | 1,023 | 177 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 |
| 2026 No-Build Traffic | 0 | 144 | 357 | 108 | 0 | 172 | 379 | 49 | 0 | 51 | 1,429 | 166 | 0 | 138 | 1,086 | 188 |
| 2026 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | | | | 5% | | 20% | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (5%) | (50%) | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 7 | 0 | 29 | 0 | 0 | 0 | 0 | 73 | 0 | 0 | 5 | 45 | 18 |
| Trip Distribution IN | | | | 5% | | 20% | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (5%) | (50%) | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 0 | 9 | 0 | 0 | 1 | 6 | 2 |
| Trip Distribution IN | | | | 5% | | 20% | | | | | 50% | | | | | |
| Trip Distribution OUT | | | | | | | | | | | | | | (5%) | (50%) | (20%) |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 1 | 0 |
| Total Primary Site Trips | 0 | 0 | 0 | 9 | 0 | 34 | 0 | 0 | 0 | 0 | 88 | 0 | 0 | 6 | 56 | 22 |
| Pass-By Distribution IN | | | | | | | | | | | | | | | | |
| Pass-By Distribution OUT | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 0 | 0 | 0 | 9 | 0 | 34 | 0 | 0 | 0 | 0 | 88 | 0 | 0 | 6 | 56 | 22 |
| 2026 Build Traffic | 0 | 144 | 357 | 117 | 0 | 206 | 379 | 49 | 0 | 51 | 1,517 | 166 | 0 | 144 | 1,142 | 210 |
| 2026 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 2% | 2% | 8% | 2% | 2% | 2% | 2% | 3% | 5% |

GA-10 Athens Hwy (West) at Midway Plaza Dwy/Dwy B

AM PEAK HOUR

| | Midway Plaza Dwy | | | | Dwy 8 | | | | GA-10 Athens Hwy (West) | | | | GA-10 Athens Hwy (West) | | | | |
|-------------------------------|------------------|------|---------|-------|------------|-------|---------|-------|-------------------------|------|---------|-------|-------------------------|------|---------|-------|------|
| | Northbound | | | | Southbound | | | | Eastbound | | | | Westbound | | | | |
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | |
| Observed 2022 Traffic Volumes | 0 | 23 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 820 | 34 | 0 | 15 | 1,712 | 0 |
| Pedestrians | | 0 | | | | | 0 | | | | 0 | | | | 0 | | |
| Conflicting Pedestrians | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3% | 0 | 0 | 2% | 2% | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | | 0 | | | | | 0 | | | | 0 | | | | 0 | | |
| Heavy Vehicles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 26 | 1 | 0 | 0 | 65 | 0 |
| Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 3% | 2% | 2% | 4% | 2% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adjustment Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Calibrated 2022 Volumes | 0 | 26 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 910 | 38 | 0 | 17 | 1,900 | 0 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 |
| 2026 No-Build Traffic | 0 | 28 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 966 | 40 | 0 | 18 | 2,017 | 0 |
| Trip Distribution IN | | | | | | | | | | | 25% | 50% | | | | | 15% |
| Trip Distribution OUT | | | | | | (15%) | | (25%) | | | | | | | | (50%) | |
| Balancing Adjustment | | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 31 | 0 | 52 | 0 | 16 | 31 | 0 | 0 | 0 | 0 | 104 | 9 |
| Trip Distribution IN | | | | | | | | | | | 25% | 50% | | | | | 15% |
| Trip Distribution OUT | | | | | | (15%) | | (25%) | | | | | | | | (50%) | |
| Balancing Adjustment | | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 2 | 0 | 3 | 7 | 0 | 0 | 0 | 0 | 5 | 2 |
| Trip Distribution IN | | | | | | | | | | | 25% | 50% | | | | | 15% |
| Trip Distribution OUT | | | | | | (15%) | | (25%) | | | 25% | | | | | (50%) | |
| Balancing Adjustment | | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 5 | 0 | 5 | 10 | 0 | 0 | 0 | 0 | 11 | 3 |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 35 | 0 | 59 | 0 | 26 | 53 | 0 | 0 | 0 | 0 | 120 | 15 |
| Pass-By Distribution IN | | | | | | | | | | | 25% | | | | | | 15% |
| Pass-By Distribution OUT | | | | | | (25%) | | (15%) | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 0 | 0 | 0 | 0 | 0 | 35 | 0 | 59 | 0 | 26 | 53 | 0 | 0 | 0 | 0 | 120 | 15 |
| 2026 Build Traffic | 0 | 28 | 0 | 12 | 0 | 35 | 0 | 59 | 0 | 26 | 1,019 | 40 | 0 | 18 | 2,137 | 15 | |
| 2026 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 3% | 3% | 2% | 2% | 2% | 4% | 2% |

PM PEAK HOUR

[illegible]

INTERSECTION VOLUME DEVELOPMENT
INTERSECTION #4
GA-10 Athens Hwy (West) at Langley Rd

AM PEAK HOUR

| | Langley Rd Northbound | | | | Langley Rd Southbound | | | | GA-10 Athens Hwy (West) Eastbound | | | | GA-10 Athens Hwy (West) Westbound | | | |
|-------------------------------|-----------------------|------|---------|-------|-----------------------|------|---------|-------|-----------------------------------|------|---------|-------|-----------------------------------|------|---------|-------|
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2022 Traffic Volumes | 0 | 35 | 1 | 19 | 0 | 25 | 3 | 90 | 0 | 26 | 756 | 24 | 1 | 10 | 1,615 | 19 |
| Pedestrians | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Conflicting Pedestrians | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 24 | 5 | 0 | 0 | 67 | 3 |
| Heavy Vehicles | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 4% | 2% | 2% | 3% | 21% | 2% | 2% | 4% | 16% |
| Heavy Vehicle % | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Peak Hour Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Adjustment Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Calibrated 2022 Volumes | 0 | 39 | 1 | 21 | 0 | 28 | 3 | 100 | 0 | 29 | 839 | 27 | 1 | 11 | 1,793 | 21 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 |
| 2026 No-Build Traffic | 0 | 41 | 1 | 22 | 0 | 30 | 3 | 106 | 0 | 31 | 890 | 29 | 1 | 12 | 1,903 | 22 |
| Trip Distribution IN | | | | | | | | | | | | | | | | 25% |
| Trip Distribution OUT | | | | | | | | | | | (25%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 52 | 0 | 0 | 0 | 16 | 0 |
| Trip Distribution IN | | | | | | | | | | | | | | | | 25% |
| Trip Distribution OUT | | | | | | | | | | | (25%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 0 |
| Trip Distribution IN | | | | | | | | | | | | | | | | 25% |
| Trip Distribution OUT | | | | | | | | | | | (25%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 5 | 0 |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 59 | 0 | 0 | 0 | 26 | 0 |
| Pass-By Distribution IN | | | | | | | | | | | | | | | | |
| Pass-By Distribution OUT | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 59 | 0 | 0 | 0 | 26 | 0 |
| 2026 Build Traffic | 0 | 41 | 1 | 22 | 0 | 30 | 3 | 106 | 0 | 31 | 949 | 29 | 1 | 12 | 1,929 | 22 |
| 2026 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 4% | 2% | 2% | 3% | 21% | 2% | 2% | 4% | 16% |

PM PEAK HOUR

| | Langley Rd Northbound | | | | Langley Rd Southbound | | | | GA-10 Athens Hwy (West) Eastbound | | | | GA-10 Athens Hwy (West) Westbound | | | |
|-------------------------------|-----------------------|------|---------|-------|-----------------------|------|---------|-------|-----------------------------------|------|---------|-------|-----------------------------------|------|---------|-------|
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2022 Traffic Volumes | 0 | 25 | 3 | 22 | 0 | 52 | 2 | 82 | 0 | 59 | 1,472 | 54 | 0 | 33 | 1,263 | 87 |
| Pedestrians | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Conflicting Pedestrians | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Heavy Vehicles | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 4% | 2% | 5% | 2% | 17% | 2% | 2% | 3% | 2% |
| Heavy Vehicle % | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Peak Hour Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Calibrated 2022 Volumes | 0 | 25 | 3 | 22 | 0 | 52 | 2 | 82 | 0 | 59 | 1,472 | 54 | 0 | 33 | 1,263 | 87 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 |
| 2026 No-Build Traffic | 0 | 27 | 3 | 23 | 0 | 55 | 2 | 87 | 0 | 63 | 1,562 | 57 | 0 | 35 | 1,341 | 92 |
| 2026 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | | | | | | | | | | | | | | | | 25% |
| Trip Distribution OUT | | | | | | | | | | | (25%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 0 | 36 | 0 |
| Trip Distribution IN | | | | | | | | | | | | | | | | 25% |
| Trip Distribution OUT | | | | | | | | | | | (25%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 4 | 0 |
| Trip Distribution IN | | | | | | | | | | | | | | | | 25% |
| Trip Distribution OUT | | | | | | | | | | | (25%) | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 3 | 0 |
| Total Primary Site Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 29 | 0 | 0 | 0 | 43 | 0 |
| Pass-By Distribution IN | | | | | | | | | | | | | | | | |
| Pass-By Distribution OUT | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 29 | 0 | 0 | 0 | 43 | 0 |
| 2026 Build Traffic | 0 | 27 | 3 | 23 | 0 | 55 | 2 | 87 | 0 | 63 | 1,591 | 57 | 0 | 35 | 1,384 | 92 |
| 2026 Build Heavy Vehicle % | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 4% | 2% | 5% | 2% | 17% | 2% | 2% | 3% | 2% |

INTERSECTION VOLUME DEVELOPMENT

INTERSECTION #5
Cooper Rd at Rosebud Rd

AM PEAK HOUR

| | Rosebud Rd Northbound | | | | Rosebud Rd Southbound | | | | Cooper Rd Eastbound | | | | Cooper Rd Westbound | | | |
|-------------------------------|-----------------------|------|---------|-------|-----------------------|------|---------|-------|---------------------|------|---------|-------|---------------------|------|---------|-------|
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2022 Traffic Volumes | 0 | 65 | 505 | 75 | 0 | 69 | 305 | 117 | 0 | 136 | 189 | 30 | 0 | 101 | 377 | 139 |
| Pedestrians | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Conflicting Pedestrians | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Heavy Vehicles | 0 | 4 | 19 | 2 | 0 | 2 | 14 | 5 | 0 | 15 | 14 | 1 | 0 | 3 | 20 | 5 |
| Heavy Vehicle % | 2% | 6% | 4% | 3% | 2% | 3% | 5% | 4% | 2% | 11% | 7% | 3% | 2% | 3% | 5% | 4% |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Adjustment Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Calibrated 2022 Volumes | 0 | 72 | 561 | 83 | 0 | 77 | 339 | 130 | 0 | 151 | 210 | 33 | 0 | 112 | 418 | 154 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 |
| 2026 No-Build Traffic | 0 | 76 | 595 | 88 | 0 | 82 | 360 | 138 | 0 | 160 | 223 | 35 | 0 | 119 | 444 | 163 |
| Trip Distribution IN | | | | | | | 20% | | | | | | | | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 42 | 0 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | | | | | | | 20% | | | | | | | | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | | | | | | | 20% | | | | | | | | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Primary Site Trips | 0 | 0 | 48 | 0 | 0 | 0 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-By Distribution IN | | | | | | | | | | | | | | | | |
| Pass-By Distribution OUT | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 0 | 0 | 48 | 0 | 0 | 0 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2026 Build Traffic | 0 | 76 | 643 | 88 | 0 | 82 | 381 | 138 | 0 | 160 | 223 | 35 | 0 | 119 | 444 | 163 |
| 2026 Build Heavy Vehicle % | 2% | 6% | 4% | 3% | 2% | 3% | 5% | 4% | 2% | 11% | 7% | 3% | 2% | 3% | 5% | 4% |

PM PEAK HOUR

| | Rosebud Rd Northbound | | | | Rosebud Rd Southbound | | | | Cooper Rd Eastbound | | | | Cooper Rd Westbound | | | |
|-------------------------------|-----------------------|------|---------|-------|-----------------------|------|---------|-------|---------------------|------|---------|-------|---------------------|------|---------|-------|
| | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right | U-Turn | Left | Through | Right |
| Observed 2022 Traffic Volumes | 0 | 24 | 417 | 152 | 0 | 130 | 389 | 131 | 0 | 119 | 402 | 24 | 0 | 149 | 268 | 147 |
| Pedestrians | 0 | | | | 0 | | | | 0 | | | | 0 | | | |
| Conflicting Pedestrians | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Bicycles | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Heavy Vehicles | 0 | 0 | 17 | 0 | 0 | 2 | 10 | 4 | 0 | 2 | 6 | 0 | 0 | 1 | 4 | 0 |
| Heavy Vehicle % | 2% | 2% | 4% | 2% | 2% | 2% | 3% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% |
| Peak Hour Factor | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 | 0.93 |
| Adjustment Factor | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Calibrated 2022 Volumes | 0 | 24 | 417 | 152 | 0 | 130 | 389 | 131 | 0 | 119 | 402 | 24 | 0 | 149 | 268 | 147 |
| Annual Growth Rate | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% | 1.5% |
| Growth Factor | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 | 1.06 |
| 2026 No-Build Traffic | 0 | 25 | 443 | 161 | 0 | 138 | 413 | 139 | 0 | 126 | 427 | 25 | 0 | 158 | 284 | 156 |
| 2026 No-Build Heavy Vehicle % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | | | | | | | 20% | | | | | | | | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Residential Trips | 0 | 0 | 18 | 0 | 0 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | | | | | | | 20% | | | | | | | | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Retail Trips | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Trip Distribution IN | | | | | | | 20% | | | | | | | | | |
| Trip Distribution OUT | | | (20%) | | | | | | | | | | | | | |
| Balancing Adjustment | | | | | | | | | | | | | | | | |
| Restaurant Trips | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Primary Site Trips | 0 | 0 | 22 | 0 | 0 | 0 | 34 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Pass-By Distribution IN | | | | | | | | | | | | | | | | |
| Pass-By Distribution OUT | | | | | | | | | | | | | | | | |
| Pass-By Trips | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Vehicular Project Trips | 0 | 0 | 22 | 0 | 0 | 0 | 34 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 2026 Build Traffic | 0 | 25 | 465 | 161 | 0 | 138 | 447 | 139 | 0 | 126 | 427 | 25 | 0 | 158 | 284 | 156 |
| 2026 Build Heavy Vehicle % | 2% | 2% | 4% | 2% | 2% | 2% | 3% | 3% | 2% | 2% | 2% | 2% | 2% | 2% | 2% | 2% |

INTERSECTION #6
GA-10 Athens Hwy (West) at Dwy A

[illegible]

GDOT Intersection Control Evaluation (ICE)

DRAFT

| | | | | | | | | | |
|--|--|--|-----|-----|-----|-----|-----|-----|---|
| GDOT PI # | N/A | <p>Note: Up to 5 alternatives may be selected and evaluated; Use this ICE Stage 1 to screen 5 or fewer alternatives to evaluate in Stage 2</p> <p>1. Does alternative address the project need in a balanced manner and in scale with the project? 2. Does alternative improve safety performance in terms of reducing severe crashes? 3. Does alternative incorporate safety, convenience and accessibility for pedestrians and/or bicyclists? 4. Does alternative improve (or preserve) traffic operations (congestion, delay, reliability, etc.)? 5. Does alternative appear feasible given the site characteristics, constraints & location context? 6. Does alternative appear feasible with respect to other project factors? 7. Overall feasible alternative (select alternative for further evaluation in Stage 2)?</p> <p>Screening Decision Justification:</p> | | | | | | | |
| Project Location: | SR10 Athens Hwy @ Dwy A | | | | | | | | |
| Existing Control: | Conventional (Minor Stop) | | | | | | | | |
| Prepared by: | Kimley-Horn | | | | | | | | |
| Date: | 8/16/2022 | | | | | | | | |
| <p>Answer "Yes" or "No" to each policy question for each control type to identify which alternatives should be evaluated in the Stage 2 Decision Record; enter justification in the rightmost column</p> | | | | | | | | | |
| <p>Intersection Alternative (see "Intersections" tab for detailed description of intersection/interchange type)</p> | | | | | | | | | |
| Unsignalized Intersections | Conventional (Minor Stop) | Yes | No | No | Yes | Yes | Yes | Yes | See Stage 2 |
| | Conventional (All-Way Stop) | No | Yes | Yes | No | No | No | No | Major street ADT too high |
| | Mini Roundabout | No | Yes | Yes | Yes | No | No | No | Major street ADT too high |
| | Single Lane Roundabout | No | Yes | Yes | No | No | No | No | Major street ADT too high |
| | Multilane Roundabout | No | Yes | Yes | No | No | No | No | Geometry does not allow for this, not in line with project purpose. |
| | RCUT (stop control) | No | Yes | No | No | Yes | No | Yes | See stage 2 |
| | RIRO w/down stream U-Turn | No | Yes | No | No | Yes | Yes | Yes | See stage 2 |
| | High-T (unsignalized) | No | No | No | No | No | No | No | Not a T-intersection |
| | Offset-T Intersections | No | Yes | No | No | No | No | No | Geometry does not allow for this, not in line with project purpose. |
| | Diamond Interch (Stop Control) | No | No | No | No | No | No | No | Not an interchange |
| | Diamond Interch (RAB Control) | No | No | No | No | No | No | No | Not an interchange |
| | Add LT Lanes on SR 22 No RT Lane Improvements | Yes | No | No | No | Yes | Yes | No | |
| | Other unsignalized (provide description): | No | No | No | No | No | No | No | N/A |
| | Signalized Intersections | Traffic Signal | No | Yes | Yes | No | No | No | No |
| Median U-Turn (Indirect Left) | | No | No | No | Yes | No | No | No | Not expected to satisfy signal warrants based only on mainline lefts. |
| RCUT (signalized) | | No | No | No | No | No | No | No | Not expected to satisfy signal warrants based only on mainline lefts. |
| Displaced Left Turn (CFI) | | No | No | No | Yes | No | No | No | Right-of-way not available, not in line with project purpose. |
| Continuous Green-T | | No | No | No | No | No | No | No | Not a T-intersection |
| Jughandle | | No | No | No | Yes | No | No | No | Right-of-way not available, not in line with project purpose. |
| Quadrant Roadway | | No | No | No | No | No | No | No | Right-of-way not available, not in line with project purpose. |
| Diamond Interch (Signal Control) | | No | No | No | No | No | No | No | Not an interchange |
| Diverging Diamond | | No | No | No | No | No | No | No | Not an interchange |
| Single Point Interchange | | No | No | No | No | No | No | No | Not an interchange |
| No LT Lane Improvements No RT Lane Improvements | | No | No | No | No | No | No | No | N/A |
| Other Signalized (provide description): | | No | No | No | No | No | No | No | N/A |

☒ = Intersection type selected for more detailed analysis in Stage 2 Alternative Selection Decision Record

Project Location: SR10 Athens Hwy @ Dwy A
 Existing Intersection Control: Conventional (Minor Stop)
 Type of Analysis: **Conventional Non-Safety Funded Project**

District: 1 - Gainesville
 County: Gwinnett
 Area: Suburb/Transit
 GDOT PI #: N/A
 Prepared by: Kimley-Horn
 Date: 8/16/2022

Opening / Design Year Traffic Operations

| | | | |
|---|--------------------|------------|--|
| Intersection meets signal/AWS warrants? | None | | Complete Streets Warrants Met? <input type="checkbox"/> PEDESTRIANS <input type="checkbox"/> BICYCLES <input type="checkbox"/> TRANSIT |
| Traffic Analysis Measure of Effectiveness | Intersection Delay | | |
| Traffic Analysis Software Used | Synchro | | |
| Analysis Time Period | AM Peak Hr | PM Peak Hr | |
| 2026 Opening Yr No-Build Peak Hr Intersection Delay | 0.0 sec | 0.0 sec | |
| 2026 Opening Yr No-Build Peak Hr Intersection V/C | 0.00 | 0.00 | |
| 2026 Design Yr No-Build Peak Hr Intersection Delay | 0.0 sec | 0.0 sec | |
| 2026 Design Yr No-Build Peak Hr Intersection V/C | 0.00 | 0.00 | |

| Crash Type | Crash Severity | | | | | Years: |
|---------------------------|----------------|----|----|----|---|--------|
| | K* | A* | B* | C* | O | 5 |
| Angle | 0 | 0 | 0 | 0 | 0 | #### |
| Head-On | 0 | 0 | 0 | 0 | 0 | #### |
| Rear End | 0 | 0 | 0 | 0 | 0 | #### |
| Sideswipe - same | 0 | 0 | 0 | 0 | 0 | #### |
| Sideswipe - opposite | 0 | 0 | 0 | 0 | 0 | #### |
| Not Collision w/Motor Veh | 0 | 0 | 0 | 0 | 0 | #### |
| TOTALS: | 0 | 0 | 0 | 0 | 0 | 0 |

* Number of crashes resulting in injuries / fatalities, not number of persons

Alternatives Analysis:

Proposed Control Type/Improvement:

Project Cost: (From CostEst Worksheet)

| | Alternative 1 | Alternative 2 | Alternative 3 | Alternative 4 | Alternative 5 |
|---------------------------------------|-----------------------------|-----------------------------|-----------------------------|-----------------------------|-----------------------------|
| | Conventional (Minor Stop) | RCUT (stop control) | RIO w/down stream U-Turn | N/A | N/A |
| | Additional description here | Additional description here | Additional description here | Additional description here | Additional description here |
| Construction Cost | \$200,000 | \$655,000 | \$538,000 | | |
| ROW Cost | \$0 | \$406,000 | \$406,000 | | |
| Environmental Cost | \$0 | \$0 | \$0 | | |
| Reimbursable Utility Cost | \$0 | \$9,000 | \$7,000 | | |
| Design & Contingency Cost | \$0 | \$170,000 | \$139,000 | | |
| Cost Adjustment (justification req'd) | 0% | 0% | 0% | | |
| Total Cost | \$200,000 | \$1,240,000 | \$1,090,000 | | |

Traffic Operations:

| | | | | | | | | |
|---|------------|------------|------------|------------|------------|------------|--|--|
| | Synchro | | Synchro | | Synchro | | | |
| Traffic Analysis Software Used | AM Peak Hr | PM Peak Hr | AM Peak Hr | PM Peak Hr | AM Peak Hr | PM Peak Hr | | |
| Analysis Period | 204.9 sec | 26.7 sec | 63.1 sec | 29.7 sec | 404.0 sec | 157.7 sec | | |
| 2026 Design Yr Build Intersection Delay | 1.19 | 0.32 | 0.76 | 0.65 | 1.47 | 1.09 | | |
| 2026 Design Yr Build Intersection V/C | | | | | | | | |

Safety Analysis:

| | | | | | |
|---|---|-----------------|-----------------------------------|--|--|
| Predefined CRF: PDO | 0% | 31% | 35% | | |
| Predefined CRF: Fatal/Inj | 0% | 53% | 54% | | |
| Predefined CRF Source: | CRF unavailable; provide user defined CRF below | NC/MO Table 4-7 | FHWA Clearinghouse #s 5555 / 5556 | | |
| User Defined CRF: PDO | | | | | |
| User Defined CRF: Fatal/Inj | | | | | |
| User Defined CRF Source (write in if applicable): | | | | | |

Environmental Impacts:¹

| | | | | | |
|----------------------------|------|------|------|--|--|
| Historic District/Property | None | None | None | | |
| Archaeology Resources | None | None | None | | |
| Graveyard | None | None | None | | |
| Stream | None | None | None | | |
| Underground Tank/Hazmat | None | None | None | | |
| Park Land | None | None | None | | |
| EJ Community | None | None | None | | |
| Wooded Area | None | None | None | | |
| Wetland | None | None | None | | |

Stakeholder Posture:

| | | | | | |
|-------------------------|---------|---------|---------|--|--|
| Local Community Support | Unknown | Unknown | Unknown | | |
| GDOT Support | Unknown | Unknown | Unknown | | |

| | | | | | |
|---------------------------------------|--------------------------------------|------------|------------|--|--|
| Final ICE Stage 2 Score: | 3.8 | 2.6 | 1.7 | | |
| Rank of Control Type Alternatives: | 1 | 2 | 3 | | |
| Final Intersection Control Selection: | 1 - Conventional (Minor Stop) | | | | |

Note: Stage 2 score is not given (shown as "-") if signal or AWS is selected as control type but respective warrants are not met

Provide additional comments and/or explain any unique analysis inputs, or results (as necessary):

Synchro 11 used for analysis. RCUT/RIO delay and v/c includes weighted average of southbound movement and corresponding displaced u-turn and travel time (worst approach). Conventional minor stop with turn lane delay and v/c represents worst approach (southbound left). Cost override tool used to include cost of conventional driveway.

| | | | | | | | | | |
|--|--|--|-----|-----|-----|-----|-----|-----|---|
| GDOT PI # | N/A | <p>Note: Up to 5 alternatives may be selected and evaluated; Use this ICE Stage 1 to screen 5 or fewer alternatives to evaluate in Stage 2</p> <p>1. Does alternative address the project need in a balanced manner and in scale with the project? 2. Does alternative improve safety performance in terms of reducing severe crashes? 3. Does alternative incorporate safety, convenience and accessibility for pedestrians and/or bicyclists? 4. Does alternative improve (or preserve) traffic operations (congestion, delay, reliability, etc.)? 5. Does alternative appear feasible given the site characteristics, constraints & location context? 6. Does alternative appear feasible with respect to other project factors? 7. Overall feasible alternative (select alternative for further evaluation in Stage 2)?</p> <p>Screening Decision Justification:</p> | | | | | | | |
| Project Location: | SR10 Athens Hwy @ Dwy B | | | | | | | | |
| Existing Control: | Conventional (Minor Stop) | | | | | | | | |
| Prepared by: | Kimley-Horn | | | | | | | | |
| Date: | 8/29/2022 | | | | | | | | |
| <p>Answer "Yes" or "No" to each policy question for each control type to identify which alternatives should be evaluated in the Stage 2 Decision Record; enter justification in the rightmost column</p> | | | | | | | | | |
| <p>Intersection Alternative (see "Intersections" tab for detailed description of intersection/interchange type)</p> | | | | | | | | | |
| Unsignalized Intersections | Conventional (Minor Stop) | Yes | No | No | Yes | Yes | Yes | Yes | See Stage 2 |
| | Conventional (All-Way Stop) | No | Yes | Yes | No | No | No | No | Major street ADT too high |
| | Mini Roundabout | No | Yes | Yes | Yes | No | No | No | Major street ADT too high |
| | Single Lane Roundabout | No | Yes | Yes | No | No | No | No | Major street ADT too high |
| | Multilane Roundabout | No | Yes | Yes | No | No | No | No | Geometry does not allow for this, not in line with project purpose. |
| | RCUT (stop control) | No | Yes | No | No | Yes | No | Yes | See stage 2 |
| | RIRO w/down stream U-Turn | No | Yes | No | No | Yes | Yes | Yes | See stage 2 |
| | High-T (unsignalized) | No | No | No | No | No | No | No | Not a T-intersection |
| | Offset-T Intersections | No | Yes | No | No | No | No | No | Geometry does not allow for this, not in line with project purpose. |
| | Diamond Interch (Stop Control) | No | No | No | No | No | No | No | Not an interchange |
| | Diamond Interch (RAB Control) | No | No | No | No | No | No | No | Not an interchange |
| | Add LT Lanes on SR 22 No RT Lane Improvements | Yes | No | No | No | Yes | Yes | No | |
| | Other unsignalized (provide description): | No | No | No | No | No | No | No | N/A |
| | Signalized Intersections | Traffic Signal | No | Yes | Yes | No | No | No | No |
| Median U-Turn (Indirect Left) | | No | No | No | Yes | No | No | No | Not expected to satisfy signal warrants based only on mainline lefts. |
| RCUT (signalized) | | No | No | No | No | No | No | No | Not expected to satisfy signal warrants based only on mainline lefts. |
| Displaced Left Turn (CFI) | | No | No | No | Yes | No | No | No | Right-of-way not available, not in line with project purpose. |
| Continuous Green-T | | No | No | No | No | No | No | No | Not a T-intersection |
| Jughandle | | No | No | No | Yes | No | No | No | Right-of-way not available, not in line with project purpose. |
| Quadrant Roadway | | No | No | No | No | No | No | No | Right-of-way not available, not in line with project purpose. |
| Diamond Interch (Signal Control) | | No | No | No | No | No | No | No | Not an interchange |
| Diverging Diamond | | No | No | No | No | No | No | No | Not an interchange |
| Single Point Interchange | | No | No | No | No | No | No | No | Not an interchange |
| No LT Lane Improvements No RT Lane Improvements | | No | No | No | No | No | No | No | N/A |
| Other Signalized (provide description): | | No | No | No | No | No | No | No | N/A |

☒ = Intersection type selected for more detailed analysis in Stage 2 Alternative Selection Decision Record

Project Location: SR10 Athens Hwy @ Dwy B
 Existing Intersection Control: Conventional (Minor Stop)
 Type of Analysis: **Conventional Non-Safety Funded Project**

District: 1 - Gainesville
 County: Gwinnett
 Area: Suburb/Transit
 GDOT PI #: N/A
 Prepared by: Kimley-Horn
 Date: 8/29/2022

Opening / Design Year Traffic Operations

| | | | |
|---|--------------------|------------|--|
| Intersection meets signal/AWS warrants? | None | | Complete Streets Warrants Met? <input type="checkbox"/> PEDESTRIANS <input type="checkbox"/> BICYCLES <input type="checkbox"/> TRANSIT |
| Traffic Analysis Measure of Effectiveness | Intersection Delay | | |
| Traffic Analysis Software Used | Synchro | | |
| Analysis Time Period | AM Peak Hr | PM Peak Hr | |
| 2026 Opening Yr No-Build Peak Hr Intersection Delay | 0.5 sec | 2.8 sec | |
| 2026 Opening Yr No-Build Peak Hr Intersection V/C | 0.17 | 0.57 | |
| 2026 Design Yr No-Build Peak Hr Intersection Delay | 0.0 sec | 0.0 sec | |
| 2026 Design Yr No-Build Peak Hr Intersection V/C | 0.00 | 0.00 | |

| Crash Type | Crash Severity | | | | | Years: |
|---|----------------|----|----|----|----|--------|
| | K* | A* | B* | C* | O | |
| Crash Data: Enter most recent 5 years of crash data | | | | | | 5 |
| Angle | 0 | 0 | 0 | 1 | 13 | 64% |
| Head-On | 0 | 0 | 0 | 0 | 2 | 9% |
| Rear End | 0 | 0 | 0 | 0 | 2 | 9% |
| Sideswipe - same | 0 | 0 | 0 | 0 | 2 | 9% |
| Sideswipe - opposite | 0 | 0 | 0 | 0 | 1 | 5% |
| Not Collision w/Motor Veh | 0 | 0 | 0 | 0 | 1 | 5% |
| TOTALS: | 0 | 0 | 0 | 1 | 21 | 22 |

* Number of crashes resulting in injuries / fatalities, not number of persons

Alternatives Analysis:

Proposed Control Type/Improvement:

Project Cost: (From CostEst Worksheet)

| | Alternative 1 | Alternative 2 | Alternative 3 | Alternative 4 | Alternative 5 |
|---------------------------------------|-----------------------------|-----------------------------|-----------------------------|-----------------------------|-----------------------------|
| | Conventional (Minor Stop) | RCUT (stop control) | RIO w/down stream U-Turn | N/A | N/A |
| | Additional description here | Additional description here | Additional description here | Additional description here | Additional description here |
| Construction Cost | \$200,000 | \$658,000 | \$541,000 | | |
| ROW Cost | \$0 | \$406,000 | \$406,000 | | |
| Environmental Cost | \$0 | \$0 | \$0 | | |
| Reimbursable Utility Cost | \$0 | \$9,000 | \$8,000 | | |
| Design & Contingency Cost | \$0 | \$170,000 | \$140,000 | | |
| Cost Adjustment (justification req'd) | 0% | 0% | 0% | | |
| Total Cost | \$200,000 | \$1,243,000 | \$1,095,000 | | |

Traffic Operations:

| | | | | | | | | |
|---|------------|------------|------------|------------|------------|------------|--|--|
| | Synchro | | Synchro | | Synchro | | | |
| Traffic Analysis Software Used | AM Peak Hr | PM Peak Hr | AM Peak Hr | PM Peak Hr | AM Peak Hr | PM Peak Hr | | |
| Analysis Period | 482.1 sec | 47.0 sec | 158.8 sec | 63.1 sec | 415.0 sec | 181.6 sec | | |
| 2026 Design Yr Build Intersection Delay | 1.69 | 0.38 | 0.83 | 0.70 | 1.51 | 1.16 | | |
| 2026 Design Yr Build Intersection V/C | | | | | | | | |

Safety Analysis:

| | | | | | |
|---|---|-----------------|-----------------------------------|--|--|
| Predefined CRF: PDO | 0% | 31% | 35% | | |
| Predefined CRF: Fatal/Inj | 0% | 53% | 54% | | |
| Predefined CRF Source: | CRF unavailable; provide user defined CRF below | NC/MO Table 4-7 | FHWA Clearinghouse #s 5555 / 5556 | | |
| User Defined CRF: PDO | | | | | |
| User Defined CRF: Fatal/Inj | | | | | |
| User Defined CRF Source (write in if applicable): | | | | | |

Environmental Impacts:¹

| | | | | | |
|----------------------------|------|------|------|--|--|
| Historic District/Property | None | None | None | | |
| Archaeology Resources | None | None | None | | |
| Graveyard | None | None | None | | |
| Stream | None | None | None | | |
| Underground Tank/Hazmat | None | None | None | | |
| Park Land | None | None | None | | |
| EJ Community | None | None | None | | |
| Wooded Area | None | None | None | | |
| Wetland | None | None | None | | |

Stakeholder Posture:

| | | | | | |
|-------------------------|---------|---------|---------|--|--|
| Local Community Support | Unknown | Unknown | Unknown | | |
| GDOT Support | Unknown | Unknown | Unknown | | |

| | | | | | |
|---------------------------------------|--------------------------------------|------------|------------|--|--|
| Final ICE Stage 2 Score: | 3.2 | 4.5 | 3.7 | | |
| Rank of Control Type Alternatives: | 3 | 1 | 2 | | |
| Final Intersection Control Selection: | 3 - Conventional (Minor Stop) | | | | |

Note: Stage 2 score is not given (shown as "-") if signal or AWS is selected as control type but respective warrants are not met

Provide additional comments and/or explain any unique analysis inputs, or results (as necessary):

Synchro 11 used for analysis. RCUT/RIO delay and v/c includes weighted average of southbound movement and corresponding displaced u-turn and travel time (worst approach). Conventional minor stop with turn lane delay and v/c represents worst approach (southbound left). Cost override tool used to include cost of conventional driveway.

Waiver Request - Level 2 / 3

In certain circumstances where an ICE would otherwise be required, an ICE may be waived based on appropriate evidence presented with a written request. Scenarios in which an ICE waiver request may be considered include:

- Proposed improvements do not substantially alter the character of the intersection, and are considered minor in nature, such as extending existing turn lane(s) or modifying signal phasing at an existing traffic signal
- The intersection consists of a public roadway intersecting a divided, multilane roadway where the access will be limited to a closed median with only right-in/right-out access that will operate acceptably; or
- The intersection is along an undivided, two-lane roadway that will not be widened and meets the following criteria:
 - Low risk in terms of exposure (total intersection entering volume less than 1,000 vehicles /day)
 - Latest 5 years of crash history is not indicative of a crash problem (no discernible crash patterns coupled with low crash frequency and severity)
 - Layout has no unusual or undesirable geometric features (such as restricted sight distance)
 - The proposed changes are not expected to adversely affect safety

If only one alternative is determined to be feasible from the ICE Stage 1, then a waiver may be submitted in lieu of completing ICE Stage 2. The waiver must clearly explain why there is no other feasible alternative. A Waiver Form should also be submitted to document an agreed upon decision to select a preferred alternative other than the highest scoring alternative in Stage 2.

ICE waiver forms with supporting documentation should be submitted for approval to the Office of Traffic Operations or District Engineer (depending on Waiver level). Questions regarding the waiver process should be routed to the State Traffic Engineer.

Project Information:

Location: SR10 Athens Hwy @ Dwy B

County: Gwinnett

GDOT District: 1 - Gainesville

Area Type: Suburb/Transition

Existing Intersection Control: Conventional (Minor Stop)

GDOT PI # (or N/A): N/A

Requested By: GDOT

Prepared By: Kimley-Horn

Date: 8/29/2022

Waiver Request Type: Driveway Permit

Traffic and Operations Data:^{1,2}

Intersection meets signal/AWS warrants?

None

Traffic Analysis Type:

Intersection Delay

Existing Major Street Avg Daily Traffic (ADT):

38,800

Existing Minor Street Avg Daily Traffic (ADT):

745

Analysis Period:

AM Peak

PM Peak

2026 Opening Yr Peak Hour Intersection Delay:

482.1 sec

47.0 sec

2026 Opening Yr Peak Hour Intersection V/C:

1.69

0.38

2026 Design Yr Peak Hour Intersection Delay:

482.1 sec

47.0 sec

2026 Design Yr Peak Hour Intersection V/C:

1.69

0.38

Crash Data (Required):³

| Crash Data: Enter most recent 5 years of crash data | Crash Severity | | | | | Years: |
|---|----------------|----------|----------|----------|-----------|-----------|
| | K* | A* | B* | C* | O | |
| Angle | 0 | 0 | 0 | 1 | 13 | 64% |
| Head-On | 0 | 0 | 0 | 0 | 2 | 9% |
| Rear End | 0 | 0 | 0 | 0 | 2 | 9% |
| Sideswipe - same | 0 | 0 | 0 | 0 | 2 | 9% |
| Sideswipe - opposite | 0 | 0 | 0 | 0 | 1 | 5% |
| Not Collision w/Motor Veh | 0 | 0 | 0 | 0 | 1 | 5% |
| TOTALS: | 0 | 0 | 0 | 1 | 21 | 22 |

* Number of crashes resulting in injuries / fatalities, not number of persons

Description of Work / Justification for Waiver (Required):

A waiver is requested to select an alternative that did not rank #1 in ICE Stage 2. A conventional (minor stop) intersection is requested at the proposed site driveway to maintain existing full movement access as well as to match the overall characteristics of the corridor to avoid negatively impacting driver expectancy. A center two-way left-turn lane currently existing along Athens Hwy and no median break is required.

Proposed Intersection Control:

Conventional (Minor Stop)

REQUESTED BY:

Matt Flynn, P.E.

Date:

8/29/2022

Title:

Project Engineer

APPROVED BY:

Date:

Name:

District Engineer or (Approved Delegate)

¹ Analysis data input on this worksheet is for proposed control & configuration on form, not the No-Build data shown on the top of Stage 2

² ADT's required if available (from data collected or nearest GDOT count station site); Capacity data optional unless needed to justify basis of the waiver request.

³ Crash data (required for all existing intersections) must be entered here independent from Stage 2 worksheet inputs (not linked)