## Transportation Analysis

# 1359 Ellsworth DRI #3613

City of Atlanta, Georgia

May 2022

Prepared for:

Stream Realty

Prepared by:

Kimley-Horn and Associates, Inc. 817 West Peachtree Street NW, Suite 601 Atlanta, GA 30308 017502007



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## **Available Upon Request**

Raw Traffic Count Data
Synchro Capacity Analyses

#### **EXECUTIVE SUMMARY**

This report presents the analysis of the anticipated traffic impacts of the proposed 1359 Ellsworth development located in Atlanta, Georgia. The approximate 5-acre site is located along the east side of Ellsworth Industrial Boulevard. The site currently consists of two (2) light industrial buildings.

The proposed development will consist of the following land uses and densities contained in **Table 1**. The project is expected to be completed by 2025 (approximately 3 years).

Table 1: Proposed Land Use and Density									
Multifamily Residential	665 dwelling units								
Commercial/Restaurant	13,000 SF								
Brewery Tap Room	12,500 SF								
Brewery Manufacturing	12,500 SF								

The DRI analysis includes an estimation of the overall vehicle trips projected to be generated by the development, also known as gross trips. Mixed-use and pass-by reductions to gross trips are included in the trip generation, as outlined in the Georgia Regional Transportation Authority (GRTA) Letter of Understanding (dated March 22, 2022).

Capacity analyses were performed for the study intersections under the Estimated 2022 conditions, the Projected 2025 No-Build conditions, and the Projected 2025 Build conditions.

- Estimated 2022 conditions represent current traffic volumes collected in March 2022 that were calibrated to account for COVID-19's impact on traffic.
- Projected 2025 No-Build conditions represent the Estimated 2022 traffic volumes grown for three (3) years using a 1.5% per year growth rate.
- Projected 2025 Build conditions represent the Projected 2025 No-Build conditions plus the addition of the project trips that are anticipated to be generated by the 1359 Ellsworth development.

#### Estimated 2022 Conditions (System Improvements)

## Ellsworth Industrial Boulevard at Chattahoochee Avenue (Intersection 1)

The signalized intersection of Chattahoochee Avenue at Ellsworth Industrial Boulevard (Intersection 1) is projected to operate at an acceptable <u>overall</u> LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour. The intersection is projected to operate at an unacceptable LOS for the southbound approach under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the PM peak hour.

The following <u>system improvements</u>, if installed, would enable Intersection 1 to meet GRTA's LOS requirements (shown in red on **Figure 7**):

- Widen the westbound approach along Chattahoochee Avenue to add one (1) left-turn lane, so that it consists of one (1) left-turn lane, one (1) through lane, and one (1) shared through/right-turn lane.
- Widen the eastbound approach along Chattahoochee Avenue to add one (1) left-turn lane and one (1) right-turn lane, so that it consists of one (1) left-turn lane, two (2) through lanes, and one (1) right-turn lane.

It should be noted that the above improvements may not be recommended, as the Chattahoochee Avenue Multi-Use Path is planned along the south side of Chattahoochee Avenue (noted in **Table 8**). Consider if the addition of turn lanes aligns with the goals for future multimodal planned improvements along Chattahoochee Avenue.

Ellsworth Industrial Boulevard at Chattahoochee Avenue (Intersection 1) Improved LOS Summary

		OS Standard: D		∕orth Ind 3oulevai		_	vorth Indu Boulevar		Ch	attahood Avenue		Cha	attahood Avenue	
Appr	oach l	LOS Standard: D		Iorthbou			Southbour		E	astbour		V	Vestbour	
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall LOS						D (43	.9)					
		Approach LOS		D (54.5	)		D (47.3)			D (40.4	)		D (43.9)	)
2022 ESTIMATED (SIGNAL)	AM	Storage	50			175								
L)		50th Queue	64		293	18		4	15	460	466	348	142	135
₽¥		95th Queue	117		441	30		12	25	642	649	533	244	231
IS:		Overall LOS						B (15	.0)					
2 E (S		Approach LOS		C (33.1	)		C (33.3)			B (15.3	)		A (9.1)	
02	P	Storage	50			175								
7		50th Queue	85		49	52		35	5	103	102	87	88	86
		95th Queue	152		90	94		61	8	188	185	156	161	155
		Overall LOS	D (44.0)											
	_	Approach LOS		D (50.5	)		D (47.1)			D (54.8	)		C (31.9)	)
9	AM	Storage	50			175								
<u>_</u>		50th Queue	70		227	18		4	15	563	571	451	137	130
NA A		95th Queue	126		356	33		12	28	764	775	639	236	224
2025 NO-BUILD (SIGNAL)		Overall LOS	B (15.6)											
25 (S		Approach LOS	C (33.4)			C (33.9)			B (15.5)			A (9.8)		
20	P	Storage	50			175								
		50th Queue	88		58	55		35	5	112	110	100	94	94
		95th Queue	161		107	100		65	8	201	198	182	172	168
		Overall LOS						D (49	.9)					
	_	Approach LOS		D (50.2	)		D (46.6)			D (50.9	)		D (48.9)	)
Δ.	AM	Storage	50			175								
<u> </u>		50th Queue	82		285	18		4	18	511	35	449	161	152
BC NA		95th Queue	149		430	33		84	30	707	64	673	268	254
2025 BUILD (SIGNAL)		Overall LOS		0 (0.4.1		ı	D (05.0)	B (16	· '			T ()		
200	5	Approach LOS		C (34.1	)	475	D (35.6)			B (14.4	)		B (11.9)	1
	PM	Storage	50		0.5	175		25	_	0.7	20	407	0.4	0.4
		50th Queue	99 175		85 153	58 103		35 65	5 8	87 158	38 67	137 238	94 172	94
		95th Queue	1/5		133	103		CO	Ö	IDB	70	∠38	1/2	168

### Ellsworth Industrial Boulevard at Huff Road (Intersection 4)

The intersection of Ellsworth Industrial Boulevard at Huff Road (Intersection 4) is projected to operate at an unacceptable <u>overall</u> LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour. The eastbound approach of the intersection is projected to operate at an unacceptable LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour.

In order to meet GRTA's LOS requirements under the Estimated 2022 conditions, the <u>system improvements</u> listed below are needed and recommended (shown in red on **Figure 7**):

- Widen the eastbound approach along Huff Road to add one (1) left-turn lane, so that it consists of one
   (1) left-turn lane and one (1) through lane.
  - Note: Right-of-way may not be available to accommodate the widening for a left-turn lane
- Install a traffic signal if and when it is warranted and approved by the City of Atlanta
  - The intersection was modified from side-street stop-control to an all-way stop-control (AWSC) in 2021 following a study by the Upper Westside CID. The study concluded that a signal warrant was possible, but that AWSC was an appropriate interim solution for a future signal as a quick response due to sight distance concerns. The study concluded that a signal may be warranted based on Warrant 1 (8 hour warrant) and Warrant 2 (4 hour warrant), but Warrant 3 (peak hour warrant) was not satisfied. A review of the AM and PM peak hours considered for this DRI indicates that a signal may be warranted.

Ellsworth Industrial Boulevard at Huff Road (Intersection 4) Improved LOS Summary

		OS Standard: D OS Standard: D		,	-			orth Indu Boulevard		Н	uff Roa	d		uff Roa	
				North	bound		So	outhbour		E	astbour		W	estbour	
			L	٦	Γ	R	L	Т	R	L	Т	R	L	Т	R
		Overall LOS						[	3 (13.2)						
	_	Approach LOS						B (19.2)			B (11.6)		E	3 (13.2)	
	Α	Storage													
l ≦ ເວ	,	50th Queue					79			74	87				122
<u> </u> ≧ ⊗		95th Queue					146			135	158				211
2022 ESTIMATED (AWSC)		Overall LOS						[	3 (19.2)						
2 E	_	Approach LOS						B (11.0)		(	C (22.7)	)	(	C (22.0)	
02	Σ	Storage													
•		50th Queue					76			48	73				147
		95th Queue					134			84	133				249
		Overall LOS		B (13.0)											
	_	Approach LOS	C (20.6) B (11.2)							E	3 (12.6)				
2	ΑM	Storage													
		50th Queue					88			79	84				122
P-G SSC		95th Queue					146			135	158				211
2025 NO-BUILD (AWSC)		Overall LOS							3 (14.3)						
25	_	Approach LOS						B (10.9)		I	B (17.8)	)	E	3 (13.8)	
20	A	Storage													
		50th Queue					55			33	54				79
		95th Queue					134			84	133				249
		Overall LOS		B (10.8)											
		Approach LOS						C (31.9)			A (3.9)		E	3 (10.6)	
	Α	Storage													
۱ΞΩ		50th Queue					149			30	14				107
		95th Queue					158			145	152				213
2025 BUILD (AWSC)		Overall LOS							3 (19.0)						
502	_	Approach LOS						B (11.9)		(	C (22.1)	)	(	C (21.3)	
, ,	P	Storage													
		50th Queue					82			48	76				152
		95th Queue					149			89	136				254

#### Marietta Boulevard at Huff Road/Kennesaw Drive (Intersection 5)

The intersection of Marietta Boulevard at Huff Road/Kennesaw Drive (Intersection 5) is projected to operate at an unacceptable <u>overall</u> LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour. The eastbound and westbound approaches of the intersection are projected to operate at an unacceptable LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour. The southbound approach is projected to operate at an unacceptable LOS under the No-Build 2025 and Build 2025 conditions during the PM peak hour.

The following <u>system improvement</u>, if installed, would improve the delay experienced by the southbound approach at intersection 5 (shown in red on **Figure 7**Figure 7):

 Widen the southbound approach along Marietta Boulevard to add one (1) left-turn lane, so that it consists of one (1) left-turn lane, one (1) through lane, and one (1) shared through/right-turn lane

Although the eastbound and westbound approaches are projected to operate at LOS E or F, no feasible improvements exist. The failing LOS is caused by existing split-phased signal timing required because of the offset geometry of Kennesaw Dr and Huff Rd. Split-phased signal timings decrease the amount of time available within the full cycle length of the signal. It is notable that Kennesaw Drive has minimal volume during the peak hours. Kennesaw Drive serves a site that has alternative access at a signalized intersection along Marietta Boulevard located approximately 1,200 feet north of the intersection of Marietta Boulevard at Huff Road/Kennesaw Drive.

No improvement is recommended to address low approach LOS for the eastbound and westbound approaches without further study of intersection geometry constraints.

Marietta Boulevard at Huff Road/Kennesaw Drive (Intersection 5) Improved LOS Summary

		OS Standard: D	Mari	etta Boule	evard	Marie	tta Boule	evard	Kenr	nesaw D	rive		uff Road	
Appro	ach L	.OS Standard: D	N	Northboun		So	outhbour		Ea	astboun		We	estboun	
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall LOS					(	C (32.4)	)					
	_	Approach LOS		C (28.6)			C (20.6)		F	(82.2)		F	(81.3)	
ΙË	ΑM	Storage			200									
I≦≘	,	50th Queue		311	22	123	292			27			312	
2022 ESTIMATED (Signal)		95th Queue		381	102	254	366			62			483	
Siç		Overall LOS					(	C (30.5)	)					
2 E	_	Approach LOS		D (39.3)			C (20.5)		[	) (44.5 <u>)</u>		C	(28.9)	
05	Δ	Storage			200									
, N		50th Queue		202	0	28	126			1			199	
		95th Queue		279	48	58	182			15			442	
		Overall LOS					[	D (36.6	)					
	_	Approach LOS		D (35.7)			C (24.2)		F	(85.2)		F	(81.1)	
<u> </u>	Α	Storage			200									
		50th Queue		374	54	167	374			29			322	
-B na		95th Queue		545	156	341	447			65			517	
NO-BU		Overall LOS	D (47.9)											
2025 NO-BUILD (Signal)	_	Approach LOS		E (76.0)			C (22.9)		[	) (44.5)		C	(28.3)	
20	Δ	Storage			200									
		50th Queue		268	0	30	164			1			202	
		95th Queue		396	49	60	220			15			469	
		Overall LOS						D (39.9	)					
	_	Approach LOS		D (36.6)			C (29.7)		F	(85.2)		F	(81.5)	
	ΑM	Storage			200									
2025 BUILD (Signal)		50th Queue		376	59	172	376			29			373	
25 BUIL (Signal)		95th Queue		456	167	346	450			65			604	
Sig		Overall LOS					[	D (38.7						
502	_	Approach LOS		D (53.9)			C (21.2)		[	) (44.5 <u>)</u>		C	(34.3)	
``	Δ	Storage			200									
		50th Queue		261	0	29	158			1			225	
		95th Queue		377	52	57	210			15			527	

<sup>\*</sup>Intersection was analyzed with HCM 2000.

#### 2025 No-Build Conditions (System Improvements)

### Ellsworth Industrial Boulevard at Elaine Avenue (Intersection 2)

The intersection of Ellsworth Industrial Boulevard at Elaine Avenue (Intersection 2) is projected to operate at an acceptable <u>overall</u> LOS under the Estimated 2022 and 2025 No-Build conditions. The northbound approach is projected to operate at LOS E under the 2025 No-Build conditions during the AM peak hour.

It should be noted that although the delays are projected to operate at unacceptable levels per GRTA's LOS requirements, the delays are not extreme.

In order to meet GRTA's LOS requirements under the 2025 No-Build conditions, the <u>system improvements</u> listed below are needed (shown in green on **Figure 8**):

- Restripe the northbound approach along Ellsworth Industrial Boulevard, so that it consists of one (1) shared through/left-turn lane and one (1) through lane.
- Restripe the receiving lane on the north leg along Ellsworth Industrial Boulevard, so that it consists of two (2) northbound receiving lanes.

The intersection is projected to operate at an unacceptable <u>overall</u> LOS under the 2025 Build conditions without proposed system improvements to the northbound approach. However, with the addition of the proposed 2025 No-Build system improvements, including an increase in northbound approach and receiving lanes, the southbound approach under 2025 Improved Build conditions requires mitigation to operate acceptably under GRTA's LOS requirements with the following Build improvement listed below (shown in blue on **Figure 9**):

Reconfigure the southbound approach along Ellsworth Industrial Boulevard, so that it consists of one
 (1) right-turn lane and one
 (1) through lane.

With the <u>system improvements</u> identified to mitigate low LOS for the northbound approach in the 2025 No-Build conditions and the system improvement identified to mitigate low LOS for the southbound approach in the 2025 Build Improved conditions, Intersection 2 would meet GRTA's LOS requirements both under 2025 Improved No-Build and 2025 Improved Build conditions.

However, it is not uncommon for stop-controlled intersections to operate with low LOS and delay. Potential improvements to the stop-controlled intersection should consider pedestrian activity, which is likely to increase at this intersection with the installation of the programmed PATH Trail and the potential future BeltLine alignment that may also interact with this intersection. A traffic signal is not likely to be warranted at this intersection based on existing or future vehicular peak hour volumes. Depending on future pedestrian activity associated with the future trail projects, a signal may be pursued to improve pedestrian crossing at the intersection but has not been considered for this DRI.

The analysis results shown in the table below are for the improved conditions at Ellsworth Industrial Boulevard at Elaine Avenue (Intersection 2), which assume the noted geometric changes.

## Ellsworth Industrial Boulevard at Elaine Avenue (Intersection 2) Improved LOS Summary

		S Standard: D OS Standard: D	Ellsworth Industrial Boulevard			_	Ellsworth Industrial Boulevard			Elaine Avenue			-		
			١	Northboun	ıd	Southbound			Eastbound			Westbound			
			L	Т	R	L	Т	R	L	Т	R	L	Т	R	
		Overall LOS						C (2	20.4)						
		Approach LOS		C (16.3)			C (23.8)			C (22.3)					
9	AM	Storage													
l ∃ ⊙		50th Queue													
- B-C		95th Queue	44	91			152		112		5				
2025 NO-BUILD (AWSC)		Overall LOS						C (1	6.6)						
25		Approach LOS		B (10.6)		C (21.2)		B (13.8)							
20	P	Storage													
		50th Queue													
		95th Queue	20	25			163		48		3				
		Overall LOS						C (1	9.5)						
	_	Approach LOS		C (20.4)			C (15.5)			C (23.5)					
	AM	Storage													
l ⊒ ∵	,	50th Queue													
B /SC		95th Queue	61	145			94	65	119		8				
2025 BUILD (AWSC)		Overall LOS						B (1	3.1)						
50%	_	Approach LOS		B (11.6)			B (13.6)			B (14.1)					
, ,	PM	Storage													
		50th Queue													
		95th Queue	29	38			100	65	51		5				

#### **Build 2025 Conditions (Site Access Improvements)**

In addition to the system improvements, the following should be considered to serve the Projected 2025 Build Conditions:

- Ellsworth Industrial Boulevard at Driveway A (Intersection 6)
  - o Construct Driveway A to consist of one (1) ingress lane and one (1) egress lane.
- Ellsworth Industrial Drive at Driveway B (Intersection 7)
  - o Construct Driveway B to consist of one (1) ingress lane and one (1) egress lane.
- Work with MARTA and the City of Atlanta to consider the relocation of MARTA Bus Stop #902230, currently
  located approximately 205 feet north of the site, onto the project site to align with proposed new sidewalks
  and pedestrian infrastructure.

Ellsworth Industrial Boulevard at Driveway A (Intersection 6) LOS Summary

	Overall LOS Standard: D Approach LOS Standard: D		Ellsworth Industrial Boulevard			Ellsworth Industrial Boulevard			-			Driveway A		
			Northbound			Southbound			Eastbound			Westbound		nd
			١	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall LOS		A (1.9)										
	AM	Approach LOS		A (0.0)			A (1.5)				C (16.5)			
		Storage												
ی ⊑ ا		50th Queue												
25 BUILD (TWSC)		95th Queue				6							24	
		Overall LOS	A (2.						2.2)					
2025 (TV		Approach LOS		A (0.0)			A (1.9)					Е	3 (12.9)	
•	A	Storage												
		50th Queue												
		95th Queue				6							13	

Ellsworth Industrial Drive at Driveway B (Intersection 7) LOS Summary

	Overall LOS Standard: D Approach LOS Standard: D		-			Driveway B			Ellsworth Industrial Drive			Ellsworth Industrial Drive			
	ļ			Northbound			Southbound			Eastbound			Westbound		
				Т	R	L	T	R	L	Т	R	L	T	R	
		Overall LOS		A (8.0)											
	_	Approach LOS				A (8.6)			A (7.3)			A (0.0)			
	Α	Storage													
BUILD VSC)	_	50th Queue													
301 180		95th Queue				6			3						
25 BUIL (TWSC)		Overall LOS						A (7.	8)						
2025 (TV		Approach LOS				A (8.6)			A (7.4)			A (0.0)			
,,	PM	Storage													
		50th Queue													
		95th Queue				6			5						

#### 1.0 PROJECT DESCRIPTION

#### 1.1 Introduction

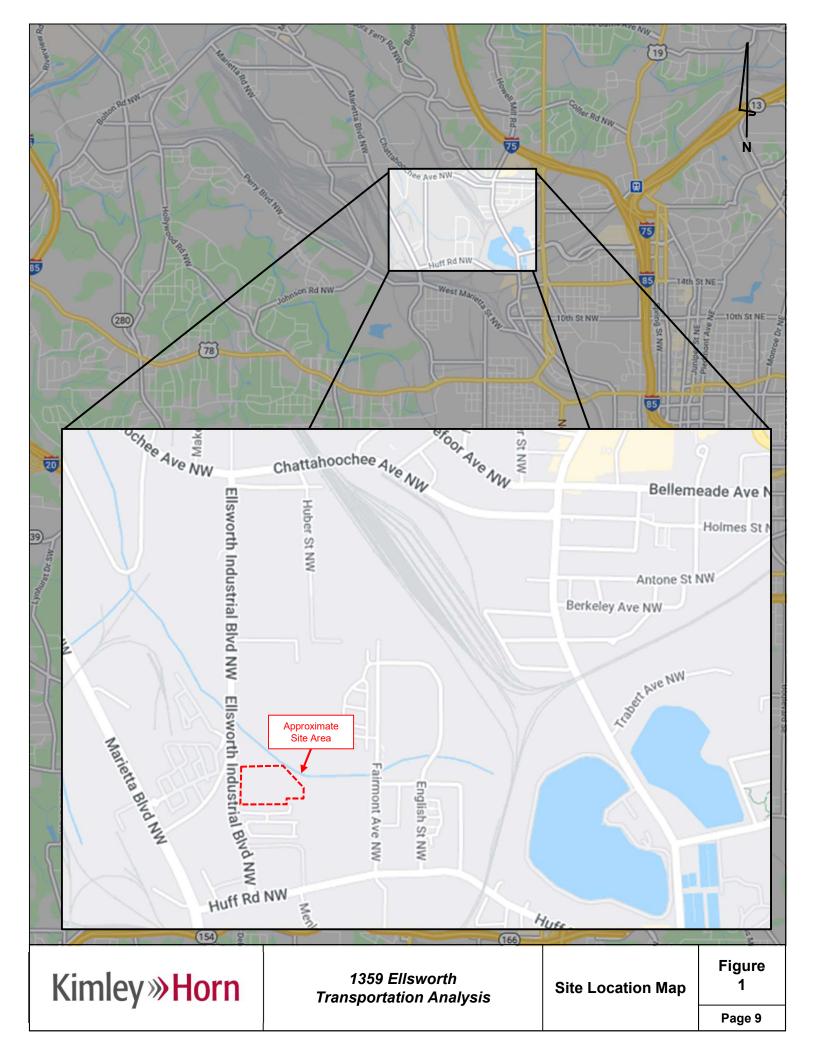
This report presents the analysis of the anticipated traffic impacts of the proposed 1359 Ellsworth development located in Atlanta, Georgia. The approximate 5-acre site is located along the east side of Ellsworth Industrial Boulevard. The project site is currently zoned I-2 (Light Industrial). Permit #Z-22-020 was filed on March 9, 2022. **Figure 1** provides a location map of the project site. **Figure 2** provides an aerial view of the project site and surrounding area.

The site currently contains two (2) light industrial buildings, consisting of 116,206 SF. The proposed development will consist of the following land uses and densities contained in **Table 2**. The project is expected to be completed by 2025 (approximately 3 years).

Table 2: Proposed Land Use and Density								
Land Use Proposed								
Multifamily Residential	665 dwelling units							
Commercial/Restaurant	13,000 SF							
Brewery Tap Room	12,500 SF							
Brewery Manufacturing	12,500 SF							

A reference of the proposed site plan is provided in **Appendix A**. A full-sized site plan consistent with GRTA's Site Plan Guidelines is also being submitted as part of the review package.

The project is considered a Development of Regional Impact (DRI) and is subject to Georgia Regional Transportation Authority (GRTA) and Atlanta Regional Commission (ARC) review due to the project size exceeding 500,000 SF of mixed-use development in a Maturing Neighborhood Area (per UGPM). The DRI was formally triggered with the filing of the Initial DRI Information (Form 1) on March 1, 2022 by the City of Atlanta. This transportation analysis includes all inputs and methodologies discussed at the DRI Methodology Meeting with GRTA, ARC, and other stakeholders. The inputs and methodologies are outlined in the GRTA Letter of Understanding (LOU) dated March 22, 2022.





Kimley»Horn

1359 Ellsworth Transportation Analysis

Site Aerial

Figure 2

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#### 1.2 Site Access

As currently envisioned, the proposed development will be accessible via two (2) access points:

- 1. **Site Driveway A** an existing to be reconstructed full-movement driveway located along the Ellsworth Industrial Boulevard, approximately 275 feet south of Elaine Avenue that will continue to operate under side-street stop control.
- Site Driveway B an existing full-movement driveway located along Ellsworth Industrial Drive (private road) approximately 500 feet east of Ellsworth Industrial Boulevard that is proposed to operate under sidestreet stop control.
  - a. Note: No changes are proposed along Ellsworth Industrial Drive (private road)

## 1.3 Internal Circulation Analysis

An internal private roadway through the site will provide access to all of the buildings and parking facilities.

## 1.4 Parking

The current number of total site parking spaces to be provided are listed below in Table 3.

		Table 3: Proposed	d Parking	
Land Use	Parking Type	Minimum (MRC-3)	Proposed	
Commercial	Car	22 1 space per 600 SF	32 Max of Min+10, Min*1.25	
Eating/Drinking (>60% Alcohol Sales)	Car	66 1 space per 450 SF	83 Max of Min+10, Min*1.25	840 shared parking
Residential	Car	374 0.55 space per 1 unit	998* 1 per 1 BR, 2 per 2+ BR	(in parking deck)
Total		Min: 454	Max: 1,113	

<sup>\*</sup>Residential unit mix pending, preliminary parking estimates assume half units as 1-BR and half 2+BR units.

Additional parking details are provided on the proposed site plan in Appendix A.

## 1.5 Alternative Transportation Facilities

There are not any existing dedicated pedestrian or bicycle facilities along the site frontage. The project site plan proposes sidewalks along the site frontage. MARTA Route 14 currently serves Ellsworth Industrial Boulevard.

As discussed in the DRI pre-review meeting, MARTA is amenable to a relocation of the Route 14 northbound stop (Stop ID 902230), which is currently located approximately 250 feet north of the project site, to align with proposed installation of sidewalks on the 1359 Ellsworth site, which will be explored in further detail in permitting.

The Upper Westside CID and MARTA are currently working on a joint project to improve the nearest Route 14 southbound stop (Stop ID 901683).

## 2.0 TRAFFIC ANALYSES, METHODOLOGY AND ASSUMPTIONS

## 2.1 Study Network Determination

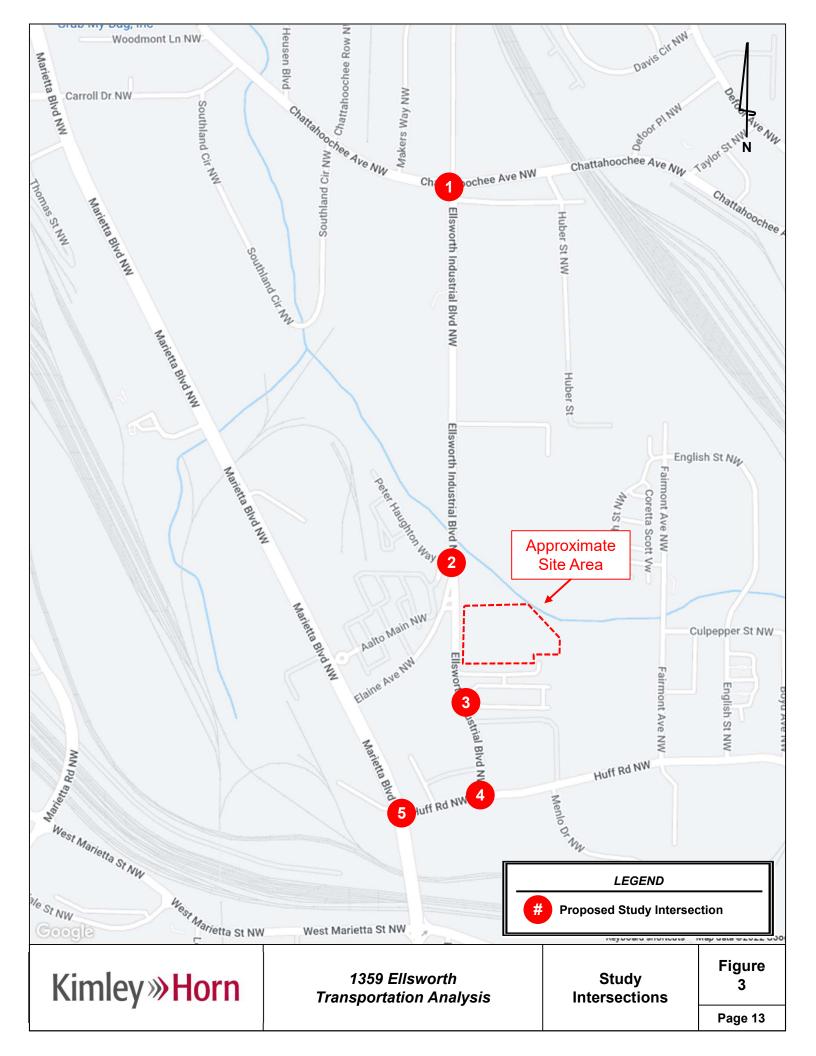
The study area was determined at the methodology meeting with input from GRTA, ARC, and other local agency stakeholders. The study includes the following five (5) off-site intersections described in **Table 4** and shown in **Figure 3**.

	Table 4: Intersection Control Summary											
	Intersection	Jurisdiction	Control									
1.	Ellsworth Industrial Boulevard at Chattahoochee Avenue	City of Atlanta	Signalized									
2.	Ellsworth Industrial Boulevard at Elaine Avenue	City of Atlanta	All-Way Stop Control (3-Leg)									
3.	Ellsworth Industrial Boulevard at Ellsworth Industrial Drive (private drive)	City of Atlanta	Side-Street Stop Control									
4.	Ellsworth Industrial Boulevard at Huff Road	City of Atlanta	All-Way Stop Control (3-Leg)									
5.	Marietta Boulevard at Ellsworth Industrial Boulevard	City of Atlanta	Signalized									

## 2.2 Existing Roadway Facilities

Roadway classification descriptions and estimated Annual Average Daily Traffic (AADT) for roadway segments within the study network are provided in **Table 5** (bolded roadways are adjacent to the site).

Table 5: Roadway Classifications												
Roadway  Lanes Posted Speed Limit Speed Limit Posted Speed Limit Speed Limit Speed Limit Posted Speed Limit Speed												
Ellsworth Industrial Boulevard	2	35 MPH	9,930	<b>Major Collector</b>								
Chattahoochee Avenue	4	35 MPH	13,400	Major Collector								
Ellsworth Industrial Drive	2	25 MPH	-	Private								
Huff Road	2	35 MPH	-	Major Collector								
Marietta Boulevard	4	35 MPH	16,700	Minor Arterial								



#### 2.3 Traffic Data Collection and Calibration

Traffic counts were collected at all five (5) existing study intersections on Tuesday, March 29, 2022. The collected counts were then calibrated using adjustment factors to account for the potential impacts of COVID-19 to typical traffic volumes and patterns.

A comparison was conducted for vehicular volumes along Ellsworth Industrial Boulevard south of Old Chattahoochee Avenue. Average Daily Traffic (ADT) volumes at this location from GDOT's Traffic Analysis & Data Application (TADA) in 2017 were grown at 1.5 percent per year for 5 years (2017 to 2022) to determine an Estimated 2022 GDOT ADT. The Estimated GDOT 2022 ADT was compared to the ADT volumes collected in March 2022 at the same location.

As a result of the volume comparison, it was determined that an <u>adjustment factor of 1.83 should be used for the existing AM turning movement counts</u>, and an <u>adjustment factor of 1.03 should be used for the existing PM turning movement counts</u>. The methodologies used in this analysis for traffic count calibration were approved by GRTA.

Traffic count peak hours for all the study intersections are shown in **Table 6**. The collected peak hour turning movement traffic counts are available upon request.

	Table 6: Traffic Count Summary												
	Intersection	Count Date	AM Peak Hour	PM Peak Hour									
1.	Ellsworth Industrial Boulevard at Chattahoochee Avenue	3/2022	8:00 – 9:00 AM	5:00 – 6:00 PM									
2.	Ellsworth Industrial Boulevard at Elaine Avenue	3/2022	8:00 – 9:00 AM	5:00 – 6:00 PM									
3.	Ellsworth Industrial Boulevard at Ellsworth Industrial Drive (private drive)	3/2022	7:45 – 8:45 AM	5:00 – 6:00 PM									
4.	Ellsworth Industrial Boulevard at Huff Road	3/2022	8:00 – 9:00 AM	5:00 – 6:00 PM									
5.	Marietta Boulevard at Ellsworth Industrial Boulevard	3/2022	8:00 – 9:00 AM	4:45 – 5:45 PM									

#### 2.3.1 Speed Data along Ellsworth Industrial Boulevard

As requested by the City of Atlanta during the Methodology Meeting, speed data was collected along Ellsworth Industrial Boulevard south of Old Chattahoochee Avenue along with the ADT volumes collection on Tuesday, March 29, 2022. The 85<sup>th</sup> percentile speed was 43 MPH and 42 MPH in the northbound and southbound directions, respectively. The complete speed data is attached in **Appendix E**.

## 2.4 Background Growth

Background traffic is defined as expected traffic on the roadway network in future year(s) absent the construction and opening of the proposed 1359 Ellsworth development. Background traffic includes a base growth rate, which is based on historical count data and population growth data. It can also include trips anticipated from nearby or adjacent other projects.

Based on methodology outlined in the GRTA Letter of Understanding (LOU), a 1.5% per year background traffic growth rate from 2022 to 2025 (3 years) was used for all roadways. Project traffic from the *Project Revive DRI* #3489 and *Project Granite DRI* #3298 developments were also included.

The Projected 2025 No-Build conditions represent the Estimated 2022 traffic volumes grown for three (3) years at 1.5% per year throughout the study network.

The Projected 2025 Build conditions represent the project trips generated by the *1359 Ellsworth* development (discussed in Section 3.0 and 4.0) added to the Projected 2025 No-Build Conditions.

## 2.5 Programmed and Planned Projects

Programmed and planned projects near the project site were researched to account for any improvements or modifications within the study network before or by the build-out year of the development. The programmed and planned projects were discussed in the methodology meeting with GRTA, ARC, and other local stakeholders.

The following projects shown in **Table 7** are programmed to occur near the development.

	Т	able 7: Prog	rammed Proje	cts			
Project Name	From / To Points:	Sponsor	GDOT PI#	ARC ID # (TIP)	Design FY	ROW / UTL FY	CST FY
Resurfacing – GDOT Group A (FC-9403)	Incl. Huff Road/Ellsworth Industrial Blvd.	City of Atlanta	N/A	ATLDOT: 1031	2018	N/A	2021
Cycle Atlanta Phase 1.0	Various Locations including West Marietta Street	City of Atlanta	0014993	<u>AT-277A</u>	2017	N/A	2022
Northside Drive Signal Updates	13 signals in the City of Atlanta and Georgia Tech Area	GDOT	0012823	<u>AT-287</u>	2014	2022	2022
Marietta TCC Combo	Incl. Chattahoochee Ave at Ellsworth Industrial Blvd	City of Atlanta	N/A	ATLDOT: <u>1053</u>	2020	N/A	2023
Atlanta Traffic Signal Enhancement Program – Phase 1	Various intersections on Greenbriar Pkwy, Sylvan Rd, 10 <sup>th</sup> St, State St, and North Ave	City of Atlanta	0017802	<u>AT-320</u>	2021	2024	2024
Howell Mill Complete Street	Marietta Street to Collier Road	City of Atlanta	N/A	ATLDOT: <u>1007</u>	2021	N/A	2024
Woodall Rail Trail	Elaine Avenue to Southland Circle/ Chattahoochee Ave	PATH Foundation/ UW CID	N/A	N/A	2022	2023	2024
Bus Stop Study	Route 14 along Ellsworth Industrial Boulevard	MARTA/ Upper Westside CID/LCI	N/A	N/A	Ongoing	N/A	N/A

<sup>\*</sup>Project information was obtained from GeoPI (GDOT), the Atlanta Region's Plan (ARC), ATLDOT Project.

Traffic signal upgrades from the Marietta TCC Combo project, highlighted in the table above, were considered in the analysis of the following intersections:

- Marietta at Huff/Kennesaw: Existing condition upgrade is installed as identified in the field, April 2022)
- Ellsworth at Chattahoochee: No-Build condition upgrade is nearly complete, but not operational as of April 2022

The following projects shown in **Table 8** are planned to occur near the development.

		Table 8: Pla	nned Projects			
Project Name	From / To Points:	Potential Sponsor	GDOT PI#	ARC ID # (TIP)	Project Timeline	Planning Document
Connect Cobb / Northwest Atlanta High-Capacity Premium Transit Service	Kennesaw State University / Midtown Atlanta	Cobb County	N/A	AR-475	2050	N/A
North Avenue Corridor High- Capacity Premium Transit Service	Marta North Avenue Station / MARTA Bankhead Rail Station	MARTA	N/A	AR-491B	2050	N/A
Marietta Blvd Complete Street – SCOPING STUDY	Donald Lee Hollowell Pkwy to Coronet Way	City of Atlanta	01107803	ATLDOT:3 058	TBD	N/A
BeltLine Upper Westside-Northside Trail (Subarea 8)	Area encompassing Huff Rd, Howell Mill Rd, Northside Drive	Atlanta BeltLine	N/A	N/A	TBD	Subarea 8 Master Plan (2012) – Plan update ongoing in 2022
Chattahoochee Avenue Multi-Use Path	Marietta Boulevard to Howell Mill Road	UW CID/ City of Atlanta	N/A	N/A	TBD	UWCID Master Plan (2021)

Available fact sheets for projects listed in the table above can be found in **Appendix D**.

#### 2.6 Level-of-Service Overview

Level-of-service (LOS) is used to describe the operating characteristics of a road segment or intersection in relation to its capacity. LOS is defined as a qualitative measure that describes operational conditions and motorists' perceptions within a traffic stream. The *Highway Capacity Manual* defines six levels-of-service, LOS A through LOS F, with A being the best and F being the worst. LOS analyses were conducted at all intersections within the study network using *Synchro 11*.

LOS for signalized intersections and all-way stop controlled intersections are reported for the intersection as a whole. One or more movements at an intersection may experience a low LOS while the intersection as a whole may operate acceptably.

LOS for unsignalized intersections with stop control on the minor street only is reported for the side street approaches and the major street left-turn movements. Low LOS for side street approaches is not uncommon, as vehicles may experience delays in turning onto a major roadway.

#### 2.7 Level-of-Service Standards

For the purposes of this traffic analysis, a LOS standard of D was assumed for all study intersections per section 3.2.2.1 of the GRTA *Development of Regional Impact Review Procedures* as specified in the LOU.

#### 3.0 Trip Generation

Gross trips associated with the proposed development were estimated using the *Institute of Transportation Engineers' (ITE) Trip Generation Manual, 11<sup>th</sup> Edition,* using equations where available. Reductions to gross trips including mixed-use reductions and alternative transportation mode reductions are considered in the analysis based on methodology outlined in the GRTA Letter of Understanding (LOU).

**Mixed-use reductions** occur when a site has a combination of different land uses that interact with one another. For example, people living in a residential development may walk to the restaurants and retail instead of driving offsite or to the site. This reduces the number of vehicle trips that will be made on the roadway, thus reducing traffic congestion. Mixed-use reductions were taken in this analysis per the LOU.

**Alternative modes reductions** are taken when a site can be accessed by modes other than vehicles (walking, bicycling, transit, etc.). Alternative modes reductions were taken in this analysis per the LOU.

**Pass-by reductions** are taken for a site when traffic normally traveling along a roadway may choose to visit a retail or restaurant establishment that is along the vehicle's path. These trips were already on the road and would therefore only be new trips on the driveways. Pass-by trips were taken for this analysis per the LOU.

**Table 9** summarizes the gross trip generation, reductions, net trip generation, and driveway volumes for the proposed *1359 Ellsworth* development.

		Table 9: T	rip Gene	ration								
l and llas	Donoity	D	aily Traffi	С	AM Pea	ık Hour	PM Pea	k Hour				
Land Use	Density	Total	Enter	Exit	Enter	Exit	Enter	Exit				
		Proposed	d Project	Trips								
140 – Manufacturing (Brewery) 12,500 SF 60 30 7 2 3 6												
221 - Multi-family Housing (Mid-Rise)	145 dwelling units	646	323	323	12	40	35	22				
222 - Multifamily Housing (High-Rise)	520 dwelling units	2,332	1,166	1,166	45	88	88	70				
932 - High-Turnover (Sit-Down) Restaurant	13,000 SF	1,394	697	697	68	56	72	46				
971 - Brewery Tap Room	12,000 SF	772	386	386	8	1	73	50				
Gross Projec	t Trips	5,204	2,602	2,602	140	187	271	194				
	Existir	ng Site Tr	ips (To Bo	e Remove	ed)							
150 - Warehousing	116,206 SF	222	111	111	29	9	11	29				
Net Project	4,982	2,491	2,491	111	178	260	165					
Mixe	-480	-240	-240	-18	-18	-39	-39					
Alternative	-746	-373	-373	-18	-28	-34	-23					
Pa	-746	-373	-373	0	0	-39	-39					
New Trip	os	3,010	1,505	1,505	75	132	148	64				

A more detailed trip generation analysis summary table is provided in **Appendix B**.

#### 4.0 TRIP DISTRIBUTION AND ASSIGNMENT

The distribution of new project trips was based on the project land uses, a review of land use densities and road facilities in the area, engineering judgement, and methodology discussions with GRTA, ARC, and other local stakeholders.

The anticipated distribution and assignment of the trips throughout the study roadway network for non-residential land uses is shown in **Figure 4.** The anticipated distribution and assignment of the trips throughout the study roadway network for residential land uses is shown in **Figure 5**. These trip assignment percentages were applied to the net project trips expected to be generated by the development, and the volumes were assigned to the roadway network. The peak hour project trips are shown by turning movement throughout the study network in **Figure 6**.

Detailed intersection volume worksheets are provided in Appendix C.

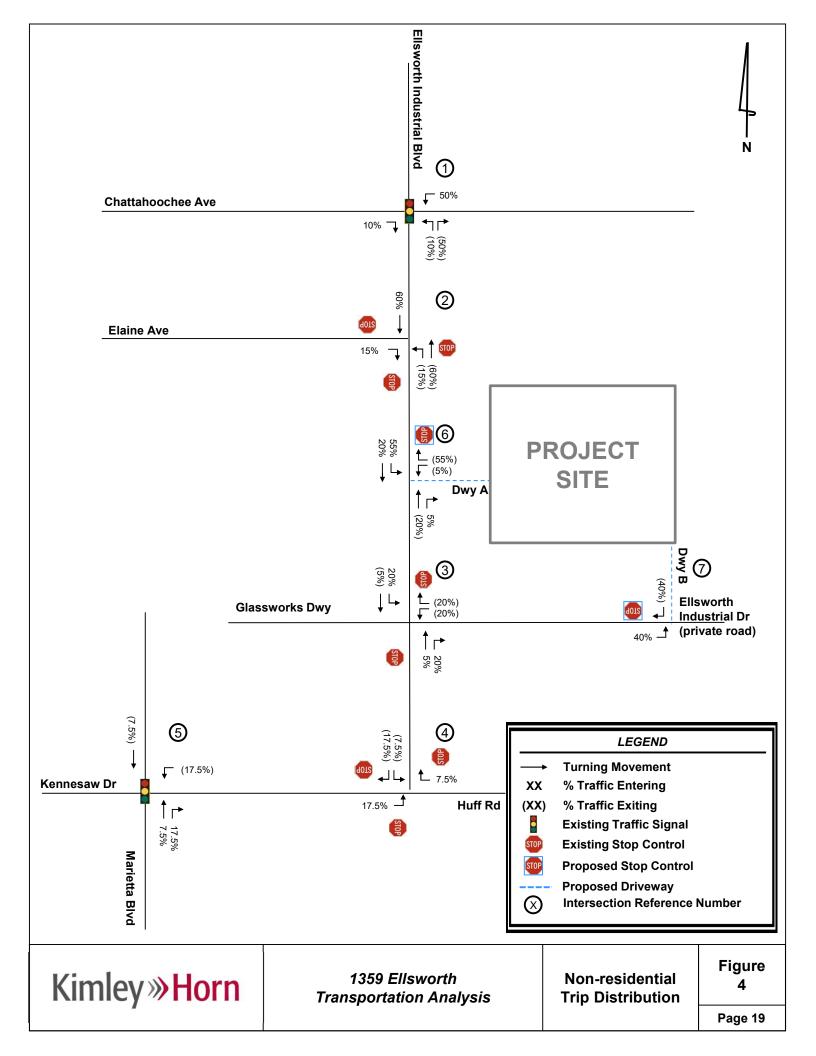
#### 5.0 TRAFFIC ANALYSIS

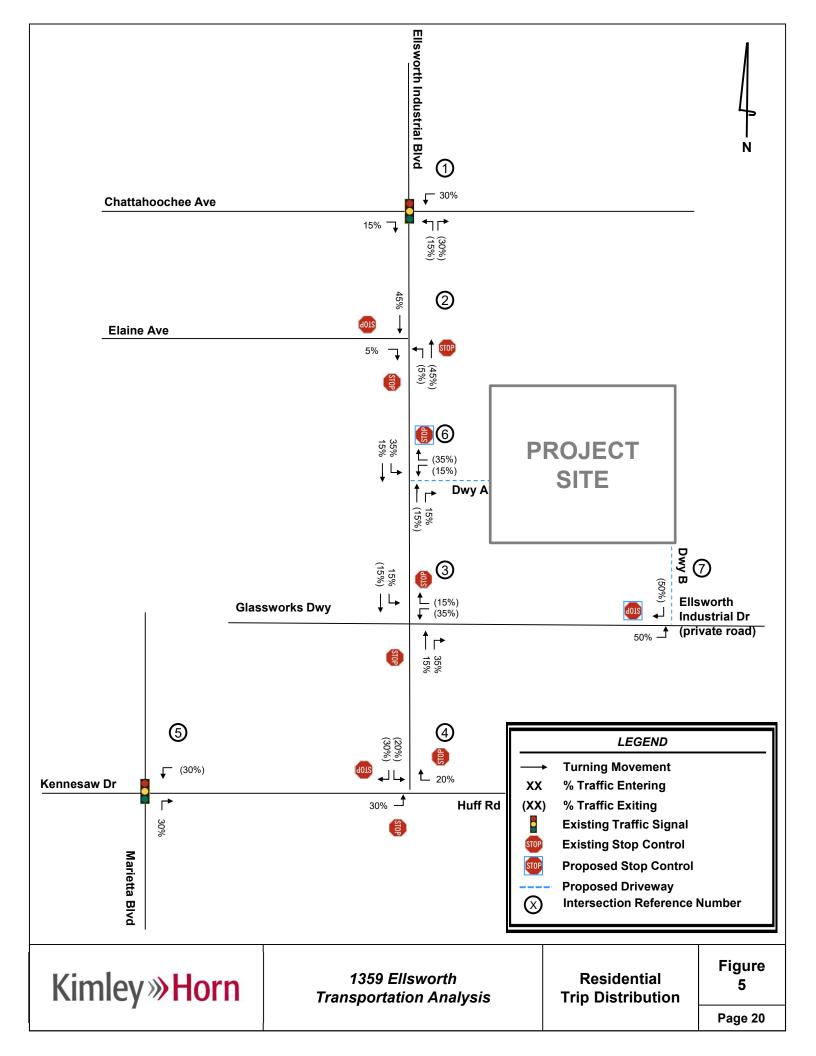
Capacity analyses were performed using *Synchro 11* for the AM and PM peak hours under the Estimated 2022 conditions, Projected 2025 No-Build conditions, and Projected 2025 Build conditions. The capacity analyses were performed using methodologies from the *Highway Capacity Manual (HCM)*, 6<sup>th</sup> Edition unless otherwise noted.

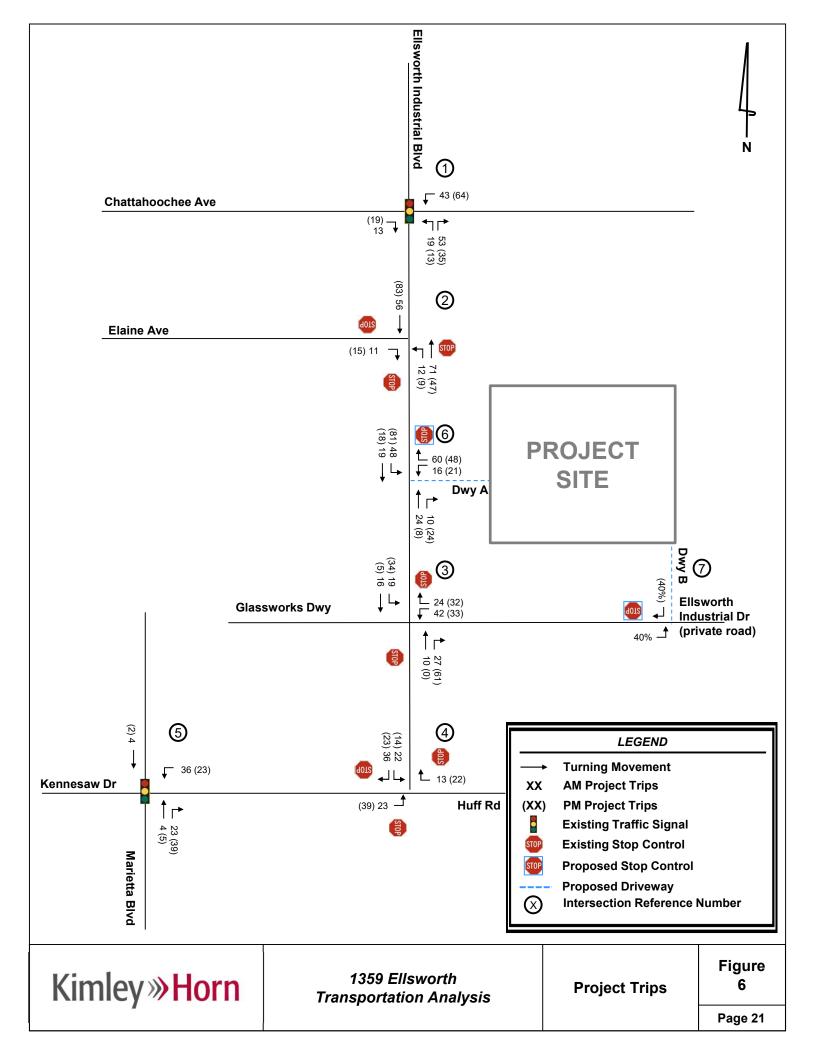
These analyses included existing roadway laneage for each of the scenarios. The traffic volumes and roadway laneage used for each scenario are shown in

**Figure 7** for Estimated 2022 conditions, **Figure 8** for Projected 2025 No-Build conditions, and **Figure 9** for Projected 2025 Build conditions.

**Sections 5.1 – 5.7** provide the results of the capacity analyses are presented for each study intersection and include projected LOS, delay, and queue lengths.







## 5.1 Ellsworth Industrial Boulevard at Chattahoochee Avenue (Intersection 1)

Ove	erall L (	OS Standard: D	_	orth Ind		Ellsworth Industrial			Chattahoochee			Chattahoochee		
		LOS Standard: D		<u>Bouleval</u>			Boulevar			Avenue			Avenue	
• • •				lorthbou		. 5	outhbou			astbour		, V	/estboun	
			L	Т	R	L	Т	R	L	Т	R	<u> </u>	Т	R
		Overall LOS				ſ	<u> </u>	F (11	1.6)				_ ,	
۵ ا	_	Approach LOS		E (60.4)	)		C (28.6)			C (25.2	)		F (221.9)	)
핃	AM	Storage	50			175								
[ <u>`</u>		50th Queue	52	260		11	2			406			701	
Į₽Ž		95th Queue	97	487		39	18			558			840	
2022 ESTIMATED (SIGNAL)		Overall LOS				C (22.			2.1)					
(\$		Approach LOS		C (30.8	)		E (67.6)			A (8.5)			B (18.9)	
02	Δ	Storage	50			175								
• • •		50th Queue	75	17		55	19			149			551	
		95th Queue	131	93		136	55			1650			2322	
		Overall LOS						F (13	3.4)					
		Approach LOS		E (65.0	)		C (28.4)	)		D (43.1	)		F (268.0)	)
9	AM	Storage	50			175								
	`	50th Queue	54	292		12	2			476			756	
A ₹		95th Queue	101	533		45	19			665			897	
2025 NO-BUILD (SIGNAL)		Overall LOS						C (27	Ź					
25 (S		Approach LOS		C (30.9	)		F (98.6)			A (8.8)			C (24.6)	
20.	Δ	Storage	50			175								
	_	50th Queue	78	22		66	20			77			274	
		95th Queue	136	103		155	57			164			616	
		Overall LOS				•	•	F (15	7.6)		•		•	
		Approach LOS		F (89.2)	)		C (28.2)	,		E (56.6	)		F (291.1)	)
	AM	Storage	50			175								
2025 BUILD (SIGNAL)	'	50th Queue	66	397		12	2			551			790	
l≳₹		95th Queue	119	623		46	19			691			930	
55 1		Overall LOS						D (38	3.8)					
(\$	_	Approach LOS		C (31.1	)		F (112.3	)		A (9.7)			D (47.6)	
,,	PM	Storage	50			175								
		50th Queue	87	41		69	20			92			384	
*14		95th Queue	148	137		164	57			169			688	

<sup>\*</sup>Intersection was analyzed with HCM 2000.

The signalized intersection of Chattahoochee Avenue at Ellsworth Industrial Boulevard (Intersection 1) is projected to operate at an acceptable <u>overall</u> LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour. The intersection is projected to operate at an unacceptable LOS for the southbound approach under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the PM peak hour.

In order to meet GRTA's LOS requirements under the Estimated 2022 conditions, the system improvements listed below are needed (shown in red on **Figure 7**):

- Widen the westbound approach along Chattahoochee Avenue to add one (1) left-turn lane, so that it consists of one (1) left-turn lane, one (1) through lane, and one (1) shared through/right-turn lane.
- Widen the eastbound approach along Chattahoochee Avenue to add one (1) left-turn lane and one (1) right-turn lane, so that it consists of one (1) left-turn lane, two (2) through lanes, and one (1) right-turn lane.

It should be noted that the Chattahoochee Avenue Multi-Use Path is planned along the south side of Chattahoochee Avenue (noted in **Table 8**). Consider if the addition of turn lanes aligns with the goals for future multimodal planned improvements along Chattahoochee Avenue.

The analysis results shown in the table below are for the improved conditions at Ellsworth Industrial Boulevard at Chattahoochee Avenue (Intersection 1), which assume the noted geometric changes.

Ove	erall L0	OS Standard: D	Ellsworth Industrial Boulevard			Ellsworth Industrial Boulevard			Chattahoochee Avenue			Chattahoochee Avenue		
Appr	oach l	_OS Standard: D		lorthbou	_		outhbou		F	astbour		V	Vestbour	
			L	T	R	L	T	R	L	T	R	L	T	R
		Overall LOS						D (43	.9)					
		Approach LOS		D (54.5	)		D (47.3)	,		D (40.4	)		D (43.9)	ı
2022 ESTIMATED (SIGNAL)	AM	Storage	50			175				,				
A (	'	50th Queue	64		293	18		4	15	460	466	348	142	135
₽ĕ		95th Queue	117		441	30		12	25	642	649	533	244	231
ESTIMA (SIGNAL)		Overall LOS						B (15	.0)					
2 E (S		Approach LOS		C (33.1	)		C (33.3)			B (15.3	)		A (9.1)	
02	PM	Storage	50			175								
~		50th Queue	85		49	52		35	5	103	102	87	88	86
		95th Queue	152		90	94		61	8	188	185	156	161	155
		Overall LOS						D (44	.0)					
	_	Approach LOS		D (50.5	)		D (47.1)			D (54.8	)		C (31.9)	
9	AM	Storage	50			175								
2 (		50th Queue	70		227	18		4	15	563	571	451	137	130
N A		95th Queue	126		356	33		12	28	764	775	639	236	224
2025 NO-BUILD (SIGNAL)		Overall LOS						B (15	.6)					
25 (\$	_	Approach LOS		C (33.4	)		C (33.9)			B (15.5	)		A (9.8)	
20	ЬМ	Storage	50			175								
		50th Queue	88		58	55		35	5	112	110	100	94	94
		95th Queue	161		107	100		65	8	201	198	182	172	168
		Overall LOS						D (49						
	_	Approach LOS		D (50.2	)		D (46.6)			D (50.9	)		D (48.9)	
ے ۵	AM	Storage	50			175								
≒ (}		50th Queue	82		285	18		4	18	511	35	449	161	152
2025 BUILD (SIGNAL)		95th Queue	149		430	33		84	30	707	64	673	268	254
25 SIG		Overall LOS Approach LOS		C (34.1	١	1	D (35.6)	B (16					B (11.9)	
20	PM		50	C (34.1	)	175	D (33.6)			B (14.4	)		<u>Б (11.9)</u> Г	
		Storage 50th Queue	99		85	58		35	5	87	38	137	94	94
		95th Queue	175		153	103		65	8	158	67	238	172	168
		Join Queue	173		100	103		UJ	U	100	01	230	112	100

## 5.2 Ellsworth Industrial Boulevard at Elaine Avenue (Intersection 2)

		S Standard: D OS Standard: D		worth Indu Boulevard		Ellsworth Industrial Boulevard			Ela	aine Aven	ue		-	
			1	Northboun		S	outhbour		Е	astboun		V	/estbour	
			L	Т	R	L	Т	R	L	T	R	L	Т	R
		Overall LOS						C (2	23.4)					
	_	Approach LOS		D (28.9)			C (18.9)			C (20.8)				
	PΑ	Storage												
l ⊵ຄ		50th Queue												
NS(		95th Queue		193			117		102		5			
2022 ESTIMATED (AWSC)		Overall LOS						B (1	4.1)					
2 5	_	Approach LOS		B (11.7)			C (15.9)			B (13.2)				
502	Σ	Storage												
•		50th Queue												
		95th Queue		46			119		43		3			
		Overall LOS						D (2	28.2)					
	_	Approach LOS		E (36.9) C (21.9) C (23.3)										
	ΔA	Storage												
<u>                                   </u>		50th Queue												
9-B		95th Queue		241			142		117		5			
2025 NO-BUILD (AWSC)		Overall LOS		C (15.1)										
25	_	Approach LOS		B (12.2)			C (17.4)			B (13.7)				
20	P	Storage												
		50th Queue												
		95th Queue		51			135		48		3			
		Overall LOS						F (5	2.7)					
		Approach LOS		F (83.9)			D (34.2)			D (25.5)				
	AM	Storage												
l ⊒ ♡		50th Queue												
BU /SC		95th Queue		442			224		127		8			
2025 BUILD (AWSC)		Overall LOS						C (2	21.3)					
50%		Approach LOS		B (14.7)			D (27.8)			B (14.3)				
	Ā	Storage												
		50th Queue												
		95th Queue		76			229		51		5			

The intersection of Ellsworth Industrial Boulevard at Elaine Avenue (Intersection 2) is projected to operate at an acceptable <u>overall</u> LOS under the Estimated 2022 and 2025 No-Build conditions. The northbound approach is projected to operate at LOS E under the 2025 No-Build conditions during the AM peak hour.

It should be noted that although the delays are projected to operate at unacceptable levels per GRTA's LOS requirements, the delays are not extreme.

In order to meet GRTA's LOS requirements under the 2025 No-Build conditions, the <u>system improvements</u> listed below are needed (shown in green on **Figure 8**):

- Restripe the northbound approach along Ellsworth Industrial Boulevard, so that it consists of one (1) shared through/left-turn lane and one (1) through lane.
- Restripe the receiving lane on the north leg along Ellsworth Industrial Boulevard, so that it consists of two (2) northbound receiving lanes.

The intersection is projected to operate at an unacceptable <u>overall</u> LOS under the 2025 Build conditions without proposed system improvements to the northbound approach. However, with the addition of the proposed 2025 No-Build system improvements, including an increase in northbound approach and receiving lanes, the southbound approach under 2025 Improved Build conditions requires mitigation to operate acceptably under GRTA's LOS requirements with the following Build improvement listed below (shown in blue on **Figure 9**):

Reconfigure the southbound approach along Ellsworth Industrial Boulevard, so that it consists of one
 (1) right-turn lane and one
 (1) through lane.

With the <u>system improvements</u> identified to mitigate low LOS for the northbound approach in the 2025 No-Build conditions and the system improvement identified to mitigate low LOS for the southbound approach in the 2025 Build Improved conditions, Intersection 2 would meet GRTA's LOS requirements both under 2025 Improved No-Build and 2025 Improved Build conditions.

However, it is not uncommon for stop-controlled intersections to operate with low LOS and delay. Potential improvements to the stop-controlled intersection should consider pedestrian activity, which is likely to increase at this intersection with the installation of the programmed PATH Trail and the potential future BeltLine alignment that may also interact with this intersection. A traffic signal is not likely to be warranted at this intersection based on existing or future vehicular peak hour volumes. Depending on future pedestrian activity associated with the future trail projects, a signal may be pursued to improve pedestrian crossing at the intersection but has not been considered for this DRI.

The analysis results shown in the table below are for the improved conditions at Ellsworth Industrial Boulevard at Elaine Avenue (Intersection 2), which assume the noted geometric changes.

		OS Standard: D OS Standard: D	Ellsworth Industrial Boulevard			_	Ellsworth Industrial Boulevard			Elaine Avenue			-	
			1	Northboun	ıd	So	outhboun	id	E	Eastbound	d	W	estboun	d
-			L	T	R	L	Т	R	L	Т	R	L	Т	R
		Overall LOS						C (2	20.4)					
		Approach LOS		C (16.3)			C (23.8)			C (22.3)				
9	AM	Storage												
ΙΞΩ	,	50th Queue												
-B-C		95th Queue	44	91			152		112		5			
2025 NO-BUILD (AWSC)		Overall LOS						C (1	16.6)					
25		Approach LOS		B (10.6)			C (21.2)			B (13.8)				
203	PM	Storage												
		50th Queue												
		95th Queue	20	25			163		48		3			
		Overall LOS						C (1	19.5)					
		Approach LOS		C (20.4)			C (15.5)			C (23.5)				
	AM	Storage												
	'	50th Queue												
		95th Queue	61	145			94	65	119		8			
2025 BUILD (AWSC)		Overall LOS						B (1	3.1)					
60 >		Approach LOS		B (11.6)			B (13.6)			B (14.1)				
.,	Storage													
	50th Queue													
		95th Queue	29	38			100	65	51		5			

## 5.3 Ellsworth Industrial Boulevard at Ellsworth Industrial Drive (Intersection 3)

		OS Standard: D OS Standard: D	Ells	worth Industrial Boulevard		n Industrial levard	Glasswor Drivewa		Ellsw	ustrial	
пррго	aon L	OO Clandara. B		Northbound		nbound	Eastbou		W	Drive estbour	nd
			L	TR	L	T R	L T	R	L	T	R
		Overall LOS		<u>'</u>		A (1.1	)				
		Approach LOS		A (0.3)	Α (	(0.2)	C (15.4	.)		B (13.6)	
2022 ESTIMATED (TWSC)	Α	Storage									
I Σ	_	50th Queue									
		95th Queue	3				8		3		
ESTIMA (TWSC)		Overall LOS				A (1.4	<b>l</b> )				
2 E	_	Approach LOS		A (0.1)	Α (	(0.1)	B (12.7	)		B (12.5)	
02	Δ	Storage									
,,		50th Queue									
		95th Queue	0				11			0	
		Overall LOS			1	A (1.1					
	_	Approach LOS		A (0.3)	Α (	(0.2)	C (15.8	)		B(13.9)	
2	ΑM	Storage									
٦Ξ		50th Queue									
)-E		95th Queue	3				8			3	
2025 NO-BUILD (TWSC)		Overall LOS			1	A (1.5			1		
25	_	Approach LOS		A (0.1)	Α (	(0.1)	B (13.0	)		B (12.8)	
20	P	Storage									
		50th Queue									
		95th Queue	0				11			0	
		Overall LOS			1	A (3.2			1		
	_	Approach LOS		A (0.3)	Α (	(0.9)	C (18.4	.)		C (24.1)	
٥	AM	Storage									
ا ⊒ ش		50th Queue									
BL VS(		95th Queue	3				11			40	
2025 BUILD (TWSC)		Overall LOS		A (0.4)	1 .	A (3)			1	D (45.5)	
20	5	Approach LOS		A (0.1)	Α (	(1.1)	C (15.4	.)		B (13.9)	
	PM	Storage									
		50th Queue					4.4			40	
		95th Queue	0				14			13	

The intersection of Ellsworth Industrial Boulevard at Ellsworth Industrial Drive/Glassworks Drive (Intersection 3) is projected to operate at an acceptable <u>overall</u> LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions. Each approach of the intersection is projected to operate acceptably under all studied scenarios. No improvements are recommended to be conditioned.

## 5.4 Ellsworth Industrial Boulevard at Huff Road (Intersection 4)

		OS Standard: D OS Standard: D		-			orth Indu Boulevard		Н	uff Roa	d	Huff Road			
				Northboun		S	outhbour		E	astboun		W	estbour		
			L	Т	R	L	Т	R	L	Т	R	L	Т	R	
		Overall LOS						(71.7)			,				
0	_	Approach LOS		,			B (14.4)		F	(117.2	)		(21.5)	,	
lΞ	PΑ	Storage													
⊈ເວ		50th Queue													
NS I		95th Queue					43			702			153		
2022 ESTIMATED (AWSC)		Overall LOS						3 (13.9)							
22	_	Approach LOS					B (12.6)		l	B (13.7)	)	Е	3 (15.0)		
502	A	Storage													
• •		50th Queue													
		95th Queue					53			76			102		
		Overall LOS		F (87.7)											
	_	Approach LOS		,			B (14.9)		F	(145.3	5)	(	(24.1)	,	
	ΑM	Storage													
] <u>5</u> 0		50th Queue													
1 SS		95th Queue					48			819			174		
2025 NO-BUILD (AWSC)		Overall LOS					E								
25	_	Approach LOS					B (13.3)		l	B (14.5)			(16.3)		
20	Ā	Storage													
		50th Queue													
		95th Queue					58			84			115		
		Overall LOS					F	(112.4)	)						
	_	Approach LOS		,			C (18.2)		F	(192.4	.)		) (31.1)	,	
	ΑM	Storage													
⊒ີ ເ		50th Queue													
BU VS(		95th Queue					74			974			214		
2025 BUILD (AWSC)		Overall LOS						C (17.9)				1			
50	_	Approach LOS					C (15.6)		(	C (17.9)	)	(	(19.5)		
	P	Storage													
		50th Queue													
		95th Queue					81			117			145		

The intersection of Ellsworth Industrial Boulevard at Huff Road (Intersection 4) is projected to operate at an unacceptable <u>overall LOS</u> under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour. The eastbound approach of the intersection is projected to operate at an unacceptable LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour.

In order to meet GRTA's LOS requirements under the Estimated 2022 conditions, the <u>system improvements</u> listed below are needed and recommended (shown in red on **Figure 7**):

- Widen the eastbound approach along Huff Road to add one (1) left-turn lane, so that it consists of one
   (1) left-turn lane and one (1) through lane.
  - Note: Right-of-way may not be available to accommodate the proposed widening for a left-turn lane
- Install a traffic signal if and when it is warranted and approved by the City of Atlanta
  - The intersection was modified from side-street stop-control to an all-way stop-control (AWSC) in 2021 following a study by the Upper Westside CID. The study concluded that a signal warrant was possible, but that AWSC was an appropriate interim solution for a future signal as a quick response due to sight distance concerns. The study concluded that a signal may be warranted based on Warrant 1 (8 hour warrant) and Warrant 2 (4 hour warrant), but Warrant 3 (peak hour warrant) was not satisfied. A review of the AM and PM peak hours considered for this DRI indicates that a signal may be warranted.

The analysis results shown in the table below are for the improved conditions at Ellsworth Industrial Boulevard at Huff Road (Intersection 4), which assume the noted geometric changes.

Overall LOS Standard: D Approach LOS Standard: D			-			Ellsworth Industrial Boulevard			Huff Road			Huff Road			
			Northbound			Sc	Southbound			Eastbound			Westbound		
			L	T	R	L	T	R	L	Т	R	L	T	R	
		Overall LOS						3 (13.2)							
	_	Approach LOS					B (19.2)		I	B (11.6)	)	I	3 (13.2)	i	
2022 ESTIMATED (AWSC)	Α	Storage													
∣≨ຄ	,	50th Queue				79			74	87				122	
l ≧S		95th Queue				146			135	158				211	
ESTIMA (AWSC)		Overall LOS		B (19.2)											
2 E	_	Approach LOS					B (11.0)			C (22.7)			C (22.0)		
502	Ā	Storage													
''		50th Queue				76			48	73				147	
		95th Queue				134			84	133				249	
		Overall LOS		B (13.0)											
	_	Approach LOS					C (20.6)		I	B (11.2)	)	I	3 (12.6)	i	
2	Α	Storage													
] <u>5</u> (5)	,	50th Queue				88			79	84				122	
		95th Queue				146			135	158				211	
2025 NO-BUILD (AWSC)		Overall LOS	B (14.3)												
25	_	Approach LOS					B (10.9)			B (17.8)	)	l l	3 (13.8)	i	
20	Ā	Storage													
		50th Queue				55			33	54				79	
		95th Queue				134			84	133				249	
		Overall LOS						3 (10.8)							
	_	Approach LOS					C (31.9)			A (3.9)		I	3 (10.6)	i	
	ΑM	Storage													
⊒ິເ	,	50th Queue				149			30	14				107	
30 /S(		95th Queue				158			145	152				213	
2025 BUILD (AWSC)		Overall LOS						3 (19.0)							
503	_	Approach LOS			1		B (11.9)	•	(	C (22.1)	)	(	C (21.3)	1	
, ,	P	Storage													
		50th Queue				82			48	76				152	
		95th Queue				149			89	136				254	

## 5.5 Marietta Boulevard at Huff Road/Kennesaw Drive (Intersection 5)

Overall LOS Standard: D			Mari	etta Boule	Marietta Boulevard			Keni	nesaw [	Drive	Huff Road			
Approa	ach L	OS Standard: D	1	Northboun		Sc	outhbour		Eastbound			W	estbour	
			L	T	R	L	T	R	L	Т	R	L	Т	R
		Overall LOS					D (46.0)							
	_	Approach LOS		B (17.0)	E (65.3)			F (82.2)			F (81.3)			
Ä	Α	Storage			200									
I ₹ ≘		50th Queue		241	17		692			27			312	
2022 ESTIMATED (Signal)		95th Queue		308	77		852			62			483	
Sig		Overall LOS		C (25.2)										
2 E		Approach LOS		C (20.2)			C (29.3)			D (44.5)		C (28.9)		
:02	Σ	Storage			200									
8		50th Queue		144	0		174			1			199	
		95th Queue		207	36		265			15			442	
		Overall LOS		F (81.4)										
		Approach LOS		B (18.8)		F	(146.3)	)		F (85.2)		F	F (81.1)	
Q.	Α	Storage			200									
	_	50th Queue		302	44		903			29			322	
J-B na		95th Queue		367	121		1045			65			517	
2025 NO-BUILD (Signal)		Overall LOS				D (47.5)								
25		Approach LOS		C (23.3)			F (87.4)			D (44.5)			(28.3)	
203	₽	Storage			200									
		50th Queue		196	0		252			1			202	
		95th Queue		261	37		397			15			469	
		Overall LOS					F	(94.3)						
	_	Approach LOS		C (21.0)		F	(175.7)	)		F (85.2)		F	(81.5)	
_	ΑM	Storage			200									
		50th Queue		305	47		912			29			373	
3UI na		95th Queue		370	130		1055			65			604	
2025 BUILD (Signal)		Overall LOS						(48.7)	)					
202 (3		Approach LOS		C (23.3)			F (91.3)			D (44.5)			(30.0)	
,,	₽	Storage			200									
		50th Queue		200	0		257			1			216	
		95th Queue		263	40		400			15			504	

<sup>\*</sup>Intersection was analyzed with HCM 2000.

The intersection of Marietta Boulevard at Huff Road/Kennesaw Drive (Intersection 5) is projected to operate at an unacceptable <u>overall</u> LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour. The eastbound and westbound approaches of the intersection are projected to operate at an unacceptable LOS under the Estimated 2022, No-Build 2025, and Build 2025 conditions during the AM peak hour. The southbound approach is projected to operate at an unacceptable LOS under the No-Build 2025 and Build 2025 conditions during the PM peak hour.

In order to improve the delay experienced under the Estimated 2022 conditions, the <u>system improvements</u> listed below are needed (shown in red on **Figure 7**):

 Widen the southbound approach along Marietta Boulevard to add one (1) left-turn lane, so that it consists of one (1) left-turn lane, one (1) through lane, and one (1) shared through/right-turn lane

The analysis results shown in the table below are for the improved conditions at Marietta Boulevard at Huff Road/Kennesaw Drive (Intersection 5), which assume the noted geometric changes.

Overall LOS Standard: D			Mari	etta Boule	evard	Marie	etta Boule	evard	Keni	nesaw D	Drive	Huff Road			
Appro	ach L	OS Standard: D	Northbound			Southbound			Eastbound			Westbound			
			L	T	R	L	Т	R	L	Т	R	L	T	R	
		Overall LOS													
		Approach LOS		C (28.6)			C (32.4)			F (82.2)			F (81.3)		
2022 ESTIMATED (Signal)	AM	Storage			200										
I≦≘		50th Queue		311	22	123	292			27			312		
ESTIMA (Signal)		95th Queue		381	102	254	366			62			483		
Sig		Overall LOS		C (30.5)											
2 E	_	Approach LOS	D (39.3)			C (20.5)				D (44.5)	)	C (28.9)			
502	PM	Storage			200										
(4		50th Queue		202	0	28	126			1			199		
		95th Queue		279	48	58	182			15			442		
		Overall LOS		D (36.6)											
	_	Approach LOS		D (35.7)			C (24.2)			F (85.2)		F	(81.1)		
9	A	Storage			200										
l ⋽ <u>~</u>		50th Queue		374	54	167	374			29			322		
na na		95th Queue		545	156	341	447			65			517		
2025 NO-BUILD (Signal)		Overall LOS						D (47.9)	)						
25	_	Approach LOS		E (76.0)			C (22.9)			D (44.5)	)		(28.3)		
20	PM	Storage			200										
		50th Queue		268	0	30	164			1			202		
		95th Queue		396	49	60	220			15			469		
		Overall LOS					I	D (39.9)	)						
	_	Approach LOS		D (36.6)			C (29.7)			F (85.2)		F	(81.5)		
	AM	Storage			200										
] = =		50th Queue		376	59	172	376			29			373		
3U Jna		95th Queue		456	167	346	450			65			604		
2025 BUILD (Signal)		Overall LOS						D (38.7)	)						
502		Approach LOS		D (53.9)			C (21.2)			D (44.5)		C	(34.3)		
, ,	PM	Storage			200										
		50th Queue		261	0	29	158			1			225		
		95th Queue		377	52	57	210			15			527		

<sup>\*</sup>Intersection was analyzed with HCM 2000.

Although the eastbound and westbound approaches are projected to operate at LOS E or F, no feasible improvements exist. The failing LOS is due to the existing signal timing. Kennesaw Drive and Huff Road are offset from one another. Due to the intersection configuration, these approaches are currently programmed to run split timings, which decreases the amount of time the approaches are serves. It is notable that Kennesaw Drive has minimal volume during the peak hours. Kennesaw Drive serves a site that has alternative access at a signalized intersection along Marietta Boulevard located approximately 1,200 feet north of the intersection of Marietta Boulevard at Huff Road/Kennesaw Drive.

No improvement is recommended without further study of intersection geometry constraints.

## 5.6 Ellsworth Industrial Boulevard at Driveway A (Intersection 6)

Overall LOS Standard: D Approach LOS Standard: D		Ellsworth Industrial Boulevard			Ellsworth Industrial Boulevard				-		Driveway A				
			N	lorthbour	nd	So	Southbound			astbour	nd	Westbound			
			L	T	R	L	Т	R	L	Т	R	L	Т	R	
		Overall LOS		A (1.9)											
		Approach LOS		A (0.0)		A (1.5)						C (16.5)			
	ΑM	Storage													
BUILD VSC)		50th Queue													
SC SC		95th Queue				6							24		
(TWSC)		Overall LOS		A (2.2)											
2025 (TV		Approach LOS		A (0.0)			A (1.9)					I	3 (12.9)		
•	Δ	Storage													
		50th Queue													
		95th Queue				6							13		

The intersection of Ellsworth Industrial Boulevard at Driveway A (Intersection 6) is projected to operate at an acceptable LOS under the Build 2025 scenario. Each approach of the intersection is projected to operate acceptably under all studied scenarios.

The recommended lane configuration for Driveway A is one lane entering the site and one lane exiting the site, as shown in the site plan. The recommended build improvements are shown in **Figure 9.** 

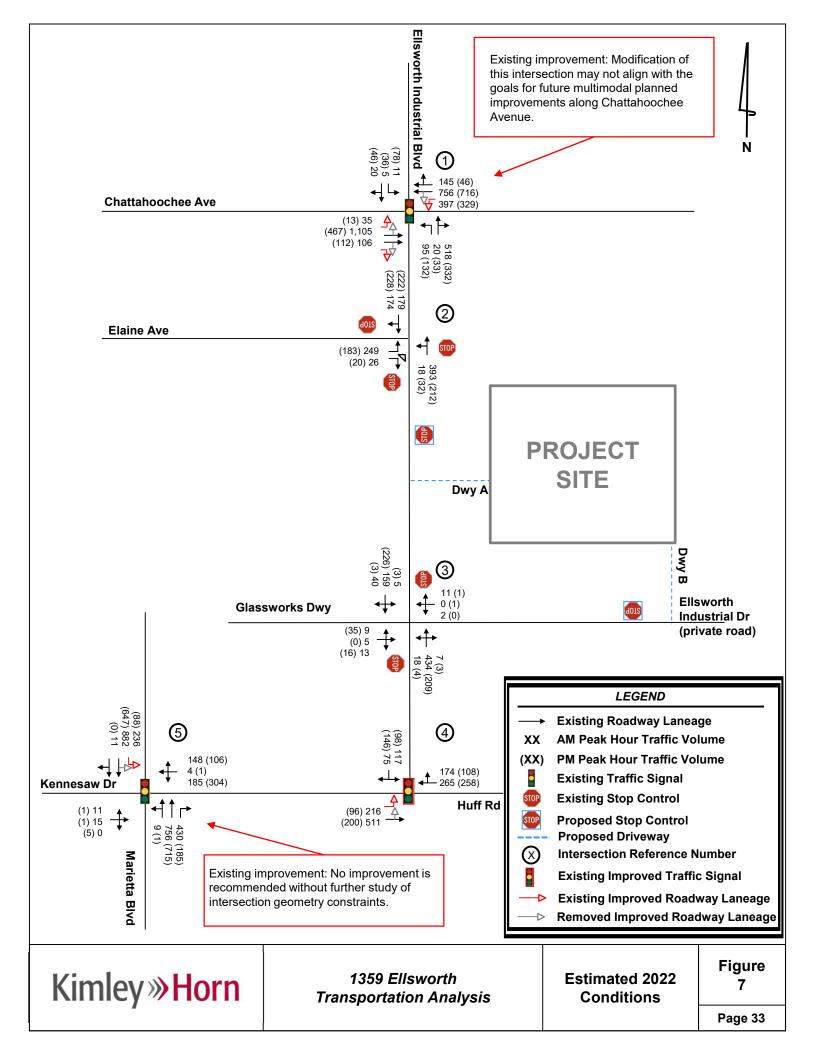
In addition to vehicular access improvements, in the vicinity of Intersection 6, a proposed relocation of MARTA Bus Stop #902230 as discussed in the Methodology Meeting should be considered in coordination with the MARTA and the City of Atlanta. The current bus stop is located approximately 250 feet north of the site where no sidewalks exist for pedestrian access. The proposed relocation would allow the bus stop to be connected to new sidewalks along the project site and tie into existing sidewalks that extend to Huff Road.

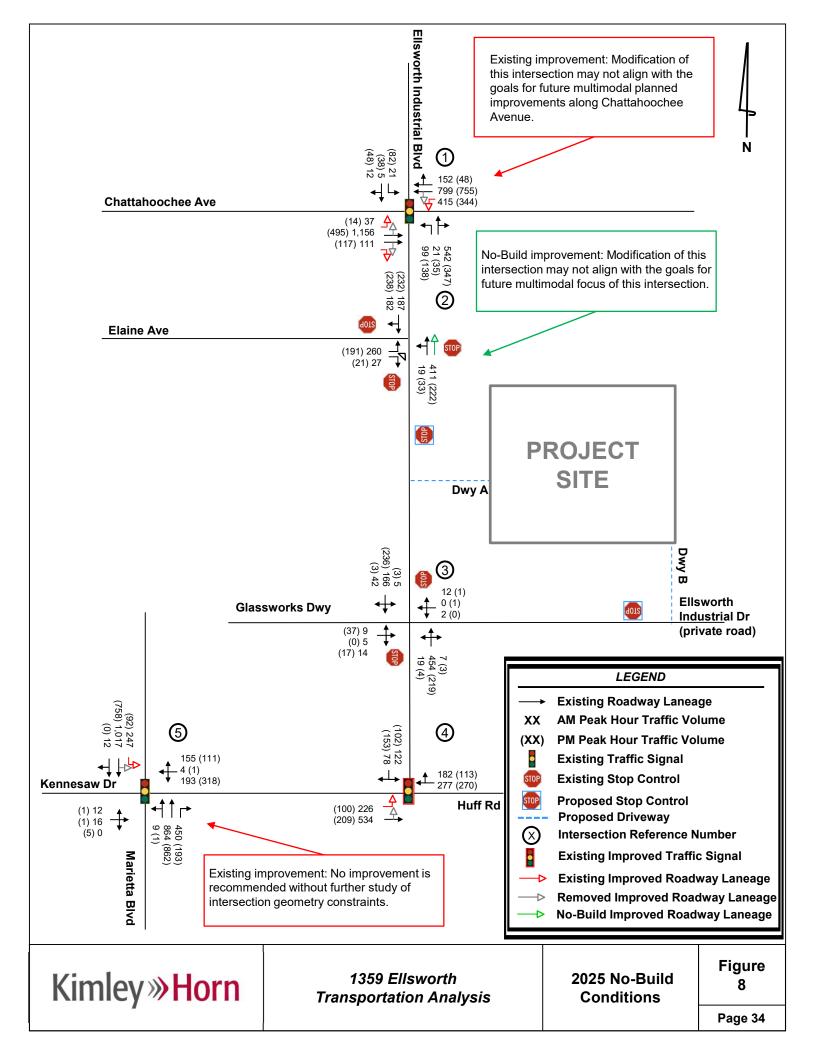
## 5.7 Ellsworth Industrial Drive at Driveway B (Intersection 7)

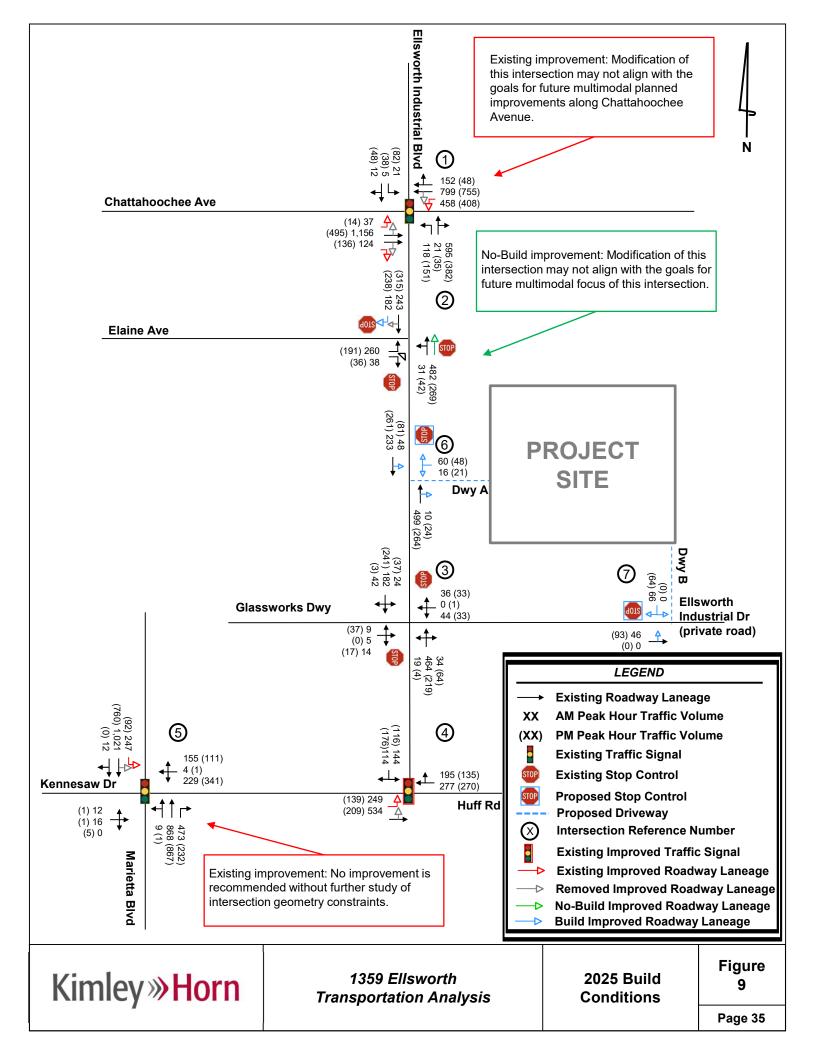
Overall LOS Standard: D Approach LOS Standard: D			- Northbound			Driveway B Southbound			Ellsworth Industrial Drive Eastbound			Ellsworth Industrial Drive				
_			IN.	ortnbour		50	outhbou			astboun		Westbound				
			L	Т	R	L	T	R	L	T	R	L	T	R		
		Overall LOS		A (8.0)												
	_	Approach LOS					A (8.6)		A (7.3)			A (0.0)				
	AM	Storage														
ا تا <sup>(</sup> (		50th Queue														
25 BUILD (TWSC)		95th Queue				6			3							
		Overall LOS					A (7.									
2025 (TV		Approach LOS					A (8.6)			A (7.4)			A (0.0)			
•	ΡМ	Storage														
		50th Queue														
		95th Queue				6			5							

The intersection of Ellsworth Industrial Drive at Driveway B (Intersection 5) is projected to operate at an acceptable LOS under the Build 2025 scenario. Each approach of the intersection is projected to operate acceptably under all studied scenarios.

The recommended lane configuration for Driveway B is one lane entering the site and one lane exiting the site, as shown in the site plan. The recommended build improvements are shown in **Figure 9.** 







# Proposed Site Plan

TOTAL SITE AREA	5.019 ACRES
NET LOT AREA (NLA)	218,621 SF
GROSS LOT AREA (GLA)	229,561 SF
EXISTING ZONING	I-2 (HEAVY INDUSTRIAL) BELTLINE OVERLAY
PROPOSED ZONING	MRC-3 (MIXED RESIDENTIAL COMMERCIAL) BELTLINE OVERLAY
PROPOSED USE	MULTI-FAMILY/COMMERCIAL
PROPOSED BUILDINGS	682,400 SF
COMMERCIAL (BUILDINGS A, B, C)	13,000 SF
BREWERY (BUILDING D)	24,520 SF
TAP ROOM	12,260 SF
MANUFACTURING	12,260 SF

INTERSECTION 2

SITE	DATA
MULTI-FAMILY (BUILDINGS A, B, C)	
MAXIMUM BUILDING AREA	644,880 GSF
MAXIMUM BUILDING HEIGHT	225'-0"
MAXIMUM RESIDENTIAL UNITS	665
RESIDENTIAL FLOOR AREA RATIO (FAR)	
MAX ALLOWED	699,587 SF (3.2 x NLA)
PROVIDED	644,880 SF (2.95 x NLA)
BUILDING COVERAGE	
MAX ALLOWED	85% NLA
NOT TO EXCEED	85% NLA
OPEN SPACE REQUIREMENTS	124,614 SF
RESIDENTIAL USES	0.57 x NLA (PER TABLE I)
OPEN SPACE PROVIDED	SHALL MEET REQUIREMENTS

MINIMUM PARKING REQUIRED PER CODE		443 SPACES
RESIDENTIAL USE (PER TABLE I)	366 SPACES	(0.55 x UNIT)
COMMERCIAL	22 SPAC	CES (1/600 SF)
EATING/DRINKING >60% ALCOHOL SALES	55 SPAC	CES (1/450 SF)
SHARED PARKING PROVIDED (4-LEVEL PARKING D	ECK)*	840 SPACES
LOADING SPACES REQUIRED		3 - 12'x35' SPACE
LOADING SPACES PROVIDED (INSIDE PARKING ST	RUCTURE)	3 SPACES
* NOTE - DROVEDED DADVING WILL NOT EVOLED A		

\* NOTE: PROVIDED PARKING WILL NOT EXCEED MAXIMUM ALLOWABLE PARKING PER BELTLINE OVERLAY REQUIREMENTS.



# LOCATION MAP

## PROJECT CONTACTS

OWNER/CLIENT
STREAM REALTY ACQUISITIONS, LLC 1180 WEST PEACHTREE STREET NW, SUITE 500 ATLANTA, GA 30329 CONTACT: BEN HAUTT PHONE: 404.962.8601 EMAIL: bhautt@streamrealty.com

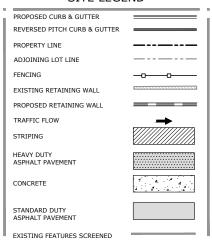
#### **CIVIL ENGINEER**

GENESIS ENGINEERING COLLABORATIVE 1325 SATELLITE BLVD NW, SUITE 202 SUWANEE, GA 30024 CONTACT: JENNIFER LOSURDO, PE PHONE: 470.375.2001 EMAIL: Jennifer.Losurdo@genesiseng.net

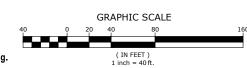
#### TRAFFIC ENGINEER

KIMLEY-HORN 817 W. PEACHTREE STREET, SUITE 601 ATLANTA, GA 30308 CONTACT: ANA EISENMAN, PE PHONE: 404.201.6155 EMAIL: ana.eisenman@kimley-horn.com

#### SITE LEGEND









1325 Satellite Blvd. NW Suite 202 Suwanee, Georgia 30024 470.375.2001 www.genesiseng.net

#3613 LLC 500 PROJECT:
1359 ELLSWORTH REDEVELOPMENT
1359 & 1355 ELLSWORTH INDUSTRIAL DRIVE
CITY OF ATLANTA, GEORGIA I REALTY ACQUISITIONS, T PEACHTREE STREET NW, SUITE GEORGIA 30329 CLIENT:
STREAM F
1180 WEST I
ATLANTA, GE REVISIONS

DRI

DRI

SITE PLAN

21-133

05/02/2022

PROJECT NUMBER:

DATE:

DRI-1

FORSYTH FABRICS/ BILLBOARD STUDIOS BUILDING A MAX. BUILDING HEIGHT 225' 12 STORY 230 UNITS DOG PARK DRÍVEWAY A/INTERSECTION 6
(EXISTING TO BE REGONSTRUCTED) BUILDING C MAX. BUILDING HEIGHT 225'
12 STORY
290 UNITS AMENITY COURTYARD BUILDING D BREWERY MAX. BUILDING HEIGHT 35' AMENITY COURTYARD BUILDING B PARKING DECK MAX. BUILDING HEIGHT 225' DRIVEWAY B/INTERSECTION 7 - STRUCTURE (4 LEVELS-840 SPACES) 145 UNITS WAEC-AM ATLANTA ELLSWORTH INDUSTRIAL DRIVE INTERSECTION CHELSEA WESTSIDE
N/E
BC ELLSWORTH-LLC
TAX PARCEL No.:17 0188000310

5.019 ACRES
218,621 SF
229,561 SF
I-2 (HEAVY INDUSTRIAL) BELTLINE OVERLAY
MRC-3 (MIXED RESIDENTIAL COMMERCIAL) BELTLINE OVERLAY
MULTI-FAMILY/COMMERCIAL
682,400 SF
13,000 SF
24,520 SF
12,260 SF
12,260 SF

SITE MULTI-FAMILY (BUILDINGS A, B, C)	DATA
MAXIMUM BUILDING AREA	644,880 GSF
MAXIMUM BUILDING HEIGHT	225'-0"
MAXIMUM RESIDENTIAL UNITS	665
RESIDENTIAL FLOOR AREA RATIO (FAR)	
MAX ALLOWED	699,587 SF (3.2 x NLA)
PROVIDED	644,880 SF (2.95 x NLA)
BUILDING COVERAGE	
MAX ALLOWED	85% NLA
NOT TO EXCEED	85% NLA
OPEN SPACE REQUIREMENTS	124,614 SF
RESIDENTIAL USES	0.57 x NLA (PER TABLE I)
OPEN SPACE PROVIDED	SHALL MEET REQUIREMENTS

MINIMUM PARKING REQUIRED PER CODE		443 SPACES
RESIDENTIAL USE (PER TABLE I)	366 SPACES	(0.55 x UNIT)
COMMERCIAL	22 SPACE	S (1/600 SF)
EATING/DRINKING >60% ALCOHOL SALES	55 SPACE	S (1/450 SF)
SHARED PARKING PROVIDED (4-LEVEL PARKING I	DECK)*	840 SPACES
LOADING SPACES REQUIRED		3 - 12'x35' SPACE
LOADING SPACES PROVIDED (INSIDE PARKING S	TRUCTURE)	3 SPACES
* NOTE: PROVIDED PARKING WILL NOT EXCEED PER BELTLINE OVERLAY REQUIREMENTS.	MAXIMUM ALLO	WABLE PARKING

CHELSEA WESTSIDE





## PROJECT CONTACTS

OWNER/CLIENT
STREAM REALTY ACQUISITIONS, LLC 1180 WEST PEACHTREE STREET NW, SUITE 500 ATLANTA, GA 30329 CONTACT: BEN HAUTT PHONE: 404.962.8601 EMAIL: bhautt@streamrealty.com

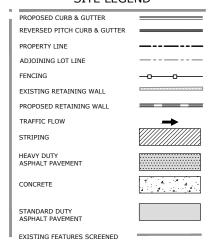
#### **CIVIL ENGINEER**

GENESIS ENGINEERING COLLABORATIVE 1325 SATELLITE BLVD NW, SUITE 202 SUWANEE, GA 30024 CONTACT: JENNIFER LOSURDO, PE PHONE: 470.375.2001 EMAIL: Jennifer.Losurdo@genesiseng.net

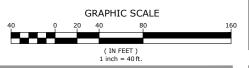
#### TRAFFIC ENGINEER

KIMLEY-HORN 817 W. PEACHTREE STREET, SUITE 601 ATLANTA, GA 30308 CONTACT: ANA EISENMAN, PE PHONE: 404.201.6155 EMAIL: ana.eisenman@kimley-horn.com

#### SITE LEGEND









1325 Satellite Blvd. NW Suite 202 Suwanee, Georgia 30024 470.375.2001 www.genesiseng.net

#3613 DRI PROJECT: 1359 ELLSWORTH REDEVELOPMENT 1359 & 1355 ELLSWORTH INDUSTRIAL DRIVE CITY OF ATLANTA, GEORGIA

LLC 500

I REALTY ACQUISITIONS, T PEACHTREE STREET NW, SUITE GEORGIA 30329

CLIENT:
STREAM F
1180 WEST I
ATLANTA, GI

REVISIONS
PROJECT NUMBER:
21-133
DATE:
05/02/2022

DRI-1

DRI

SITE PLAN

BUILDING A
MAX. BUILDING HEIGHT 225'
12 STORY
230 UNITS BUILDING C MAX. BUILDING HEIGHT 225'
12 STORY
290 UNITS BUILDING I 24,520 S MAX. BUILDING HEIGHT 35' AMENITY (4 LEVELS-840 SPACES ELLSWORTH INDUSTRIAL DRIVE PRIVATE ROAD

# **Trip Generation Analysis**

	Trip Generation Ar	10	d Edition Handbook Da 359 Ellsworth Atlanta. GA	ily IC & 3rd Edition AM/PN	IC)							
Land Use	Setting		aily Trips			AM Peak Hou		PM Peak Hour				
	<u> </u>		Density	Total	ln	Out	Total	In	Out	Total	ln	Out
Proposed Project Trips												
140 Manufacturing	General Urban/Suburban	12,500	Sq. Ft. GFA	60	30	30	9	7	2	9	3	6
221 Multifamily Housing (Mid-Rise)	General Urban/Suburban	145	dwelling units	646	323	323	52	12	40	57	35	22
222 Multifamily Housing (High-Rise)	General Urban/Suburban	520	dwelling units	2,332	1,166	1,166	133	45	88	158	88	70
932 High-Turnover (Sit-Down) Restaurant	General Urban/Suburban	13,000	Sq. Ft. GFA	1,394	697	697	124	68	56 1	118	72	46
971 Brewery Tap Room	General Urban/Suburban	12,500	Sq. Ft. GFA	772	386	386	9	8	1	123	73	50
Total Proposed Trips				5,204	2,602	2,602	327	140	187	465	271	194
Warehouse Trips (% of Total)				1%			3%	78%	22%	2%	33%	67%
Residential Trips (% of Total)				57%			57%	31%	69%		57%	43%
Restaurant Trips (% of Total)				42%			41%	57%	43%	52%	60%	40%
Existing Site Trips (To Be Removed)												
LUC Land Use	Setting	Density	Units	HIDE THIS ROW	Column1	Column2	Column3	Column4	Column5	Column6	Column7	Column8
150 Warehousing	General Urban/Suburban	116,206	Sq. Ft. GFA	222	111	111	38	29	9	40	11	29
Total Existing Site Trips (To Be Removed)	•	•		222	111	111	38	29	9	40	11	29
Total Proposed Project Trips Total Existing Site Trips (To Be Removed)				5,204 -222	2,602 -111	2,602 -111	-38	140 -29	-9	-40	-11	-29
Gross Project Trips				5,204	2,602	2,602	327	140	187	465	271	194
Brewery (Manufacturing) Trips				60	30	30	9	7	2	9	3	6
Car Trips (100% of Warehousing Trips)				60	30	30	9	7	2	9	3	6
Alternative Mode Reductions (10%)				-6	-3	-3		-1	0	-1	0	
Adjusted Car Trips				54	27	27	8	6	2	8	3	5
Residential Trips				2,978	1,489	1,489	185	57	128	215	123	92
Mixed-Use Reductions				-240	-120	-120		-2			-19	
Alternative Mode Reductions (20%)				-548	-274	-274	-33	-11	-22	-35	-21	-14
Adjusted Residential Trips				2,190	1,095	1,095	134	44	90	141	83	58
Restaurant/Brewery Trips				2,166	1,083	1,083	133	76	57	241	145	96
Mixed-Use Reductions				-240	-120	-120		-16			-20	
Alternative Mode Reductions (10%)				-192	-96	-96		-6	-6	-20	-13	
Pass By Reductions (Based on ITE Rates)				-746	-373	-373	0	0			-39	-39
Adjusted Restaurant Trips				988	494	494	103	54	49	104	73	30
Mixed-Use Reductions - TOTAL				-480	-240	-240	-36	-18	-18	-78	-39	-39
Alternative Mode Reductions - TOTAL				-746	-373	-373		-18			-34	-23
Pass-By Reductions - TOTAL				-746	-373	-373	0	0			-39	
Total Existing Site Trips (To Be Removed)				-222	-111	-111	-38	-29	-9	-40	-11	
New Trips				3,010	1,505	1,505		75				
Driveway Volumes				3,756	1,878	1,878	207	75	132	291	187	

# **Intersection Volume Worksheets**

INTERSECTION VOLUME DEVELOPMENT
INTERSECTION #1
Chattahoochee Ave NW/Chattahoochee Ave NW at Ellsworth Industrial Blvd NW

						AK HOUR										
		Ellsworth Industrial Blvd NW				Ellsworth Industrial Blvd NW				Chattahood	hee Ave NW		Chattahoochee Ave NW			
		Northbound				Southbound			Eastbound				Westbound			
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	52	11	283	0	11	3	6	0	19	604	58	0	217	413	79
Pedestrians			0				0				0				0	
Conflicting Pedestrians		0		0		0		0		0		0		0		0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				0				0		•		0				0
Heavy Vehicles	0	11	0	33	0	3	0	4	0	0	64	5	0	16	36	1
Heavy Vehicle %	2%	21%	2%	12%	2%	27%	2%	67%	2%	2%	11%	9%	2%	7%	9%	2%
Peak Hour Factor		0	.94	•		. 0	.94			0	.94				).94	•
Adjustment Factor	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83
Estimated 2022 Volumes	0	95	20	518	0	20	5	11	0	35	1,105	106	0	397	756	145
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	4	1	24	0	1	0	1	0	2	50	5	0	18	35	7
Approved Development Trips 1 - Project Revive DRI #3489											1				8	
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	1	0	0	0	8	0
2025 No-Build Traffic	0	99	21	542	0	21	5	12	0	37	1,156	111	0	415	799	152
			•	•	Proj	ect Trips	•								•	
Trip Distribution IN												10%		50%		
Trip Distribution OUT		(10%)		(50%)												
Warehouse Car Trips	0	0	0	1	0	0	0	0	0	0	0	1	0	3	0	0
Trip Distribution IN												15%		30%		
Trip Distribution OUT		(15%)		(30%)												
Residential Trips	0	14	0	27	0	0	0	0	0	0	0	7	0	13	0	0
Trip Distribution IN												10%		50%		
Trip Distribution OUT		(10%)		(50%)												
Restaurant Trips	0	5	0	25	0	0	0	0	0	0	0	5	0	27	0	0
Total Vehicular Project Trips	0	19	0	53	0	0	0	0	0	0	0	13	0	43	0	0
2025 Build Traffic	0	118	21	595	0	21	5	12	0	37	1,156	124	0	458	799	152

					DM DE	AK HOUR										
		llsworth Indu	strial Blvd NV	V			ustrial Blvd NV	/	l .	Chattahood	hee Ave NW		Г	Chattahood	hee Ave NW	
		Northbound				Southbound			Eastbound				Westbound			
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	128	32	322	0	76	35	45	0	13	453	109	0	319	695	45
Pedestrians			0				1				7				0	
Conflicting Pedestrians		7		0		0		7		1		0		0		1
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				0				0				0				0
Heavy Vehicles	0	3	2	5	0	0	4	0	0	0	6	3	0	13	20	0
Heavy Vehicle %	2%	2%	6%	2%	2%	2%	11%	2%	2%	2%	2%	3%	2%	4%	3%	2%
Peak Hour Factor		0	.93			0	.93			0	.93			0	.93	
Adjustment Factor	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03
Estimated 2022 Volumes	0	132	33	332	0	78	36	46	0	13	467	112	0	329	716	46
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	6	2	15	0	4	2	2	0	1	21	5	0	15	33	2
Approved Development Trips 1 - Project Revive DRI #3489											7				6	
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	7	0	0	0	6	0
2025 No-Build Traffic	0	138	35	347	0	82	38	48	0	14	495	117	0	344	755	48
					Proj	ject Trips										
Trip Distribution IN												10%		50%		
Trip Distribution OUT	_	(10%)		(50%)						_						
Warehouse Car Trips	0	1	0	3	0	0	0	0	0	0	0	0	0	2	0	0
Trip Distribution IN					г					1		15%		30%	1	
Trip Distribution NV		(15%)		(30%)					1			1570		30%		
Residential Trips	0	9	0	17	0	0	0	0	0	0	0	12	0	25	0	0
Residential Trips	U	9		1/				0				12		23	U	
Trip Distribution IN					Г				1			10%	1	50%	1	
Trip Distribution OUT		(10%)		(50%)	<b>—</b>							10/0		50/0		
Restaurant Trips	0	3	0	15	0	0	0	0	0	0	0	7	0	37	0	0
***************************************												· · · · · · · · · · · · · · · · · · ·			· · · · ·	
Total Vehicular Project Trips		13	0	35	0	0	0	0	0	0	0	19	0	64	0	0
•					•		•			•			•			
2025 Build Traffic	0	151	35	382	0	82	38	48	0	14	495	136	0	408	755	48

INTERSECTION VOLUME DEVELOPMENT
INTERSECTION #2
Elaine Ave NW at Ellsworth Industrial Blvd NW

				Lidilic / C	TO THE OF EAT		tilai biva ivv									
					AM PI	AK HOUR										
		Ellsworth Indu	ıstrial Blvd NV	V		Ellsworth Indu	ıstrial Blvd NV	V		Elaine .	Ave NW					
		Northbound				Southbound				Eastl	oound		Westbound			
	U-Turn	U-Turn Left Through Right			U-Turn	Left	Through	Right	U-Turn	U-Turn Left Through Right				Left	Through	Right
Observed 2022 Traffic Volumes	0	10	215	0	0	0	98	95	0	136	0	14	0	0	0	0
Pedestrians		•	0	•			0	•		•	2				1	
Conflicting Pedestrians		2		1		1		2		0		0		0		0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0
Conflicting Bicycles				0				0				0				0
Heavy Vehicles	0	2	20	0	0	0	9	14	0	27	0	0	0	0	0	0
Heavy Vehicle %	2%	20%	9%	2%	2%	2%	9%	15%	2%	20%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.88			0	.88			0	.88				0.88	
Adjustment Factor	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83
Estimated 2022 Volumes	0	18	393	0	0	0	179	174	0	249	0	26	0	0	0	0
			•	•			•		•	•	•	•	•		•	
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	1	18	0	0	0	8	8	0	11	0	1	0	0	0	0
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	19	411	0	0	0	187	182	0	260	0	27	0	0	0	0
			•	•	Pro	ect Trips	•		•	•	•		•		•	
Trip Distribution IN							60%					15%				
Trip Distribution OUT		(15%)	(60%)													
Warehouse Car Trips	0	0	1	0	0	0	4	0	0	0	0	1	0	0	0	0
	•	•	•	•	•	•	•	•	•	•	•	•	•		•	
Trip Distribution IN							45%					5%				
Trip Distribution OUT		(5%)	(45%)													
Residential Trips	0	5	41	0	0	0	20	0	0	0	0	2	0	0	0	0
	•												•			
Trip Distribution IN							60%					15%				
Trip Distribution OUT		(15%)	(60%)													
Restaurant Trips	0	7	29	0	0	0	32	0	0	0	0	8	0	0	0	0
	•	•			•	•	•	•	•	•		•	•	•	•	•
Total Vehicular Project Trips	0	12	71	0	0	0	56	0	0	0	0	11	0	0	0	0
• •															1	
2025 Build Traffic	0	31	482	0	T 0	0	243	182	0	260	0	38	T 0	0	0	0

						AK HOUR										
		Ellsworth Indu	strial Blvd NV	/		Ellsworth Indu	ıstrial Blvd NW	1		Elaine /	Ave NW					
	Northbound				Southbound				Easth	ound		Westbound				
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	31	206	0	0	0	216	221	0	178	0	19	0	0	0	0
Pedestrians		•	1				0			•	8				3	
Conflicting Pedestrians		8		3		3		8		0		1		1		0
Bicycles	0	2	0	0	0	0	1	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				0				1				0				0
Heavy Vehicles	0	0	6	0	0	0	7	10	0	7	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	3%	2%	2%	2%	3%	5%	2%	4%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.94			0	.94			0.	94			0	.94	
Adjustment Factor	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03
Estimated 2022 Volumes	0	32	212	0	0	0	222	228	0	183	0	20	0	0	0	0
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	1	10	0	0	0	10	10	0	8	0	1	0	0	0	0
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	33	222	0	0	0	232	238	0	191	0	21	0	0	0	0
					Proj	ect Trips										
Trip Distribution IN							60%					15%				
Trip Distribution OUT		(15%)	(60%)													
Warehouse Car Trips	0	1	3	0	0	0	2	0	0	0	0	0	0	0	0	0
Trip Distribution IN							45%					5%				-
Trip Distribution OUT		(5%)	(45%)													
Residential Trips	0	3	26	0	0	0	37	0	0	0	0	4	0	0	0	0
Trip Distribution IN	<u> </u>	(	(0000)				60%					15%				-
Trip Distribution OUT	<u> </u>	(15%)	(60%)					_		_						-
Restaurant Trips	0	5	18	0	0	0	44	0	0	0	0	11	0	0	0	0
Total Vehicular Project Trips		9	47	0	0	0	83	0	0	0	0	15	0	0	0	0
Total Veniculai Froject (Tips			-4/				. 03					13				
2025 Build Traffic	1 0	42	269	0	0	0	315	238	0	191	0	36	0	0	0	0

INTERSECTION VOLUME DEVELOPMENT
INTERSECTION #3
Driveway/Ellsworth Industrial Dr at Ellsworth Industrial Blvd NW

				.,,												
						AK HOUR										
		Ellsworth Indu	ıstrial Blvd NV	/		Ellsworth Indu	ıstrial Blvd NV	V		Driv	eway			Ellsworth	Industrial Dr	
		North	bound			South	bound			Eastl	oound		1	Wes	tbound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	10	237	4	0	3	87	22	0	5	3	7	0	1	0	6
Pedestrians			0				2				0				0	
Conflicting Pedestrians		0		0		0		0		2		0		0		2
Bicycles	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				1				0				0				0
Heavy Vehicles	0	0	22	0	0	0	6	1	0	1	1	4	0	0	0	2
Heavy Vehicle %	2%	2%	9%	2%	2%	2%	7%	5%	2%	20%	33%	57%	2%	2%	2%	33%
Peak Hour Factor			.82				.82				.82				).82	
Adjustment Factor	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83
Estimated 2022 Volumes	0	18	434	7	0	5	159	40	0	9	5	13	0	2	0	11
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	1	20	0	0	0	7	2	0	0	0	1	0	0	0	1
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	19	454	7	0	5	166	42	0	9	5	14	0	2	0	12
					Proj	ect Trips										
Trip Distribution IN			5%	20%		20%										
Trip Distribution OUT							(5%)							(20%)		(20%)
Warehouse Car Trips	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN			15%	35%		15%										
Trip Distribution OUT							(15%)							(35%)		(15%)
Residential Trips	0	0	7	15	0	7	14	0	0	0	0	0	0	32	0	14
Trip Distribution IN			5%	20%		20%										
Trip Distribution OUT							(5%)							(20%)		(20%)
Restaurant Trips	0	0	3	11	0	11	2	0	0	0	0	0	0	10	0	10
Pass-By Distribution IN			-40%	40%		15%	-15%						<u> </u>		1	
Pass-By Distribution OUT	-													(15%)	1	(40%)
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Vehicular Project Trips	0	0	10	27	0	19	16	0	0	0	0	0	0	42	0	24
2025 Build Traffic	I 0	19	464	34	I 0	24	182	42		9	5	14	I 0	44	0	36
2025 Build Traffic	0	19	464	34	0	24	182	42	0	9	5	14	1 0	44	0	36

						AK HOUR										
		Ellsworth Indi	ustrial Blvd NV	V		Ellsworth Indu	ustrial Blvd NV	V		Driv	eway			Ellsworth I	ndustrial Dr	
		North	bound			South	nbound			Eastl	oound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	4	203	3	0	3	219	3	0	34	0	16	0	0	1	1
Pedestrians			1				5				7				4	
Conflicting Pedestrians		7		4		4		7		5		1		1		5
Bicycles	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				1				0				0				0
Heavy Vehicles	0	0	4	0	0	0	6	0	0	1	0	0	0	0	1	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	3%	2%	2%	3%	2%	2%	2%	2%	100%	2%
Peak Hour Factor		0	.90			0	.90			0	.90			0	.90	
Adjustment Factor	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03
Estimated 2022 Volumes	0	4	209	3	0	3	226	3	0	35	0	16	0	0	1	1
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	10	0	0	0	10	0	0	2	0	1	0	0	0	0
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	4	219	3	0	3	236	3	0	37	0	17	0	0	1	1
					Proj	ect Trips										
Trip Distribution IN			5%	20%		20%										
Trip Distribution OUT							(5%)							(20%)		(20%)
Warehouse Car Trips	0	0	0	1	0	1	0	0	0	0	0	0	0	1	0	1
Trip Distribution IN			15%	35%		15%										
·			15%	35%		15%	(450()							(250/)		(450()
Trip Distribution OUT	0		12	29	_	12	(15%)		_		-	_	0	(35%)		(15%)
Residential Trips	0	0	12	29	0	12	9	0	0	0	0	0	U	20	0	9
Trip Distribution IN			5%	20%		20%		1	r		1	1			1	
Trip Distribution IN Trip Distribution OUT	-		5%	20%	<del>                                     </del>	20%	(5%)							(20%)		(20%)
Restaurant Trips	0	0	4	15	0	15	2	0	0	0	0	0	0	6	0	6
nestaurant mps			4	15		15			0	- 0				- 0		
Pass-By Distribution IN		l	-40%	40%	Г	15%	-15%	Π	1						T .	
Pass-By Distribution OUT			-7070	-0/0	<del>                                     </del>	13/0	1370							(15%)		(40%)
Pass-By Trips	0	0	-16	16	0	6	-6	0	0	0	0	0	0	6	0	16
/ E														-		
Total Vehicular Project Trips		0	0	61	0	34	5	0	0	0	0	0	0	33	0	32
2025 Build Traffic	0	4	219	64	0	37	241	3	0	37	0	17	0	33	1	33

INTERSECTION VOLUME DEVELOPMENT
INTERSECTION #4
Huff Rd NW at Ellsworth Industrial Blvd NW

					AM PE	AK HOUR										
					I	Ellsworth Indu	strial Blvd NV	V	I	Huff I	Rd NW			Huff	Rd NW	
		North	bound			South	bound			Fasti	ound		l	West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	0	0	0	0	64	0	41	0	118	279	0	0	0	145	0
Pedestrians		•	2				2	•		•	0				1	•
Conflicting Pedestrians		0		1		1		0		2		2		2		2
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Conflicting Bicycles				0				0				0				0
Heavy Vehicles	0	0	0	0	0	9	0	3	0	11	32	0	0	0	22	9
Heavy Vehicle %	2%	2%	2%	2%	2%	14%	2%	7%	2%	9%	11%	2%	2%	2%	15%	2%
Peak Hour Factor		0	.95			0.	.95			0	95			0	.95	
Adjustment Factor	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83
Estimated 2022 Volumes	0	0	0	0	0	117	0	75	0	216	511	0	0	0	265	0
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	0	0	0	5	0	3	0	10	23	0	0	0	12	0
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	0	0	0	0	122	0	78	0	226	534	0	0	0	277	0
					Proj	ect Trips										
Trip Distribution IN										17.5%						7.5%
Trip Distribution OUT						(7.5%)		(17.5%)								
Warehouse Car Trips	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
Trip Distribution IN										30%						20%
Trip Distribution OUT						(20%)		(30%)								
Residential Trips	0	0	0	0	0	18	0	27	0	13	0	0	0	0	0	9
Trip Distribution IN										17.5%						7.5%
Trip Distribution OUT						(7.5%)		(17.5%)	l							
Restaurant Trips	0	0	0	0	0	4	0	9	0	9	0	0	0	0	0	4
Total Vehicular Project Trips	0	0	0	0	0	22	0	36	0	23	0	0	0	0	0	13
2025 Build Traffic	1 0	0	0	0	I 0		0		0	249	534	0	I 0	0	277	13
2025 Build Traffic	0	0	0	0	0	144	0	114	0	249	534	0	0	0	277	13

						AK HOUR										
	l						strial Blvd NV	,			Rd NW				Rd NW	
	l		bound				bound				oound				bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	0	0	0	0	95	0	142	0	93	194	0	0	0	250	105
Pedestrians			3				1				0				8	
Conflicting Pedestrians		0		8		8		0		11		3		3		11
Bicycles	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				0				0				0				0
Heavy Vehicles	0	0	0	0	0	4	0	3	0	1	12	0	0	0	10	4
Heavy Vehicle %	2%	2%	2%	2%	2%	4%	2%	2%	2%	2%	6%	2%	2%	2%	4%	4%
Peak Hour Factor		0.	89			0.	89			0.	89				.89	
Adjustment Factor	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03
Estimated 2022 Volumes	0	0	0	0	0	98	0	146	0	96	200	0	0	0	258	108
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	0	0	0	4	0	7	0	4	9	0	0	0	12	5
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	0	0	0	0	102	0	153	0	100	209	0	0	0	270	113
					Proj	ect Trips										
Trip Distribution IN										17.5%						7.5%
Trip Distribution OUT						(7.5%)		(17.5%)								
Warehouse Car Trips	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0
Trip Distribution IN										30%						20%
Trip Distribution OUT						(20%)		(30%)								
Residential Trips	0	0	0	0	0	12	0	17	0	25	0	0	0	0	0	17
Trip Distribution IN										17.5%						7.5%
Trip Distribution OUT						(7.5%)		(17.5%)								
Restaurant Trips	0	0	0	0	0	2	0	5	0	13	0	0	0	0	0	5
Total Vehicular Project Trips		0	0	0	I 0	14	0	23	0	39	0	0	0	0	0	22
Total venicular Project Trips		0		0		14		23		39				0		
2025 Build Traffic	0	0	0	0	0	116	0	176	0	139	209	0	0	0	270	135

INTERSECTION VOLUME DEVELOPMENT
INTERSECTION #5
Kennesaw Dr/Huff Rd NW at Marietta Blvd NW

					AM PE	AK HOUR										
		Marietta	Blvd NW			Marietta	Blvd NW			Kenne	saw Dr		l	Huff	Rd NW	
		North	bound			South	nbound			Eastl	ound		l	West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	5	413	235	0	129	482	6	0	6	8	0	0	101	2	81
Pedestrians			0				0				0				1	
Conflicting Pedestrians		0		1		1		0		0		0		0		0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				0				0				0				0
Heavy Vehicles	0	0	51	13	0	17	57	2	0	1	5	0	0	7	0	17
Heavy Vehicle %	2%	2%	12%	6%	2%	13%	12%	33%	2%	17%	63%	2%	2%	7%	2%	21%
Peak Hour Factor		0	.95			0	.95			0	.95	•		0	.95	
Adjustment Factor	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83
Estimated 2022 Volumes	0	9	756	430	0	236	882	11	0	11	15	0	0	185	4	148
	•		•				•		•	•		•			•	
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	35	20	0	11	40	1	0	1	1	0	0	8	0	7
Approved Development Trips 1 - Project Revive DRI #3489			41				10									
Approved Development Trips 2 - Project Granite DRI #3298			32				85									
Total Approved Development Trips	0	0	73	0	0	0	95	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	9	864	450	0	247	1,017	12	0	12	16	0	0	193	4	155
	•	•	•		Proj	ect Trips				•		•	•			
Trip Distribution IN			7.5%	17.5%												
Trip Distribution OUT							(7.5%)							(17.5%)		
Warehouse Car Trips	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
					•		•									
Trip Distribution IN				30%												
Trip Distribution OUT														(30%)		
Residential Trips	0	0	0	13	0	0	0	0	0	0	0	0	0	27	0	0
	•	•	•		•				•	•		•	•			
Trip Distribution IN			7.5%	17.5%												
Trip Distribution OUT							(7.5%)							(17.5%)		
Restaurant Trips	0	0	4	9	0	0	4	0	0	0	0	0	0	9	0	0
Total Vehicular Project Trips	0	0	4	23	0	0	4	0	0	0	0	0	0	36	0	0
2025 Build Traffic	0	9	868	473	0	247	1,021	12	0	12	16	0	0	229	4	155

					PM PE	AK HOUR										
		Marietta	Blvd NW			Marietta	a Blvd NW			Kenne	saw Dr			Huff I	Rd NW	
		North	bound			Sout	nbound			Eastl	oound		l	West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	1	694	180	0	85	628	0	0	1	1	5	0	295	1	103
Pedestrians			0			•	0	•		•	0	•			1	
Conflicting Pedestrians		0		1		1		0		0		0		0		0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				0				0				0				0
Heavy Vehicles	0	1	38	3	0	6	50	0	0	0	0	0	0	7	0	9
Heavy Vehicle %	2%	100%	5%	2%	2%	7%	8%	2%	2%	2%	2%	2%	2%	2%	2%	9%
Peak Hour Factor		0	.94			C	.94			0	.94			0	.94	
Adjustment Factor	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03
Estimated 2022 Volumes	0	1	715	185	0	88	647	0	0	1	1	5	0	304	1	106
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	33	8	0	4	30	0	0	0	0	0	0	14	0	5
Approved Development Trips 1 - Project Revive DRI #3489			29				43									
Approved Development Trips 2 - Project Granite DRI #3298			85				38									
Total Approved Development Trips	0	0	114	0	0	0	81	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	1	862	193	0	92	758	0	0	1	1	5	0	318	1	111
					Proj	ject Trips										
Trip Distribution IN			7.5%	17.5%												
Trip Distribution OUT							(7.5%)							(17.5%)		
Warehouse Car Trips	0	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0
Trip Distribution IN				30%	г		1					1				
Trip Distribution IN Trip Distribution OUT				30%										(30%)		
Residential Trips	0	0	0	25	0	0	0	0	0	0	0	0	0	17	0	0
Residential Irips	0	- 0		25					U	0				1/		
Trip Distribution IN			7.5%	17.5%					1	1		1				
Trip Distribution IN Trip Distribution OUT			7.5%	17.5%			(7.5%)					_		(17.5%)		
Restaurant Trips	0	0	5	13	0	0	2	0	0	0	0	0	0	5	0	0
- Commence and Company																
Total Vehicular Project Trips		0	5	39	0	0	2	0	0	0	0	0	0	23	0	0
	•															
2025 Build Traffic	0	1	867	232	0	92	760	0	0	1	1	5	0	341	1	111

INTERSECTION VOLUME DEVELOPMENT
INTERSECTION #6
Driveway A at Ellsworth Industrial Parkway

					AM PE	AK HOUR										
		Ellsworth Indu	ustrial Parkway	,		Ellsworth Indu	strial Parkwa	v					I	Drive	way A	
			bound			South	bound			Easth	ound		l	West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	0	248	0	0	0	112	0	0	0	0	0	0	0	0	0
Pedestrians			0				2				0				0	
Conflicting Pedestrians		0		0		0		0		2		0		0		2
Bicycles	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				1				0				0				0
Heavy Vehicles	0	0	25	0	0	0	7	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	10%	2%	2%	2%	6%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.82			0.	.82			0.	82			0	.82	
Adjustment Factor	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83
Estimated 2022 Volumes	0	0	454	0	0	0	205	0	0	0	0	0	0	0	0	0
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	21	0	0	0	9	0	0	0	0	0	0	0	0	0
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	0	475	0	0	0	214	0	0	0	0	0	0	0	0	0
					Proj	ect Trips										
Trip Distribution IN				5%		55%	20%									
Trip Distribution OUT			(20%)											(5%)		(55%)
Warehouse Car Trips	0	0	0	0	0	3	1	0	0	0	0	0	0	0	0	1
				15%											1	
Trip Distribution IN	-		70	15%		35%	15%							(*****)		(2220)
Trip Distribution OUT			(15%)				_							(15%)	_	(35%)
Residential Trips	0	0	14	7	0	15	7	0	0	0	0	0	0	14	0	32
Trip Distribution IN				5%		55%	20%									
Trip Distribution OUT			(20%)	370		3370	20/0							(5%)		(55%)
Balancing Adjustment			(20/0)											(370)		(3370)
Restaurant Trips	0	0	10	3	0	30	11	0	0	0	0	0	0	2	0	27
Nestaurant Trips			10	,		30	- 11									
Pass-By Distribution IN			-20%	20%		25%	-25%									
Pass-By Distribution OUT						,,,	9,1							(25%)		(20%)
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
					•			•		•	•		•			
Total Vehicular Project Trips	0	0	24	10	0	48	19	0	0	0	0	0	0	16	0	60
2025 Build Traffic	0	0	499	10	0	48	233	0	0	0	0	0	0	16	0	60

					PM PE	AK HOUR										
		llsworth Indu	ustrial Parkwa	у		Ellsworth Indu	ıstrial Parkwa	У					I	Drive	eway A	
		North	bound			South	bound			Eastl	oound			West	tbound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	0	238	0	0	0	225	0	0	0	0	0	0	0	0	0
Pedestrians			1				5				0				0	
Conflicting Pedestrians		0		0		0		0		5		1		1		5
Bicycles	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				1				0				0				0
Heavy Vehicles	0	0	5	0	0	0	6	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.90			0.	.90	•		0	.90	•		0	.90	
Adjustment Factor	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03
Estimated 2022 Volumes	0	0	245	0	0	0	232	0	0	0	0	0	0	0	0	0
			•		•		•								•	
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	11	0	0	0	11	0	0	0	0	0	0	0	0	0
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	0	256	0	0	0	243	0	0	0	0	0	0	0	0	0
					Proj	ect Trips	•								•	
Trip Distribution IN				5%		55%	20%									
Trip Distribution OUT			(20%)											(5%)		(55%)
Warehouse Car Trips	0	0	1	0	0	2	1	0	0	0	0	0	0	0	0	3
Trip Distribution IN				15%		35%	15%									
Trip Distribution OUT			(15%)											(15%)		(35%)
Residential Trips	0	0	9	12	0	29	12	0	0	0	0	0	0	9	0	20
Trip Distribution IN				5%		55%	20%									
Trip Distribution OUT			(20%)											(5%)		(55%)
Restaurant Trips	0	0	6	4	0	40	15	0	0	0	0	0	0	2	0	17
Pass-By Distribution IN			-20%	20%		25%	-25%							(01)		(222)
Pass-By Distribution OUT	<u> </u>				<u> </u>									(25%)		(20%)
Pass-By Trips	0	0	-8	8	0	10	-10	0	0	0	0	0	0	10	0	8
Total Vehicular Project Trips		0		24	I 0	81	18	0	0	0			0	21	0	48
Total venicular Project Trips		0	8			81	18			0	0	0		21		48
2025 Build Traffic	0	0	264	24	1 0	81	261	0	0	0	0	0	0	21	0	48
2023 Dulla Hallic	U	J	204	24	U U	91	201	U	U	U	U	U		21	U	40

# INTERSECTION VOLUME DEVELOPMENT INTERSECTION #7 Ellsworth Industrial Drive at Driveway B

					AM PE	AK HOUR										
					Ι	Drive	way B		l e	Ellsworth In	dustrial Drive		l e	Ellsworth Ir	dustrial Drive	
		North	bound				bound				oound		l		tbound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians			0				0				0				0	
Conflicting Pedestrians		0		0		0		0		0		0		0		0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				0				0				0				0
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.82			0	.82			0	.82			(	1.82	
Adjustment Factor	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83	1.83
Estimated 2022 Volumes	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
					Proj	ect Trips										
Trip Distribution IN										40%						
Trip Distribution OUT								(40%)								
Warehouse Car Trips	0	0	0	0	0	0	0	1	0	2	0	0	0	0	0	0
									_							
Trip Distribution IN										50%						
Trip Distribution OUT								(50%)								
Residential Trips	0	0	0	0	0	0	0	45	0	22	0	0	0	0	0	0
Trip Distribution IN										40%						
Trip Distribution OUT								(40%)								1
Restaurant Trips	0	0	0	0	0	0	0	20	0	22	0	0	0	0	0	0
									_							
Pass-By Distribution IN										55%						
Pass-By Distribution OUT								(55%)								
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Vehicular Project Trips	0	0	0	0	0	0	0	66	0	46	0	0	0	0	0	0
2025 Build Traffic	0	0	0	0	0	0	0	66	0	46	0	0	0	0	0	0

					PM PE	AK HOUR										
						Drive	way B			Ellsworth In	dustrial Drive			Ellsworth In	dustrial Drive	
		North	bound			South	bound			Eastl	ound			West	tbound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2022 Traffic Volumes	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians			0				0				7				4	
Conflicting Pedestrians		7		4		4		7		0		0		0		0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Conflicting Bicycles				0				0				0				0
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.90			0	.90			0	90			0.	.90	
Adjustment Factor	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03	1.03
Estimated 2022 Volumes	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Background Growth Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Approved Development Trips 1 - Project Revive DRI #3489																
Approved Development Trips 2 - Project Granite DRI #3298																
Total Approved Development Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 No-Build Traffic	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
					Proj	ect Trips										
Trip Distribution IN										40%						
Trip Distribution OUT								(40%)								
Warehouse Car Trips	0	0	0	0	0	0	0	2	0	1	0	0	0	0	0	0
Trip Distribution IN										50%						
Trip Distribution OUT								(50%)								
Residential Trips	0	0	0	0	0	0	0	29	0	42	0	0	0	0	0	0
Trip Distribution IN	-		-		-			(40%)	<del></del>	40%					<del>                                     </del>	<b>—</b>
Trip Distribution OUT							_					_				
Restaurant Trips	0	0	0	0	0	0	0	12	0	29	0	0	0	0	0	0
Pass-By Distribution IN			1						г —	55%						
Pass-By Distribution OUT	-							(55%)	-	3370						
Pass-By Trips	0	0	0	0	0	0	0	21	0	21	0	0	0	0	0	0
1000 04 11100								- 21						- 0		
Total Vehicular Project Trips		0	0	0	0	0	0	64	0	93	0	0	0	0	0	0
			<u> </u>		L -					- 55						
2025 Build Traffic	0	0	0	0	0	0	0	64	0	93	0	0	0	0	0	0

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# Resurfacing - GDOT Group A (FC-9403)

TYPE

PROJECT NUMBER

COUNCIL DISTRICTS

RESURFACING

1031

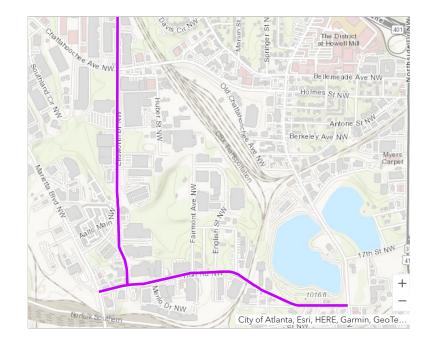
03, 04, 08, 09, 10, 11, 12

## Scope

Includes the resurfacing of 8th St, Beecher St, Bolton Rd, Campbellton Rd, Collier Dr, Collier Rd, County Line Rd, Defoors Ferry Rd, Ellsworth Industrial Blvd, Fair Dr, Fairburn Rd, Hills Ave, Howell Mill, Huff Rd, Margaret Mitchell Dr, Metropolitan Pkwy, Northside Pkwy, Northwest Dr, Peachtree Battle Ave, Stanton Rd, and Stone Hogan Connector. Bicycle lanes included on Fair Dr from Metropolitan Pkwy to Pryor Rd.

PAID	\$13,857,434
PROJECT START	Apr 2016
DESIGN FINISH	Oct 2018
CONSTRUCTION START	Oct 2017
CONSTRUCTION FINISH	Apr 2021

Disclaimer: Project schedules and scopes are subject to change.



### PHASE

Not Started Planning & Development Design Construction Complete

## **Additional Project Information**

## ATLANTA DEPARTMENT OF TRANSPORTATION (ATLDOT)

Atlanta Department of Transportation (ATLDOT)
Atlanta City Hall
55 Trinity Avenue SW, Suite 4350
Atlanta GA 30303

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## AT-277A

## Atlanta Region's Plan RTP (2020) PROJECT FACT SHEET

Short Title	CYCLE ATLANTA PHASE 1.0 - IMPLEMENTATION AT VARIOUS LOCATIONS	Coller Rd NW  Book and A
GDOT Project No.	0014993	W 278 Pence de Leon Ave
Federal ID No.	N/A	Voseph
Status	Programmed	Atienta Ho
Service Type	Last Mile Connectivity / Pedestrian Facility	402
Sponsor	City of Atlanta	78 a 38 p 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9
Jurisdiction	City of Atlanta	Signature of the state of the s
Analysis Level	In the Region's Air Quality Conformity Analysis	Avon Ave SW
Existing Thru Lane	5/4/3 <b>LCI</b>	Network Year 2030
Planned Thru Lane	4/3/2 Flex	Corridor Length 4.6 miles
Detailed Description a	and Justification	
This project will install the b	oicycle facilities identified in the ARC funded Cycle Atlar	nta: Phase 1.0 study. These facilities will support the existing and

This project will install the bicycle facilities identified in the ARC funded Cycle Atlanta: Phase 1.0 study. These facilities will support the existing and planned compact development in the central core of the city, as well as within the Atlanta BeltLine Planning Area by supporting cycling as a mode of transportation between varied land uses. Projects include (1) protected bike lanes on Mangum/Walker/Peters/Lee - part of Corridor A, (2) bike lanes and buffered bike lanes on R. McGill Blvd - part of Corridor C, and (3) the Bicycle Boulevard/Neighborway along Woodward Avenue - part of Corridor D. The projects add 4.6 miles of high quality bicycle facilities to Atlanta's network and make key connections within the 31-mile Phase 1.0 network. Portions of this project are located in Equitable Target Areas.

Phas	Phase Status & Funding Status			TOTAL PHASE	BREAKDOWN	REAKDOWN OF TOTAL PHASE COST BY FUNDING SOUR						
Info	rmation		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE				
PE	TAP - Urban (>200K) (ARC)	AUTH	2017	\$237,500	<del>\$190,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$47,500</del>				
CST	Local Jurisdiction/Municipality Funds		2022	\$2,950,000	\$0,000	\$0,000	\$0,000	\$2,950,000				
				\$3,187,500	\$190,000	\$0,000	\$0,000	\$2,997,500				

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases

7

A:C

## AT-287

## Atlanta Region's Plan RTP (2020) PROJECT FACT SHEET

Short Title	US 19/41/SR 3 (NORTHSIDE DRIVE) SIG AT 13 LOCATIONS	GNAL UPGRADES	hee Avenum Tourism Rd. W. Mariette	13 403
GDOT Project No.	0012823		Rd	75
Federal ID No.	N/A		W Manotta Sing	14th St NW
Status	Programmed		1	Georgia Institute of Technology
Service Type	Roadway / Operations & Safety		A T	Georgia S
Sponsor	GDOT		1	Technology 3
Jurisdiction	City of Atlanta		0 0.25 0.5 Miles	North Ave NW 00
Analysis Level	Exempt from Air Quality Analysis (40 CF	R 93)		Nor Z
Existing Thru Lane		LCI	Network Year	TBD
Planned Thru Lane	6	Flex	Corridor Length	N/A miles
Detailed Description a	nd Justification			
approximately 2.5 miles, wit	orthside Drive) and Hemphill Avenue at SF h 11 signal upgrades: North Avenue, Don Bellemeade Avenue, I-75 SB, I-75 NB, and	ald Lee Hollowell Pa	arkway NW, Marietta Street, 1	

Pha	se Status & Funding	Status	FISCAL	TOTAL PHASE	BREAKDOWN	OF TOTAL PHAS	E COST BY FUN	COST BY FUNDING SOURCE				
Info	rmation		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE				
PE	STP - Urban (>200K) (ARC)	AUTH	2014	\$325,000	<del>\$325,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>				
PE	Surface Transportation Block Grant (STBG) Program Flex (GDOT)	AUTH	2018	\$106,000	<del>\$106,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	\$0,000				
ROW	Surface Transportation Block Grant (STBG) Program - Urban (>200K) (ARC)	AUTH	2020	\$466,140	<del>\$466,140</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>				
UTL	Congestion Mitigation & Air Quality Improvement (CMAQ)		2022	\$497,831	\$497,831	\$0,000	\$0,000	\$0,000				
CST	CST Congestion Mitigation & Air Quality Improvement (CMAQ)		2022	\$2,420,906	\$2,420,906	\$0,000	\$0,000	\$0,000				
				\$3,815,877	\$3,815,877	\$0,000	\$0,000	\$0,000				

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion

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## Marietta TCC Combo

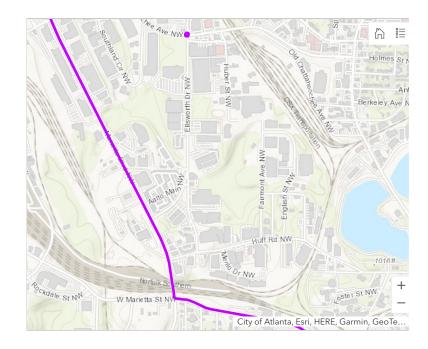
PROJECT NUMBER COUNCIL DISTRICTS 03, 04, 09 TRAFFIC SIGNALS 1053

## Scope

Includes the replacement of traffic signal LEDs, cabinets, controller monitors, signal wiring, communications, ADA ramps and timing at multiple intersections on three (3) corridors: Marietta Blvd, Chattahoochee Ave, and West Marietta St.

PAID	\$743,381
PROJECT START	Sep 2015
DESIGN FINISH	Oct 2020
CONSTRUCTION START	Oct 2015
CONSTRUCTION FINISH	Nov 2023

Disclaimer: Project schedules and scopes are subject to change.



**PHASE** 

Not Started

Planning & Development Design Construction Complete

## **Additional Project Information**

#### ATLANTA DEPARTMENT OF TRANSPORTATION (ATLDOT)

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## AT-320

# Atlanta Region's Plan RTP (2020) PROJECT FACT SHEET

Short Title	ATLANTA TRAFFIC SIGNAL ENHANCEMENT PROGRAM - PHASE 1 AT VARIOUS INTERSECTIONS ON GREENBRIAR PARKWAY, SYLVAN ROAD, 10TH STREET, STATE STREET AND NORTH AVENUE	280 On the Park And P
GDOT Project No.	0017802	Allanda Allanda S
Federal ID No.	N/A	World E days Dr. 6. 115 15 15 15 15 15 15 15 15 15 15 15 15
Status	Programmed	Corcade Rd SW
Service Type	Roadway / Operations & Safety	The state of the companion of the state of t
Sponsor	City of Atlanta	7) ISJ Manual Too (160 )
Jurisdiction	City of Atlanta	0 1 % Whites Constant A SW 4 54
Analysis Level	Exempt from Air Quality Analysis (40 CFR 93)	20 5 5
Existing Thru Lane	N/A LCI	Network Year TBD
Planned Thru Lane	N/A Flex	Corridor Length N/A miles
Detailed Description a	nd Justification	
enhancements include but n	enhancements at intersections on Greenbriar Pkwy, Sylvan F ot limited to signal equipment upgrades, detection upgrades installation and signal timing optimization to reduce over al	s, pavement marking improvements, ADA ramps, 4G or

Phas	se Status & Funding	Status	FISCAL	TOTAL PHASE	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE							
Info	rmation		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE				
PE	Surface Transportation Block Grant (STBG) Program - Urban (>200K) (ARC)	AUTH	2021	\$400,000	<del>\$320,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$80,000</del>				
ROW Local Jurisdiction/Municipality Funds			2022	\$182,614	\$0,000	\$0,000	\$0,000	\$182,614				
UTL	Local Jurisdiction/Municipality Funds		2024	\$187,000	\$0,000	\$0,000	\$0,000	\$187,000				
CST Surface Transportation Block Grant (STBG) Program - Urban (>200K) (ARC)			2024	\$2,171,656	\$1,737,325	\$0,000	\$0,000	\$434,331				
				\$2,941,270	\$2,057,325	\$0,000	\$0,000	\$883,945				

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases







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## **Howell Mill Complete Street**

TYPE PROJECT NUMBER COUNCIL DISTRICTS

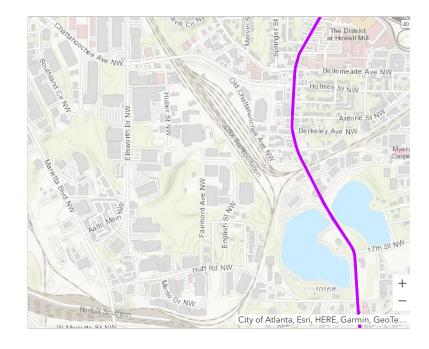
COMPLETE STREET 1007 03, 08, 09

## Scope

Includes resurfacing, restriping, new fiber communication between intersections, signals upgrades, partial corridor raised bicycle lanes, sidewalk repairs, additional mid-block crossings, 3 new signalized intersections, partial road diet, and ADA upgrades from Marietta St to Collier Rd.

PAID	\$1,352,716
PROJECT START	Jan 2016
DESIGN FINISH	Oct 2021
CONSTRUCTION START	Oct 2022
CONSTRUCTION FINISH	Sep 2024

Disclaimer: Project schedules and scopes are subject to change.



#### **PHASE**

Not Started Planning & Development Design Construction Complete

## **Additional Project Information**

#### ATLANTA DEPARTMENT OF TRANSPORTATION (ATLDOT)

Atlanta Department of Transportation (ATLDOT)
Atlanta City Hall
55 Trinity Avenue SW, Suite 4350
Atlanta GA 30303

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# Ellsworth Industrial Boulevard Speed Data

#### Bi-Directional Speed & Class Count | | NB EB Speed 15min



Site 1

Ellsworth Industrial Blvd NW,
south of Old Chattahoochee Ave NW

Date
Tuesday, March 29, 2022

Lat/Long 33.797841°, -84.427033°

Q Click here for Map

0000 - 2400 (Weekday 24h Session) NB EB Speed 15min

								ound, (Moven								15min
Time 0000 - 0015	5-14 mph 0	15-19 mph 0	20-24 mph 0	25-29 mph 0	30-34 mph	35-39 mph 2	40-44 mph	45-49 mph	50-54 mph 0	55-59 mph 0	60-64 mph 0	65-69 mph 0	70-74 mph 0	75-79 mph 0	80-99 mph 0	Total 11
0005 - 0015	0	0	1	0	3	2	1	1	0	1	0	0	0	0	0	9
0030 - 0045	0	0	0	1	1	0	5	5	0	0	0	0	0	0	0	12
0045 - 0100	0	0	1	0	2	5	3	2	0	0	0	0	0	0	0	13
0100 - 0115	0	0	0	0	0	0	2	1	0	1	0	0	0	0	0	4
0115 - 0130 0130 - 0145	0	0	0	0	0	3 2	7 6	2	0	0	0	0	0	0	0	16 9
0145 - 0200	0	0	0	3	0	5	3	0	0	0	0	0	0	0	0	11
0200 - 0215	0	0	0	0	0	3	1	1	0	0	0	0	0	0	0	5
0215 - 0230	0	0	0	1	1	2	1	1	1	0	0	0	0	0	0	7
0230 - 0245	0	0	0	2	1	2	4	1	0	0	1	0	0	0	0	11
0245 - 0300	0	0	0	0	0	2	1	3	1	0	0	0	0	0	0	7
0300 - 0315 0315 - 0330	0	0	0	0	0	3 0	1 2	1 2	1 3	1 0	0	0	0	0	0	7
0315 - 0330	0	0	0	0	0	3	4	2	1	0	0	0	0	0	0	10
0345 - 0400	0	0	0	0	0	1	0	3	1	0	0	0	0	0	0	5
0400 - 0415	1	0	0	0	2	1	3	0	0	0	1	0	0	0	0	8
0415 - 0430	0	0	0	0	0	1	4	1	0	0	0	0	0	0	0	6
0430 - 0445 0445 - 0500	0	0	0	0	0	2 5	5	0	1 0	0	0	0	0	0	0	8 15
0500 - 0515	0	0	0	0	3	3	2	4	0	0	0	0	0	0	0	12
0515 - 0530	0	0	0	0	1	1	4	2	0	0	0	0	0	0	0	8
0530 - 0545	0	0	0	0	2	5	3	2	1	0	0	0	0	0	0	13
0545 - 0600	0	0	0	0	1	7	4	3	0	0	0	0	0	0	0	15
0600 - 0615	0	0	0	0	1	8	6	5	1	1	1	0	0	0	0	23
0615 - 0630 0630 - 0645	0	1 0	0	1 0	5	6 14	10 17	3	1	0	0	0	0	0	0	27 36
0645 - 0700	0	0	0	2	8	14 15	9	5	0	0	0	0	0	0	1	40
0700 - 0715	0	0	0	3	5	33	19	10	1	0	0	0	0	0	0	71
0715 - 0730	0	0	0	0	12	28	24	10	0	0	0	0	0	0	0	74
0730 - 0745	0	0	0	0	7	42	18	11	2	0	0	0	0	0	0	80
0745 - 0800	0	0	0	1	12	21	26	10	1	0	0	0	0	0	0	71
0800 - 0815	0	0	0	3	6	39	18	7	4	0	1	0	0	0	1	79
0815 - 0830 0830 - 0845	0	0	1 0	0	18 14	38 34	42 28	3 8	1 4	0	0	0	0	0	0	103 89
0845 - 0900	0	0	0	0	10	29	26	6	1	0	0	0	0	0	0	72
0900 - 0915	0	0	0	1	5	27	28	6	1	0	0	0	0	0	0	68
0915 - 0930	0	0	0	2	10	25	29	7	3	0	1	0	0	0	0	77
0930 - 0945	0	0	0	1	11	21	22	6	2	1	0	0	0	0	0	64
0945 - 1000 1000 - 1015	0	0	0	2	9	18	24	5	2	0	0	0	0	0	0	60
1015 - 1030	0	0	0 2	5 2	13 11	21 26	11 29	10 6	2 1	1 0	0	0	0	0	1 0	64 78
1030 - 1045	0	ő	1	12	16	26	13	6	1	0	0	0	0	0	0	75
1045 - 1100	0	0	0	2	14	25	23	3	1	1	0	0	0	0	0	69
1100 - 1115	0	0	2	2	19	39	11	8	1	1	0	1	0	0	0	84
1115 - 1130	0	0	1	9	20	29	16	8	0	0	0	0	0	0	0	83
1130 - 1145 1145 - 1200	0	0	0 2	3 2	18 17	33 36	22 20	6	2 2	0	0	0	0	0	0	84 83
1200 - 1215	0	0	3	9	18	37	20	6	4	1	1	0	0	0	0	101
1215 - 1230	1	1	1	10	27	41	19	5	1	1	0	0	0	0	1	108
1230 - 1245	1	2	4	4	11	31	14	7	1	0	0	0	0	0	0	75
1245 - 1300	0	0	0	9	19	41	12	6	1	0	0	0	0	0	0	88
1300 - 1315	0	0	1	8	15	30	22	8	0	0	0	1	0	0	0	85
1315 - 1330	0	0	0	9	22	39	17	2	1	0	0	0	0	0	0	90
1330 - 1345 1345 - 1400	0	0	0	4	29 6	29 23	22 17	4 7	0	0	0	0	0	0	0	88 59
1400 - 1415	0	0	4	5	18	40	10	10	1	2	0	0	0	0	0	90
1415 - 1430	0	0	1	1	10	21	15	6	4	0	0	0	0	0	0	58
1430 - 1445	1	2	1	2	17	19	19	1	3	0	1	0	0	0	0	66
1445 - 1500	0	0	0	8	7	28	18	8	3	0	0	0	0	0	1	73
1500 - 1515 1515 - 1530	0	0	0	7 1	27 16	48 47	21 26	4 15	0	0	0	0	0	0	0	111 105
1530 - 1545	0	0	4	12	21	32	16	3	1	1	1	0	0	0	1	92
1545 - 1600	0	0	2	4	13	23	30	8	0	0	0	0	0	0	1	81
1600 - 1615	1	1	5	5	20	36	24	4	1	0	0	0	0	1	0	98
1615 - 1630	0	0	2	6	16	38	26	10	1	0	0	0	0	0	0	99
1630 - 1645 1645 - 1700	0	0	3	3 7	18 15	36 31	35 23	11 1	4 3	0	0	0	0	0	0	110 81
1645 - 1700 1700 - 1715	0	0	1	5	27	31	23	6	1	1	0	0	0	0	2	81 97
1715 - 1730	0	0	3	9	20	46	27	5	2	0	0	0	0	0	0	112
1730 - 1745	0	0	0	2	17	40	31	5	2	0	0	0	0	0	1	98
1745 - 1800	0	1	2	9	24	34	24	9	0	0	0	0	0	0	0	103
1800 - 1815	5	0	3	7	21	34	31	8	2	0	0	0	0	0	0	111
1815 - 1830 1830 - 1845	0	1 0	5 2	6 5	28 7	32 22	22 16	6 5	0	0	0	0	0	0	0	102 57
1830 - 1845 1845 - 1900	1	0	4	1	13	25	22	5	0	0	0	0	0	0	0	71
1900 - 1915	0	0	0	7	17	25	21	6	2	0	0	0	0	0	0	78
1915 - 1930	0	0	1	7	10	22	18	9	3	1	0	0	0	0	0	71
1930 - 1945	0	0	0	1	9	20	19	3	2	0	1	0	0	0	0	55
1945 - 2000	0	0	2	4	7	34	13	4	0	1	0	0	0	0	0	65
2000 - 2015 2015 - 2030	0	0 2	3 2	3	14 10	28 12	8 12	4 3	0	0	0	0	0	0	0	60 44
2015 - 2030	0	0	0	7	10	12	9	5	0	0	0	0	0	0	0	44
2045 - 2100	0	0	0	7	21	17	3	2	0	0	0	0	0	0	0	50
2100 - 2115	0	0	1	7	10	15	9	3	0	0	0	0	0	0	0	45
2115 - 2130	0	0	2	6	14	17	9	3	0	0	0	0	0	0	0	51
2130 - 2145	0	0	4	3	13	8	9	4	1	0	0	0	0	0	0	42
2145 - 2200	0	0	2	5	4	14	6	3	0	0	0	0	0	0	0	34 38
2200 - 2215 2215 - 2230	0	0	2 5	6	11 10	8 10	9	0	0	0	0	0	0	0	0	38
2230 - 2245	0	0	0	5	8	9	4	1	0	0	0	0	0	0	0	27
2245 - 2300	0	0	0	3	7	17	5	1	0	0	0	0	0	0	0	33
2300 - 2315	0	0	0	5	5	7	6	2	1	0	0	0	0	0	0	26
2315 - 2330	0	0	1	3	4	17	4	0	1	0	0	0	0	0	0	30
2330 - 2345 2345 - 2400	0	0	0	3	3	7 11	7 5	2 5	0	0	0	0	0	0	0	22 30
2545 - 2400	U	U	U	U	9	-11	5	5	U	U	U	U	U	U	U	50

Number in Pace 3408 (64.6%)

Summary

		Northbound, (Movement 1.1)														Total
Time	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	l
Tuesday, March 29, 2022	11	13	92	310	984	1941	1351	430	97	19	11	2	0	1	10	5272
Session Total			92	310	984	1941	1351	430	97	19					10	5272

#### Speed Statistics by 15min

	Speed Statist	ics by 15iiiiii										
Time	Total	% Split	Min	Max	Mean	Median	15%	Perci	entile 85%	95%	>PSL (30mph)	>PSL % (30mph)
0000 - 0015	11	0.21	34	45	41	42	38	42	43	44	11	100
0015 - 0030	9	0.17	22	58	38	38	31	38	46	54	8	89
0030 - 0045 0045 - 0100	12 13	0.23 0.25	26 22	47 48	42 38	44 39	37 32	44 39	47 43	47 47	11 12	92 92
0100 - 0115	4	0.23	40	55	46	45	41	45	52	54	4	100
0115 - 0130	16	0.30	24	57	41	42	38	42	47	55	14	88
0130 - 0145	9	0.17	38	46	41	40	39	40	42	44	9	100
0145 - 0200	11	0.21	28	44	36	36	29	36	40	42	8	73
0200 - 0215 0215 - 0230	5 7	0.09	36 28	45 52	40 39	39 37	37 30	39 37	43 48	44 51	5 5	100 71
0230 - 0245	11	0.21	28	62	40	40	30	40	46	55	9	82
0245 - 0300	7	0.13	35	52	43	46	36	46	48	51	7	100
0300 - 0315	7	0.13	37	57	44	43	38	43	51	55	7	100
0315 - 0330	7	0.13	40	54	47	48	41	48	53	54	7	100
0330 - 0345 0345 - 0400	10 5	0.19	35 35	51 52	43 46	42 49	39 42	42 49	47 50	49 51	10 5	100 100
0400 - 0415	8	0.15	4	63	36	39	30	39	42	56	6	75
0415 - 0430	6	0.11	37	45	42	43	39	43	44	45	6	100
0430 - 0445	7	0.13	36	52	42	42	39	42	45	50	7	100
0445 - 0500	16	0.30	32	48	41	41	37	41	45	47	16	100
0500 - 0515 0515 - 0530	11 9	0.21 0.17	34 30	49 48	41 41	43 42	34 39	43 42	47 47	48 48	11 8	100 89
0530 - 0545	13	0.25	32	51	40	39	35	39	46	50	13	100
0545 - 0600	15	0.28	32	49	40	38	35	38	46	48	15	100
0600 - 0615	23	0.44	33	60	42	40	38	40	48	58	23	100
0615 - 0630 0630 - 0645	21 42	0.40	17 32	52 60	38 40	40 40	34 36	40 40	44 43	47 45	19 42	90 100
0630 - 0645 0645 - 0700	42 40	0.80	32 25	60 93	40 40	40 39	36 34	40 39	43 43	45 47	42 37	100 93
0700 - 0715	71	1.35	25	50	39	39	35	39	45	47	68	96
0715 - 0730	74	1.40	30	48	39	39	34	39	44	46	71	96
0730 - 0745	76	1.44	30	52	39	39	35	39	44	46	75	99
0745 - 0800 0800 - 0815	75 72	1.42	28 27	53 95	40 40	40 39	34 35	40 39	45 45	48 50	72 69	96 96
0800 - 0815 0815 - 0830	110	2.09	27	95 52	38	39	35	39	45 42	43	105	96 95
0830 - 0845	89	1.69	30	55	40	39	34	39	43	49	88	99
0845 - 0900	72	1.37	30	53	39	39	35	39	43	45	70	97
0900 - 0915	68	1.29	29	52	39	40	35	40	43	46	67	99
0915 - 0930	72	1.37	25	61	39	40	34	40	44	49	69	96 94
0930 - 0945 0945 - 1000	69 60	1.31	26 28	55 50	39 39	39 40	34 34	39 40	44 44	48 48	65 57	94 95
1000 - 1015	64	1.21	28	83	39	38	31	38	45	50	56	88
1015 - 1030	78	1.48	24	61	38	39	34	39	42	46	71	91
1030 - 1045	73	1.38	22	49	36	35	29	35	42	45	59	81
1045 - 1100 1100 - 1115	71 78	1.35 1.48	28 22	58 68	38 38	39 37	34 33	39 37	42 43	47 46	66 74	93 95
1115 - 1130	89	1.69	21	49	36	36	30	36	41	47	75	84
1130 - 1145	84	1.59	25	52	38	38	34	38	43	47	80	95
1145 - 1200	83	1.57	20	57	37	37	33	37	42	45	78	94
1200 - 1215	101	1.92	20	63	37	37	31	37	43	50	87	86
1215 - 1230 1230 - 1245	100 83	1.90 1.57	9 13	80 52	37 35	36 37	31 26	36 37	42 42	45 45	85 64	85 77
1245 - 1300	88	1.67	25	51	36	36	31	36	40	45	75	85
1300 - 1315	85	1.61	22	65	37	37	32	37	43	46	76	89
1315 - 1330	90	1.71	25	50	36	36	30	36	40	44	76	84
1330 - 1345 1345 - 1400	81 66	1.54 1.25	29 17	47 50	37 38	37 39	32 34	37 39	42 43	44 45	75 60	93 91
1400 - 1415	86	1.63	22	57	38	37	31	37	43	45	74	86
1415 - 1430	62	1.18	24	53	39	39	34	39	45	50	57	92
1430 - 1445	66	1.25	13	61	37	37	30	37	43	50	54	82
1445 - 1500	73	1.38	25	89	39	38	32	38	45	50	63	86
1500 - 1515 1515 - 1530	111 97	2.11 1.84	21 29	53 48	37 39	37 38	32 35	37 38	42 43	45 47	99 95	89 98
1530 - 1545	100	1.84	29	48 85	36	35	29	35	43	47	83	83
1545 - 1600	81	1.54	21	86	39	39	34	39	43	47	73	90
1600 - 1615	98	1.86	8	78	37	38	32	38	42	45	84	86
1615 - 1630	99	1.88	21	50	38	38	33	38	42	46	90	91
1630 - 1645 1645 - 1700	105 86	1.99 1.63	21 21	53 53	38 37	39 39	32 32	39 39	43 41	48 45	98 75	93 87
1700 - 1715	88	1.63	22	89	38	36	31	36	41	48	75	85
1715 - 1730	121	2.30	21	53	37	37	31	37	43	45	104	86
1730 - 1745	98	1.86	27	85	39	39	34	39	43	47	91	93
1745 - 1800	103	1.95	17	48	36	36	31	36	42	47	88	85
1800 - 1815 1815 - 1830	111 97	2.11 1.84	7 19	53 51	36 36	38 37	30 30	38 37	42 42	46 47	91 82	82 85
1830 - 1845	62	1.18	22	47	37	38	32	38	42	45	54	87
1845 - 1900	71	1.35	4	48	37	38	33	38	43	45	64	90
1900 - 1915	78	1.48	27	52	37	37	32	37	43	45	70	90
1915 - 1930	71	1.35	24	56 63	39	38	33	38	45	49	62	87
1930 - 1945 1945 - 2000	53 67	1.01 1.27	28 23	62 56	39 37	39 38	33 34	39 38	43 41	48 45	50 61	94 91
2000 - 2015	58	1.10	22	47	36	36	31	36	40	45	51	88
2015 - 2030	46	0.87	17	48	35	37	30	37	41	45	36	78
2030 - 2045	47	0.89	27	47	36	37	30	37	42	46	36	77
2045 - 2100	50	0.95	25	45	34	34	30	34	39	42	39	78
2100 - 2115 2115 - 2130	45 46	0.85 0.87	22 21	48 47	35 35	35 35	29 30	35 35	42 40	45 45	36 36	80 78
2130 - 2145	47	0.89	21	50	35	35	29	35	43	49	32	68
2145 - 2200	34	0.64	22	49	36	38	27	38	40	46	26	76
2200 - 2215	38	0.72	19	48	34	34	28	34	40	43	26	68
2215 - 2230	32	0.61	24	42	33	34	25	34	39	40	21	66
2230 - 2245 2245 - 2300	26 34	0.49	28 26	45 48	35 36	35 37	29 30	35 37	40 40	43 43	18 28	69 82
2300 - 2315	24	0.46	26	50	36	36	29	36	40	45	17	71
2315 - 2330	32	0.61	24	52	37	37	32	37	41	45	27	84
2330 - 2345	22	0.42	25	48	38	39	32	39	42	47	19	86
2345 - 2400	30	0.57	30	49	38	38	33	38	44	47	29	97
Session Total	5272		4	95	38	38	32	38	43	47	4682	89
Jession rotal	JE12				- 30	. 36	- 32	- 30			1002	- 65

#### Bi-Directional Speed & Class Count | | SB WB Speed 15min

Atlanta, GA

Marr Traffic DATA COLLECTION www.marrtraffic.com

Site 1	
Eller on add to decembed the of Anna?	

Ellsworth Industrial Blvd NW, south of Old Chattahoochee Ave NW

Date
Tuesday, March 29, 2022

Lat/Long 33.797841°, -84.427033° Click here for Map

0000 - 2400 (Weekday 24h Session) SB WB Speed 15min

							Southh	ound, (Moven	ont 1 2)							15min
Time	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph			55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	Total
0000 - 0015	0	0	0	3	4	7	4	2	1	0	0	0	0	0	0	21
0015 - 0030	0	0	3	2	4	7	6	2	1	0	0	0	0	0	0	25
0030 - 0045	0	0	0	3	1	4	4	3	0	1	1	0	0	1	0	18
0045 - 0100 0100 - 0115	0	0	0	0	0	5	5	2	0	0	0	0	0	0	0	12 5
0115 - 0130	1	0	0	0	1	1	2	3	0	0	0	0	0	0	0	8
0130 - 0145	0	0	0	0	0	0	1	1	1	0	0	0	0	0	0	3
0145 - 0200	0	0	0	0	0	0	2	1	0	0	0	0	0	0	0	3
0200 - 0215	0	0	2	0	0	0	2	1	2	0	0	0	0	0	0	7
0215 - 0230	0	0	0	0	1	1	0	0	0	0	0	0	0	0	1	3
0230 - 0245	0	0	0	0	1	3	1	1	0	0	0	0	0	0	0	6
0245 - 0300	0	0	1	0	0	3	1	3	0	0	0	0	0	0	0	8
0300 - 0315 0315 - 0330	0	0	0	0	0	0	2	1	0	1 0	0	0	0	0	0	6 3
0330 - 0345	0	0	0	0	0	1	1	1	1	1	0	0	0	0	0	5
0345 - 0400	0	0	0	2	1	2	0	3	0	0	0	0	0	0	0	8
0400 - 0415	0	0	0	0	1	6	2	0	0	0	0	0	0	0	0	9
0415 - 0430	0	0	0	0	2	1	2	0	0	0	0	0	0	0	0	5
0430 - 0445	0	0	0	0	0	0	3	5	0	0	0	0	0	0	0	8
0445 - 0500	0	0	0	0	0	4	4	1	0	1	0	0	0	0	0	10
0500 - 0515	0	0	0	0	1	5	5	3	1	1	0	0	0	0	0	16
0515 - 0530	0	0	0	1	2	5 9	5	2	0	2	0	0	0	0	0	17
0530 - 0545 0545 - 0600	0	0	0	1	1 5	15	6 5	3 4	2	0	0	0	0	0	0	22 31
0600 - 0615	0	0	0	0	2	9	9	4	3	1	0	0	0	0	0	28
0615 - 0630	0	1	0	0	4	10	12	7	1	1	0	0	0	0	0	36
0630 - 0645	0	0	0	2	6	12	11	3	0	1	0	0	0	0	0	35
0645 - 0700	0	0	0	2	12	16	16	2	0	0	1	0	0	0	0	49
0700 - 0715	0	0	0	10	15	18	13	1	0	1	0	0	0	0	0	58
0715 - 0730	0	0	0	3	8	15	13	1	1	0	0	0	0	0	0	41
0730 - 0745	0	0	0	3	15	16	13	5	0	0	0	0	0	0	1	53
0745 - 0800 0800 - 0815	0	0	0	0	8	25 28	6 20	3 5	0	0	0	0	0	0	0	45 63
0800 - 0815 0815 - 0830	0	0	2	1	8 17	28	12	2	0	0	0	0	0	0	0	63 57
0830 - 0845	0	0	0	4	21	28	7	2	0	0	0	0	0	0	0	62
0845 - 0900	1	0	1	0	24	30	19	5	0	0	0	0	0	0	0	80
0900 - 0915	1	0	1	3	13	37	28	4	1	0	0	0	0	0	0	88
0915 - 0930	1	0	1	5	20	26	23	1	1	0	0	0	0	0	0	78
0930 - 0945	1	0	1	7	17	26	19	3	0	1	0	0	0	0	0	75
0945 - 1000	0	0	0	1	12	24	20	6	0	0	0	1	0	0	0	64
1000 - 1015	1	1	0	18	19	24	6	3	1	0	0	0	0	0	0	73
1015 - 1030 1030 - 1045	0	2 0	0	13 2	26 16	27 33	3 11	2	0	0	0	0	0	0	1 0	76 67
1045 - 1100	0	1	5	12	28	19	4	5	0	2	0	0	0	0	0	76
1100 - 1115	0	0	0	4	13	30	23	6	0	1	0	0	0	0	1	78
1115 - 1130	1	0	0	8	18	19	16	6	1	0	0	0	0	0	0	69
1130 - 1145	0	0	0	7	24	35	16	6	0	1	0	0	0	0	0	89
1145 - 1200	0	2	0	3	19	25	26	4	2	0	0	0	0	0	0	81
1200 - 1215	0	0	0	6	20	46	16	3	2	1	0	0	0	0	0	94
1215 - 1230	4	0	1	12	24	26	13	2	1	1	0	0	0	0	0	84
1230 - 1245 1245 - 1300	0	0	2	5 8	14 23	26 28	15 21	4	0	0	0	0	0	0	0	66 91
1300 - 1315	1	0	1	8	39	27	13	8	0	0	0	0	0	0	0	97
1315 - 1330	0	0	3	14	30	29	13	4	1	0	0	0	0	0	0	94
1330 - 1345	0	0	3	8	34	38	12	11	2	0	0	0	0	0	0	108
1345 - 1400	0	0	1	7	28	39	15	4	1	0	0	0	0	0	0	95
1400 - 1415	0	0	0	9	27	36	17	4	2	0	0	0	1	0	0	96
1415 - 1430	1	0	5	8	11	28	24	7	3	0	0	0	0	0	0	87
1430 - 1445	0	0	4	9	20	22	14	5	1	0	0	0	0	0	0	75
1445 - 1500	0	1	2	6	15 33	34	14 18	4	1 2	0	0	0	0	0	0	77 89
1500 - 1515 1515 - 1530	0	0	6	8	33 11	20 34	18	3 6	2	1	0	0	0	0	0	89 85
1530 - 1545	0	0	2	5	43	28	18	1	2	1	0	0	0	0	0	100
1545 - 1600	0	0	4	9	14	30	24	7	1	0	1	0	0	0	0	90
1600 - 1615	0	2	2	19	30	26	11	6	2	1	0	0	0	0	0	99
1615 - 1630	0	0	1	2	9	40	27	9	1	0	0	0	0	0	0	89
1630 - 1645	0	0	1	2	12	37	17	11	1	0	0	0	0	0	0	81
1645 - 1700 1700 - 1715	0	0	0	8	17 34	32	25	4	3	0	0	0	0	0	0	89 119
1700 - 1715 1715 - 1730	0	1	3	6 12	34 13	45 40	26 20	6 5	1	0	0	0	0	0	0	119 95
1730 - 1745	0	1	0	8	28	30	15	9	2	2	0	0	0	0	0	95
1745 - 1800	0	1	4	3	28	42	32	2	2	1	0	0	0	0	0	115
1800 - 1815	0	0	4	5	22	50	28	9	2	0	0	0	0	0	0	120
1815 - 1830	0	1	5	13	36	32	16	6	1	0	0	0	0	0	1	111
1830 - 1845	0	0	2	9	15	37	14	5	0	0	0	0	0	0	0	82
1845 - 1900	0	0	0	12	33	28	11	2	1	0	0	0	0	0	0	87
1900 - 1915	0	0	4	12	20	24	14	3	1	1	0	0	0	0	0	79
1915 - 1930 1930 - 1945	0	0	1 2	2 11	16 25	15 29	24 13	11 3	1	1 0	0	0	0	0	0	71 85
1930 - 1945	1	1	1	7	25	23	14	6	2	0	0	0	0	0	0	80
2000 - 2015	0	0	1	7	19	20	7	5	1	0	0	0	0	0	0	60
2015 - 2030	0	0	1	7	20	29	8	2	0	1	0	0	0	0	0	68
2030 - 2045	0	0	1	4	20	28	17	2	0	0	0	0	0	0	0	72
2045 - 2100	0	0	2	5	10	29	13	4	1	0	0	0	0	0	0	64
2100 - 2115	0	0	1	7	17	17	16	5	1	1	0	0	0	0	0	65
2115 - 2130	0	0	1	10	20	24	9	8	2	0	0	0	0	0	0	74
2130 - 2145 2145 - 2200	0	0	4	4	13 13	18	16	3 2	1	0	0	0	0	0	0	59 46
2145 - 2200 2200 - 2215	0	0	1	6	13 5	16 14	7 15	6	0	0	0	0	0	0	0	45
2215 - 2230	0	0	0	0	7	14	7	6	1	0	0	0	0	0	0	35
2230 - 2245	0	0	0	3	15	13	9	3	0	0	1	0	0	0	0	44
2245 - 2300	0	1	2	5	3	16	6	2	0	0	0	0	0	0	0	35
2300 - 2315	0	0	2	3	12	8	10	5	2	0	0	0	0	0	0	42
2315 - 2330	4	0	0	6	9	6	3	3	1	0	0	0	0	0	0	32
2330 - 2345	0	0	1	5	4	6	4	1	1	1	0	0	0	0	0	23
2345 - 2400	0	0	1	1	3	8	6	1	1	0	0	0	0	0	0	21

| 22 | 18 | 109 | 462 | 1331 | 1921 | 1135 | 366 | 82 | 30 | 5 | 2 | 1 | 1 | 5 | **5490** 10mph Pace Speed Number in Pace 32-41 3410 (62.1%)

Summary

							Southb	ound, (Movem	ent 1.2)							Total
Time	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	l
Tuesday, March 29, 2022	22	18	109	462	1331	1921	1135	366	82	30	5	2	1	1	5	5490
Session Total		18	109	462	1331	1921	1135	366	82	30						5490

#### Speed Statistics by 15min

	Speed Statist	ics by 15iiiiii										
Time	Total	% Split	Min	Max	Mean	Median	15%	Perc 50%	entile 85%	95%	>PSL (30mph)	>PSL % (30mph)
0000 - 0015	21	0.38	27	51	37	37	32	37	43	49	18	(SUMPH) 86
0015 - 0030	25	0.46	22	51	36	36	29	36	42	48	20	80
0030 - 0045	18	0.33	27	76	43	41	31	41	51	64	15	83
0045 - 0100 0100 - 0115	12 5	0.22	37 33	48 51	41 43	40 45	38 38	40 45	44 47	47 50	12 5	100 100
0105 - 0115	8	0.05	14	47	39	42	32	42	47	47	7	88
0130 - 0145	3	0.05	43	51	46	45	44	45	49	50	3	100
0145 - 0200	3	0.05	40	47	42	40	40	40	45	46	3	100
0200 - 0215	7	0.13	22	53	40	43	24	43	52	53	5	71
0215 - 0230 0230 - 0245	3 6	0.05 0.11	32 31	81 45	50 37	37 37	34 34	37 37	68 41	77 44	3 6	100 100
0245 - 0300	8	0.11	24	46	39	41	35	41	45	46	7	88
0300 - 0315	6	0.11	41	57	48	48	41	48	53	56	6	100
0315 - 0330	3	0.05	31	48	40	42	34	42	46	47	3	100
0330 - 0345	5	0.09	39	57	48	47	41	47	55	56	5	100
0345 - 0400 0400 - 0415	8	0.15 0.16	27 30	48 42	38 38	37 38	29 35	37 38	47 41	48 42	6 8	75 89
0415 - 0430	5	0.09	30	43	37	39	31	39	42	43	4	80
0430 - 0445	7	0.13	43	47	45	46	44	46	46	47	7	100
0445 - 0500	11	0.20	35	57	42	40	37	40	45	52	11	100
0500 - 0515	15	0.27	34	55	42	41	36	41	49	52	15	100
0515 - 0530 0530 - 0545	18 22	0.33	29 27	57 54	41 41	41 40	35 35	41 40	47 45	55 52	17 21	94 95
0545 - 0600	31	0.56	29	52	39	39	34	39	45	48	29	94
0600 - 0615	28	0.51	31	57	42	41	35	41	47	52	28	100
0615 - 0630	34	0.62	19	58	41	41	35	41	46	49	33	97
0630 - 0645	37	0.67	27	58	39 38	39	33	39	43	45 45	34	92
0645 - 0700 0700 - 0715	49 58	0.89 1.06	27 27	62 55	38 35	38 35	34 29	38 35	43 41	45	43 42	88 72
0715 - 0730	41	0.75	27	51	38	38	32	38	43	43	37	90
0730 - 0745	52	0.95	27	86	38	38	32	38	43	46	47	90
0745 - 0800	46	0.84	19	47	37	38	33	38	40	45	42	91
0800 - 0815 0815 - 0830	61 59	1.11	22 22	62 47	39 36	38 36	35 33	38 36	42 42	47 43	60 53	98 90
0815 - 0830 0830 - 0845	62	1.07	26	47	36 35	35	33	36 35	42 39	43	55	90 89
0845 - 0900	80	1.46	4	49	37	38	32	38	41	45	74	93
0900 - 0915	88	1.60	12	51	38	38	33	38	42	45	82	93
0915 - 0930	75	1.37	24	50	37	37	31	37	42	44	66	88
0930 - 0945 0945 - 1000	78 64	1.42 1.17	4 29	58 66	36 39	37 39	30 34	37 39	42 43	44 47	65 62	83 97
1000 - 1015	73	1.33	9	50	34	34	28	34	39	45	49	67
1015 - 1030	76	1.38	16	82	34	34	27	34	39	40	56	74
1030 - 1045	62	1.13	12	52	37	36	32	36	42	45	58	94
1045 - 1100	81	1.48	16	57	34	33	27	33	39	46	54	67
1100 - 1115 1115 - 1130	76 71	1.38 1.29	27 9	108 50	39 37	38 37	32 30	38 37	43 43	46 46	70 57	92 80
1130 - 1145	89	1.62	26	57	37	36	32	36	41	47	81	91
1145 - 1200	81	1.48	17	53	37	37	32	37	43	47	72	89
1200 - 1215	94	1.71	27	55	37	37	30	37	42	45	79	84
1215 - 1230	81	1.48	1 21	56	33 37	34	29	34	40	43	57	70
1230 - 1245 1245 - 1300	69 91	1.26 1.66	14	47 59	36	37 36	32 30	37 36	42 42	45 45	60 75	87 82
1300 - 1315	97	1.77	4	48	35	34	30	34	42	46	82	85
1315 - 1330	94	1.71	22	54	35	35	29	35	40	43	72	77
1330 - 1345	102	1.86	22	54	36	35	30	35	43	48	85	83
1345 - 1400 1400 - 1415	101 93	1.84 1.69	24 27	50 72	36 36	36 35	31 30	36 35	40 41	44 45	88 78	87 84
1415 - 1430	90	1.64	11	53	37	39	29	39	43	47	75	83
1430 - 1445	75	1.37	21	50	35	35	29	35	43	47	58	77
1445 - 1500	77	1.40	15	51	36	37	30	37	41	45	63	82
1500 - 1515	89	1.62	7	53	35	34	30	34	42	45	72	81
1515 - 1530 1530 - 1545	80 105	1.46 1.91	22 23	55 56	37 36	37 34	29 31	37 34	42 42	48 44	66 92	83 88
1545 - 1600	90	1.64	23	64	37	38	30	38	42	44	75	88
1600 - 1615	99	1.80	19	56	35	34	29	34	42	47	74	75
1615 - 1630	89	1.62	22	50	39	39	35	39	44	47	82	92
1630 - 1645	76 94	1.38	24	50 52	39 37	39	34 32	39 37	44 42	48 47	72 84	95
1645 - 1700 1700 - 1715	94 109	1.71	26 27	52 69	37 37	37 37	32 32	37	42	47	98	89 90
1715 - 1730	105	1.91	15	50	36	36	30	36	41	46	87	83
1730 - 1745	95	1.73	19	58	37	36	31	36	43	48	81	85
1745 - 1800	115	2.09	16	55	37	37	32	37	42	44	103	90
1800 - 1815 1815 - 1830	120 105	2.19 1.91	22 19	53 84	37 35	37 34	32 29	37 34	42 42	47 45	107 74	89 70
1815 - 1830 1830 - 1845	105 88	1.91	19 23	48	36	37	31	37	42	45 45	76	70 86
1845 - 1900	87	1.58	25	52	35	34	30	34	40	43	69	79
1900 - 1915	79	1.44	20	58	35	35	29	35	42	45	60	76
1915 - 1930	71	1.29	24	55	39	40	34	40	45	49	66	93
1930 - 1945 1945 - 2000	81 84	1.48 1.53	4 14	50 53	35 36	35 36	29 30	35 36	40 43	43 46	65 68	80 81
2000 - 2015	58	1.53	24	47	35	35	30	35	43	45	45	78
2015 - 2030	70	1.28	22	56	36	36	30	36	40	45	58	83
2030 - 2045	72	1.31	21	45	36	37	32	37	42	43	65	90
2045 - 2100	64	1.17	23	52	37	37	33	37	42	46	57	89
2100 - 2115 2115 - 2130	65 70	1.18	24 21	56 52	37 36	37 37	30 30	37 37	42 43	48 47	53 57	82 81
2115 - 2130 2130 - 2145	70 63	1.28	21 21	52 53	36 36	37 37	30 31	37 37	43 43	47 45	57 54	81 86
2145 - 2200	46	0.84	24	50	36	35	30	35	40	45	37	80
2200 - 2215	45	0.82	23	48	38	39	32	39	43	47	40	89
2215 - 2230	35	0.64	30	50	39	38	34	38	45	47	33	94
2230 - 2245	40	0.73	25	48	36	36	30	36	42	45 47	33	83
2245 - 2300 2300 - 2315	39 41	0.71 0.75	18 22	62 50	36 37	37 36	29 30	37 36	40 43	47	31 34	79 83
2315 - 2330	33	0.60	0	51	32	34	26	34	44	47	22	67
2330 - 2345	23	0.42	24	55	36	36	26	36	40	51	17	74
2345 - 2400	21	0.38	23	50	37	38	34	38	42	48	18	86
Session Total	5490	1	0	108	37	37	30	37	42	47	4653	85
Jession Total	3430		- 0	100	- 3/	- 3/	- 5U	- 5/	***	**/	4033	- 63

#### Bi-Directional Speed & Class Count || Bi-Directional Speed 15min



Site 1

Ellsworth Industrial Blvd NW,
south of Old Chattahoochee Ave NW

Date Tuesday, March 29, 2022

Lat/Long 33.797841°, -84.427033°



0000 - 2400 (Weekday 24h Session) Bi-Directional Speed 15min

								ound, (Moven								15min
Time 0000 - 0015	5-14 mph 0	15-19 mph 0	20-24 mph 0	25-29 mph 3	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph 0	60-64 mph 0	65-69 mph 0	70-74 mph 0	75-79 mph 0	80-99 mph 0	Total 32
0005 - 0015	0	0	4	2	7	9	7	3	1	1	0	0	0	0	0	34
0030 - 0045	0	0	0	4	2	4	9	8	0	1	1	0	0	1	0	30
0045 - 0100	0	0	1	0	2	10	8	4	0	0	0	0	0	0	0	25
0100 - 0115	0	0	0	0	1	0	3	3	1	1	0	0	0	0	0	9
0115 - 0130 0130 - 0145	0	0	0	0	0	4 2	9	5 2	1	0	0	0	0	0	0	24 12
0145 - 0200	0	0	0	3	0	5	5	1	0	0	0	0	0	0	0	14
0200 - 0215	0	0	2	0	0	3	3	2	2	0	0	0	0	0	0	12
0215 - 0230	0	0	0	1	2	3	1	1	1	0	0	0	0	0	1	10
0230 - 0245	0	0	0	2	2	5	5	2	0	0	1	0	0	0	0	17
0245 - 0300	0	0	1	0	0	5	2	6	1	0	0	0	0	0	0	15
0300 - 0315 0315 - 0330	0	0	0	0	0	3 0	3	2	3	2	0	0	0	0	0	13 10
0330 - 0345	0	0	0	0	0	4	5	3	2	1	0	0	0	0	0	15
0345 - 0400	0	0	0	2	1	3	0	6	1	0	0	0	0	0	0	13
0400 - 0415	1	0	0	0	3	7	5	0	0	0	1	0	0	0	0	17
0415 - 0430	0	0	0	0	2	2	6	1	0	0	0	0	0	0	0	11
0430 - 0445 0445 - 0500	0	0	0	0	0	2 9	8 10	5 4	0	0	0	0	0	0	0	16 25
0500 - 0515	0	0	0	0	4	8	7	7	1	1	0	0	0	0	0	28
0515 - 0530	0	0	0	1	3	6	9	4	0	2	0	0	0	0	0	25
0530 - 0545	0	0	0	1	3	14	9	5	3	0	0	0	0	0	0	35
0545 - 0600	0	0	0	1	6	22	9	7	1	0	0	0	0	0	0	46
0600 - 0615 0615 - 0630	0	0 2	0	0	3	17 16	15 22	9 10	4 2	2	1 0	0	0	0	0	51 63
0615 - 0630	0	0	0	2	9	26	28	3	1	1	1	0	0	0	0	71
0645 - 0700	0	0	0	4	20	31	25	7	0	0	1	0	0	0	1	89
0700 - 0715	0	0	0	13	20	51	32	11	1	1	0	0	0	0	0	129
0715 - 0730	0	0	0	3	20	43	37	11	1	0	0	0	0	0	0	115
0730 - 0745	0	0	0	3	22	58	31	16	2	0	0	0	0	0	1	133
0745 - 0800 0800 - 0815	0	0	0	3	20 14	46 67	32 38	13 12	4	0	0 2	0	0	0	0	116 142
0815 - 0830	0	0	3	1	35	61	54	5	1	0	0	0	0	0	0	160
0830 - 0845	0	0	0	4	35	62	35	10	4	1	0	0	0	0	0	151
0845 - 0900	1	0	1	0	34	59	45	11	1	0	0	0	0	0	0	152
0900 - 0915	1	0	1	4	18	64	56	10	2	0	0	0	0	0	0	156
0915 - 0930 0930 - 0945	1	0	1	7	30	51 47	52 41	8	4 2	0 2	1 0	0	0	0	0	155 139
0945 - 1000	0	0	0	8	28 21	42	44	11	2	0	0	1	0	0	0	124
1000 - 1015	1	1	0	23	32	45	17	13	3	1	0	0	0	0	1	137
1015 - 1030	0	2	4	15	37	53	32	8	1	0	1	0	0	0	1	154
1030 - 1045	1	0	1	14	32	59	24	9	2	0	0	0	0	0	0	142
1045 - 1100	0	1	5	14	42	44	27	8	1	3	0	0	0	0	0	145
1100 - 1115 1115 - 1130	0	0	2	6 17	32 38	69 48	34 32	14 14	1	2 0	0	1 0	0	0	1 0	162 152
1130 - 1145	0	0	0	10	42	68	38	12	2	1	0	0	0	0	0	173
1145 - 1200	0	2	2	5	36	61	46	7	4	1	0	0	0	0	0	164
1200 - 1215	0	0	3	15	38	83	38	9	6	2	1	0	0	0	0	195
1215 - 1230	5	1	2	22	51	67	32	7	2	2	0	0	0	0	1	192
1230 - 1245 1245 - 1300	1	2 0	6	9 17	25 42	57 69	29 33	11 10	1 2	0	0	0	0	0	0	141 179
1300 - 1315	1	0	2	16	54	57	35	16	0	0	0	1	0	0	0	182
1315 - 1330	0	0	3	23	52	68	30	6	2	0	0	0	0	0	0	184
1330 - 1345	0	0	3	12	63	67	34	15	2	0	0	0	0	0	0	196
1345 - 1400	0	1	2	10	34	62	32	11	2	0	0	0	0	0	0	154
1400 - 1415 1415 - 1430	0	0	4 6	14 9	45 21	76 49	27 39	14 13	3 7	0	0	0	1 0	0	0	186 145
1430 - 1445	1	2	5	11	37	49	33	6	4	0	1	0	0	0	0	141
1445 - 1500	0	1	2	14	22	62	32	12	4	0	0	0	0	0	1	150
1500 - 1515	1	1	4	16	60	68	39	7	4	0	0	0	0	0	0	200
1515 - 1530	0	0	6	9	27	81	43	21	2	1	0	0	0	0	0	190
1530 - 1545 1545 - 1600	0	0	6	17	64 27	60 53	34 54	4 15	3	2	1	0	0	0	1	192 171
1545 - 1600 1600 - 1615	0	0	6 7	13 24	50	53 62	35	10	3	0	0	0	0	1	0	197
1615 - 1630	0	0	3	8	25	78	53	19	2	0	0	0	0	0	0	188
1630 - 1645	0	0	4	5	30	73	52	22	5	0	0	0	0	0	0	191
1645 - 1700	0	0	1	15	32	63	48	5	6	0	0	0	0	0	0	170
1700 - 1715 1715 - 1730	0	0	1 6	11 21	61 33	79 86	46 47	12 10	2	1 0	0	0	0	0	2	216 207
1730 - 1745	0	1	0	10	45	70	46	14	4	2	0	0	0	0	1	193
1745 - 1800	0	2	6	12	52	76	56	11	2	1	0	0	0	0	0	218
1800 - 1815	5	0	7	12	43	84	59	17	4	0	0	0	0	0	0	231
1815 - 1830	0	2	10	19	64	64	38	12	3	0	0	0	0	0	1	213
1830 - 1845 1845 - 1900	0	0	4	14 13	22 46	59 53	30 33	10 7	0	0	0	0	0	0	0	139 158
1900 - 1915	0	0	4	19	37	49	35	9	3	1	0	0	0	0	0	157
1915 - 1930	0	0	2	9	26	37	42	20	4	2	0	0	0	0	0	142
1930 - 1945	1	0	2	12	34	49	32	6	3	0	1	0	0	0	0	140
1945 - 2000	1	1	3	11	32	57	27	10	2	1	0	0	0	0	0	145
2000 - 2015 2015 - 2030	0	0 2	4 3	10 10	33 30	48 41	15 20	9 5	0	0	0	0	0	0	0	120 112
2015 - 2030 2030 - 2045	0	0	1	10	30	41 41	20 26	7	0	0	0	0	0	0	0	112
2045 - 2100	0	0	2	12	31	46	16	6	1	0	0	0	0	0	0	114
2100 - 2115	0	0	2	14	27	32	25	8	1	1	0	0	0	0	0	110
2115 - 2130	0	0	3	16	34	41	18	11	2	0	0	0	0	0	0	125
2130 - 2145	0	0	8	7	26	26	25	7	2	0	0	0	0	0	0	101
2145 - 2200	0	0	3	11	17	30	13 24	5 7	1	0	0	0	0	0	0	80 83
2200 - 2215 2215 - 2230	0	0	5	10 3	16 17	22 24	24 11	6	0	0	0	0	0	0	0	83 67
2230 - 2245	0	0	0	8	23	22	13	4	0	0	1	0	0	0	0	71
2245 - 2300	0	1	2	8	10	33	11	3	0	0	0	0	0	0	0	68
2300 - 2315	0	0	2	8	17	15	16	7	3	0	0	0	0	0	0	68
2315 - 2330	4	0	1	9	13	23	7	3	2	0	0	0	0	0	0	62
2330 - 2345 2345 - 2400	0	0	1	8 1	7 12	13 19	11 11	3 6	1	1 0	0	0	0	0	0	45 51
2545 - 2400						4.9				,	,	,	,	J	,	

4 1 2 15 10762 Number in Pace 6764 (62.9%)

Summan

							Southb	ound, (Moven	nent 1.2)							Total
Time	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	l
Tuesday, March 29, 2022	33	31	201	772	2315	3862	2486	796	179	49	16	4	1	2	15	10762
Session Total	33	31	201	772	2315	3862	2486	796	179	49	16					10762

#### Speed Statistics by 15min

	Speed Statist	ics by 15min										
T	Total	% Split	Min	Max	Mean	Median	450/		entile	050/	>PSL	>PSL %
Time 0000 - 0015	32	0.30	27	51	39	39	15% 34	50% 39	85% 43	95% 47	(30mph) 29	(30mph) 91
0015 - 0030	34	0.32	22	58	37	37	29	37	43	50	28	82
0030 - 0045	30	0.28	26	76	42	43	32	43	47	59	26	87
0045 - 0100 0100 - 0115	25 9	0.23	22 33	48 55	39 45	39 45	35 40	39 45	44 50	48 53	24 9	96 100
0100 - 0115	24	0.08	33 14	57	45	45	35	45	47	53	21	88
0130 - 0145	12	0.11	38	51	42	41	40	41	45	48	12	100
0145 - 0200	14	0.13	28	47	37	39	29	39	40	45	11	79
0200 - 0215	12	0.11	22	53	40	42	32	42	47	52	10	83
0215 - 0230 0230 - 0245	10 17	0.09 0.16	28 28	81 62	42 39	37 39	31 31	37 39	50 45	68 51	8 15	80 88
0245 - 0300	15	0.14	24	52	41	43	35	43	47	49	14	93
0300 - 0315	13	0.12	37	57	46	45	38	45	53	57	13	100
0315 - 0330	10	0.09	31	54	45	47	40	47	52	54	10	100
0330 - 0345	15	0.14	35	57	44	42	39	42	51	54	15	100
0345 - 0400 0400 - 0415	13 17	0.12	27 4	52 63	41 37	47 38	33 31	47 38	49 42	50 46	11 14	85 82
0415 - 0430	11	0.10	30	45	40	42	35	42	44	45	10	91
0430 - 0445	14	0.13	36	52	44	44	40	44	46	49	14	100
0445 - 0500	27	0.25	32	57	41	40	36	40	45	48	27	100
0500 - 0515 0515 - 0530	26 27	0.24	34 29	55 57	42 41	42 41	35 35	42 41	48 47	51 53	26 25	100 93
0530 - 0545	35	0.33	27	54	40	39	35	39	45	51	34	97
0545 - 0600	46	0.43	29	52	39	39	35	39	45	48	44	96
0600 - 0615	51	0.47	31	60	42	40	37	40	48	55	51	100
0615 - 0630 0630 - 0645	55 79	0.51	17 27	58 60	40 39	40 39	34 34	40 39	46 43	49 45	52 76	95 96
0645 - 0700	79 89	0.73	27	93	39	39	34	39	43	45	76 80	96 90
0700 - 0715	129	1.20	25	55	37	37	31	37	42	47	110	85
0715 - 0730	115	1.07	27	51	38	39	34	39	43	46	108	94
0730 - 0745	128	1.19	27	86	39	39	34	39	43	47	122	95
0745 - 0800 0800 - 0815	121 133	1.12 1.24	19 22	53 95	39 40	39 39	34 35	39 39	43 44	47 48	114 129	94 97
0815 - 0830	169	1.57	22	52	38	39	33	39	42	43	158	93
0830 - 0845	151	1.40	26	55	38	37	34	37	42	47	143	95
0845 - 0900	152	1.41	4	53	38	38	33	38	43	45	144	95
0900 - 0915 0915 - 0930	156 147	1.45 1.37	12 24	52 61	38 38	39 38	34 33	39 38	42 43	46 47	149 135	96 92
0930 - 0945	147	1.37	4	58	38	37	32	37	43	46	130	88
0945 - 1000	124	1.15	28	66	39	39	34	39	43	48	119	96
1000 - 1015	137	1.27	9	83	36	36	29	36	43	48	105	77
1015 - 1030	154	1.43	16	82	36	36	30	36	41	45	127	82
1030 - 1045 1045 - 1100	135 152	1.25 1.41	12 16	52 58	36 36	36 36	31 30	36 36	42 42	45 46	117 120	87 79
1100 - 1115	154	1.43	22	108	38	37	32	37	43	46	144	94
1115 - 1130	160	1.49	9	50	36	37	30	37	43	46	132	83
1130 - 1145	173	1.61	25	57	37	37	32	37	42	47	161	93
1145 - 1200 1200 - 1215	164 195	1.52 1.81	17 20	57 63	37 37	37 37	32 31	37 37	43 42	47 48	150 166	91 85
1215 - 1230	181	1.68	1	80	35	35	30	35	41	45	142	78
1230 - 1245	152	1.41	13	52	36	37	30	37	42	45	124	82
1245 - 1300	179	1.66	14	59	36	36	30	36	42	45	150	84
1300 - 1315 1315 - 1330	182 184	1.69 1.71	4 22	65 54	36 35	37 36	31 30	37 36	43 40	46 44	158 148	87 80
1330 - 1345	183	1.70	22	54	37	36	32	36	43	46	160	87
1345 - 1400	167	1.55	17	50	37	37	32	37	42	45	148	89
1400 - 1415	179	1.66	22	72	37	37	31	37	42	48	152	85
1415 - 1430	152	1.41	11	53	38	39	32	39	44	48	132	87
1430 - 1445 1445 - 1500	141 150	1.31	13 15	61 89	36 37	36 37	30 30	36 37	43 42	47 47	112 126	79 84
1500 - 1515	200	1.86	7	53	36	36	31	36	42	45	171	86
1515 - 1530	177	1.64	22	55	38	38	33	38	43	47	161	91
1530 - 1545	205	1.90	22	85	36	35	31	35	42	45	175	85
1545 - 1600 1600 - 1615	171 197	1.59	21 8	86 78	38 36	39 35	32 29	39 35	43 42	47 47	148 158	87 80
1615 - 1630	188	1.83	21	78 50	38	35	34	39	42	47	172	91
1630 - 1645	181	1.68	21	53	38	39	33	39	44	48	170	94
1645 - 1700	180	1.67	21	53	37	38	32	38	42	46	159	88
1700 - 1715 1715 - 1730	197 226	1.83 2.10	22 15	89 53	37 36	36 37	31 30	36 37	43 42	47 46	173 191	88 85
1715 - 1730	193	1.79	19	85	38	37	32	37	42	46	172	89
1745 - 1800	218	2.03	16	55	37	37	32	37	42	45	191	88
1800 - 1815	231	2.15	7	53	37	37	31	37	42	47	198	86
1815 - 1830	202	1.88	19	84	36	35	30	35	42	47	156	77
1830 - 1845 1845 - 1900	150 158	1.39 1.47	22 4	48 52	37 36	37 37	31 30	37 37	42 42	45 44	130 133	87 84
1900 - 1915	157	1.46	20	58	36	36	30	36	43	45	130	83
1915 - 1930	142	1.32	24	56	39	39	33	39	45	49	128	90
1930 - 1945	134	1.25	4	62	37	37	31	37	42	45	115	86
1945 - 2000 2000 - 2015	151 116	1.40	14 22	56 47	36 36	37 35	31 30	37 35	42 42	46 45	129 96	85 83
2015 - 2030	116	1.08	17	56	36	36	30	36	41	45	94	81
2030 - 2045	119	1.11	21	47	36	37	31	37	42	45	101	85
2045 - 2100	114	1.06	23	52	36	35	30	35	40	45	96	84
2100 - 2115 2115 - 2130	110 116	1.02	22 21	56 52	36 36	35 36	30 30	35 36	42 42	46 46	89 93	81 80
2115 - 2130 2130 - 2145	116 110	1.08	21 21	52 53	36 36	36 36	30 29	36 36	42 43	46 48	93 86	80 78
2145 - 2200	80	0.74	22	50	36	37	28	37	40	45	63	79
2200 - 2215	83	0.77	19	48	36	37	29	37	42	45	66	80
2215 - 2230	67	0.62	24	50	36	36	30	36	42	46	54	81
2230 - 2245 2245 - 2300	66 73	0.61 0.68	25 18	48 62	36 36	35 37	30 30	35 37	42 40	45 45	51 59	77 81
2300 - 2315	65	0.60	22	50	36	36	30	36	42	48	51	78
2315 - 2330	65	0.60	0	52	34	37	29	37	42	47	49	75
2330 - 2345	45	0.42	24	55	37	37	29	37	41	48	36	80
2345 - 2400	51	0.47	23	50	38	38	33	38	43	48	47	92
Session Total	10762		0	108	37	37	31	37	43	47	9335	87

#### Bi-Directional Speed & Class Count | | NB EB Speed 60min

Atlanta, GA



Site 1
Ellsworth Industrial Blvd NW,
south of Old Chattahoochee Ave NW

Date Tuesday, March 29, 2022

Weather Mostly Cloudy 63°F

Lat/Long 33.797841°, -84.427033°

Click here for Map

0000 - 2400 (Weekday 24h Session) NB EB Speed 60min

							Northbo	ound, (Moven	ent 1.1)							60m
Time	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	Tot
0000 - 0100	0	0	2	1	7	9	16	9	0	1	0	0	0	0	0	4
0100 - 0200	0	0	1	4	0	10	18	4	1	2	0	0	0	0	0	4
0200 - 0300	0	0	0	3	2	9	7	6	2	0	1	0	0	0	0	3
0300 - 0400	0	0	0	0	0	7	7	8	6	1	0	0	0	0	0	2
0400 - 0500	1	0	0	0	3	9	18	4	1	0	1	0	0	0	0	3
0500 - 0600	0	0	0	0	7	16	13	11	1	0	0	0	0	0	0	4
0600 - 0700	0	1	0	3	17	43	42	13	3	1	2	0	0	0	1	12
0700 - 0800	0	0	0	4	36	124	87	41	4	0	0	0	0	0	0	25
0800 - 0900	0	0	1	3	48	140	114	24	10	1	1	0	0	0	1	34
0900 - 1000	0	0	0	6	35	91	103	24	8	1	1	0	0	0	0	2
1000 - 1100	0	0	3	21	54	98	76	25	5	2	1	0	0	0	1	2
1100 - 1200	0	0	5	16	74	137	69	25	5	2	0	1	0	0	0	3
1200 - 1300	2	3	8	32	75	150	67	24	7	2	1	0	0	0	1	3
1300 - 1400	0	1	2	24	72	121	78	21	2	0	0	1	0	0	0	3.
1400 - 1500	1	2	6	16	52	108	62	25	11	2	1	0	0	0	1	2
1500 - 1600	0	0	8	24	77	150	93	30	3	1	1	0	0	0	2	3
1600 - 1700	1	1	11	21	69	141	108	26	9	0	0	0	0	1	0	3
1700 - 1800	0	1	6	25	88	154	102	25	5	1	0	0	0	0	3	4
1800 - 1900	6	1	14	19	69	113	91	24	4	0	0	0	0	0	0	3
1900 - 2000	0	0	3	19	43	101	71	22	7	2	1	0	0	0	0	2
2000 - 2100	0	2	5	20	58	70	32	14	0	0	0	0	0	0	0	2
2100 - 2200	0	0	9	21	41	54	33	13	1	0	0	0	0	0	0	1
2200 - 2300	0	1	7	17	36	44	22	3	0	0	0	0	0	0	0	1
2300 - 2400	0	0	1	11	21	42	22	9	2	0	0	0	0	0	0	1

10mph Pace Speed	Number in Pace	15th Percentile	50th Percentile	Average	85th Percentile	95th Percentile
33-42	3408 (64.6%)	32	38	38	43	47

Summary

							Northbo	ound, (Movem	ent 1.1)							Total
TIME	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	Iotai
Tuesday, March 29, 2022	11	13	92	310	984	1941	1351	430	97	19	11	2	0	1	10	5272
Session Total	11	13	92	310	984	1941	1351	430	97	19	11	2	0	1	10	5272

#### AADT

	Total Days	Coverage	ADT	AADT	SD
Total Days	1	0.27	5272.00	5272.00	-
Weekdays	1	0.27	5272.00	5272.00	-
Weekend Days	-	-	-	-	-

#### Speed Statistics by Hour

	Total	% Split	Min	Max	Mean	Median		Perc	entile		>PSL	>PSL %
TIME	Total	70 Split	IVIIII	IVIAX	ivicali	iviculaii	15%	50%	85%	95%	(30mph)	(30mph)
0000 - 0100	45	0.85	22	58	40	41	32	41	46	47	42	93
0100 - 0200	40	0.76	24	57	40	40	35	40	45	54	35	88
0200 - 0300	30	0.57	28	62	41	40	32	40	48	52	26	87
0300 - 0400	29	0.55	35	57	45	45	38	45	51	54	29	100
0400 - 0500	37	0.70	4	63	40	41	36	41	45	49	35	95
0500 - 0600	48	0.91	30	51	40	40	35	40	47	49	47	98
0600 - 0700	126	2.39	17	93	40	39	34	39	45	50	121	96
0700 - 0800	296	5.61	25	53	39	39	35	39	45	47	286	97
0800 - 0900	343	6.51	24	95	39	39	34	39	43	47	332	97
0900 - 1000	269	5.10	25	61	39	40	34	40	44	48	258	96
1000 - 1100	286	5.42	22	83	38	38	32	38	43	48	252	88
1100 - 1200	334	6.34	20	68	37	37	32	37	42	47	307	92
1200 - 1300	372	7.06	9	80	36	36	30	36	42	46	311	84
1300 - 1400	322	6.11	17	65	37	37	32	37	43	45	287	89
1400 - 1500	287	5.44	13	89	38	37	31	37	43	50	248	86
1500 - 1600	389	7.38	21	86	37	37	32	37	43	47	350	90
1600 - 1700	388	7.36	8	78	37	38	32	38	42	47	347	89
1700 - 1800	410	7.78	17	89	37	37	31	37	43	47	358	87
1800 - 1900	341	6.47	4	53	37	38	31	38	42	46	291	85
1900 - 2000	269	5.10	23	62	38	38	32	38	43	48	243	90
2000 - 2100	201	3.81	17	48	35	35	30	35	41	45	162	81
2100 - 2200	172	3.26	21	50	35	35	29	35	42	47	130	76
2200 - 2300	130	2.47	19	48	35	35	29	35	40	43	93	72
2300 - 2400	108	2.05	24	52	37	37	31	37	42	47	92	85

#### Bi-Directional Speed & Class Count | | SB WB Speed 60min

Atlanta, GA



Site 1
Ellsworth Industrial Blvd NW,
south of Old Chattahoochee Ave NW

Date Tuesday, March 29, 2022

Weather Mostly Cloudy 63°F

Lat/Long 33.797841°, -84.427033°



Click here for Map

0000 - 2400 (Weekday 24h Session) SB WB Speed 60min

							Southbo	ound, (Movem	ent 1.2)							60min
Time	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	Total
0000 - 0100	0	0	3	8	9	23	19	9	2	1	1	0	0	1	0	76
0100 - 0200	1	0	0	0	2	1	6	7	2	0	0	0	0	0	0	19
0200 - 0300	0	0	3	0	2	7	4	5	2	0	0	0	0	0	1	24
0300 - 0400	0	0	0	2	2	3	4	6	3	2	0	0	0	0	0	22
0400 - 0500	0	0	0	0	3	11	11	6	0	1	0	0	0	0	0	32
0500 - 0600	0	0	0	3	9	34	21	12	4	3	0	0	0	0	0	86
0600 - 0700	0	1	0	4	24	47	48	16	4	3	1	0	0	0	0	148
0700 - 0800	0	1	0	18	46	74	45	10	1	1	0	0	0	0	1	197
0800 - 0900	1	0	4	5	70	109	58	14	0	0	1	0	0	0	0	262
0900 - 1000	3	0	3	16	62	113	90	14	2	1	0	1	0	0	0	305
1000 - 1100	2	4	7	45	89	103	24	13	2	2	0	0	0	0	1	292
1100 - 1200	1	2	0	22	74	109	81	22	3	2	0	0	0	0	1	317
1200 - 1300	5	0	7	31	81	126	65	13	4	3	0	0	0	0	0	335
1300 - 1400	1	0	8	37	131	133	53	27	4	0	0	0	0	0	0	394
1400 - 1500	1	1	11	32	73	120	69	20	7	0	0	0	1	0	0	335
1500 - 1600	1	1	14	31	101	112	77	17	7	2	1	0	0	0	0	364
1600 - 1700	0	2	4	31	68	135	80	30	7	1	0	0	0	0	0	358
1700 - 1800	0	3	7	29	103	157	93	22	6	3	0	1	0	0	0	424
1800 - 1900	0	1	11	39	106	147	69	22	4	0	0	0	0	0	1	400
1900 - 2000	2	1	8	32	86	91	65	23	5	2	0	0	0	0	0	315
2000 - 2100	0	0	5	23	69	106	45	13	2	1	0	0	0	0	0	264
2100 - 2200	0	0	7	27	63	75	48	18	5	1	0	0	0	0	0	244
2200 - 2300	0	1	3	12	30	57	37	17	1	0	1	0	0	0	0	159
2300 - 2400	4	0	4	15	28	28	23	10	5	1	0	0	0	0	0	118
	•															
Session Total	22	18	109	462	1331	1921	1135	366	82	30	5	2	1	1	5	5490

10mph Pace Speed	Number in Pace	1	15th Percentile	50th Percentile	Average	85th Percentile	95th Percentile
32-41	3410 (62.1%)		30	37	37	42	47

Summary

	Southbound, (Movement 1.2)													Total		
TIME	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	iotai
Tuesday, March 29, 2022	22	18	109	462	1331	1921	1135	366	82	30	5	2	1	1	5	5490
Session Total	22	18	109	462	1331	1921	1135	366	82	30	5	2	1	1	5	5490

#### AADT

	Total Days	Coverage	ADT	AADT	SD
Total Days	1	0.27	5490.00	5490.00	-
Weekdays	1	0.27	5490.00	5490.00	-
Weekend Days	-	-	-	-	-

#### Speed Statistics by Hour

TIME 0000 - 0100 0100 - 0200 0200 - 0300 0300 - 0400	76 19 24	% Split 1.38 0.35	Min 22	Max 76	Mean	Median	15%	50%	85%	95%	(20	(20 1-)
0100 - 0200 0200 - 0300 0300 - 0400	19		22	76				3070	03/0	53/0	(30mph)	(30mph)
0200 - 0300 0300 - 0400	-	0.35		70	39	39	32	39	46	51	65	86
0300 - 0400	24		14	51	42	43	37	43	47	51	18	95
		0.44	22	81	40	39	31	39	46	53	21	88
0400 0500	22	0.40	27	57	43	44	34	44	52	57	20	91
0400 - 0500	32	0.58	30	57	41	41	36	41	46	47	30	94
0500 - 0600	86	1.57	27	57	40	39	35	39	47	52	82	95
0600 - 0700 1	148	2.70	19	62	39	39	34	39	45	49	138	93
0700 - 0800 1	197	3.59	19	86	37	37	31	37	42	46	168	85
0800 - 0900 2	262	4.77	4	62	37	37	33	37	42	45	242	92
0900 - 1000 3	305	5.56	4	66	37	37	32	37	42	45	275	90
1000 - 1100	292	5.32	9	82	34	34	29	34	39	45	217	74
1100 - 1200 3	317	5.77	9	108	37	37	32	37	43	47	280	88
1200 - 1300 3	335	6.10	1	59	36	37	30	37	42	45	271	81
1300 - 1400 3	394	7.18	4	54	36	35	30	35	41	45	327	83
1400 - 1500 3	335	6.10	11	72	36	37	30	37	42	45	274	82
1500 - 1600 3	364	6.63	7	64	36	36	30	36	42	45	305	84
1600 - 1700 3	358	6.52	19	56	37	37	31	37	43	47	312	87
1700 - 1800 4	424	7.72	15	69	37	37	31	37	42	46	369	87
1800 - 1900 4	400	7.29	19	84	36	35	30	35	42	45	326	82
1900 - 2000 3	315	5.74	4	58	36	36	30	36	43	47	259	82
2000 - 2100 2	264	4.81	21	56	36	37	31	37	42	45	225	85
2100 - 2200 2	244	4.44	21	56	36	36	30	36	42	46	201	82
2200 - 2300 1	159	2.90	18	62	37	37	32	37	43	47	137	86
2300 - 2400 1	118	2.15	0	55	35	36	28	36	43	48	91	77

#### Bi-Directional Speed & Class Count | | Bi-Directional Speed 60min

Atlanta, GA



te	1				

Ellsworth Industrial Blvd NW, south of Old Chattahoochee Ave NW

Date Tuesday, March 29, 2022

Weather Mostly Cloudy 63°F

Lat/Long 33.797841°, -84.427033°

Click here for Map

0000 - 2400 (Weekday 24h Session) Bi-Directional Speed 60min

							Southbo	ound, (Movem	ent 1.2)							60mii
Time	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	Tota
0000 - 0100	0	0	5	9	16	32	35	18	2	2	1	0	0	1	0	121
0100 - 0200	1	0	1	4	2	11	24	11	3	2	0	0	0	0	0	59
0200 - 0300	0	0	3	3	4	16	11	11	4	0	1	0	0	0	1	54
0300 - 0400	0	0	0	2	2	10	11	14	9	3	0	0	0	0	0	51
0400 - 0500	1	0	0	0	6	20	29	10	1	1	1	0	0	0	0	69
0500 - 0600	0	0	0	3	16	50	34	23	5	3	0	0	0	0	0	134
0600 - 0700	0	2	0	7	41	90	90	29	7	4	3	0	0	0	1	274
0700 - 0800	0	1	0	22	82	198	132	51	5	1	0	0	0	0	1	493
0800 - 0900	1	0	5	8	118	249	172	38	10	1	2	0	0	0	1	605
0900 - 1000	3	0	3	22	97	204	193	38	10	2	1	1	0	0	0	574
1000 - 1100	2	4	10	66	143	201	100	38	7	4	1	0	0	0	2	578
1100 - 1200	1	2	5	38	148	246	150	47	8	4	0	1	0	0	1	651
1200 - 1300	7	3	15	63	156	276	132	37	11	5	1	0	0	0	1	707
1300 - 1400	1	1	10	61	203	254	131	48	6	0	0	1	0	0	0	716
1400 - 1500	2	3	17	48	125	228	131	45	18	2	1	0	1	0	1	622
1500 - 1600	1	1	22	55	178	262	170	47	10	3	2	0	0	0	2	753
1600 - 1700	1	3	15	52	137	276	188	56	16	1	0	0	0	1	0	746
1700 - 1800	0	4	13	54	191	311	195	47	11	4	0	1	0	0	3	834
1800 - 1900	6	2	25	58	175	260	160	46	8	0	0	0	0	0	1	74:
1900 - 2000	2	1	11	51	129	192	136	45	12	4	1	0	0	0	0	584
2000 - 2100	0	2	10	43	127	176	77	27	2	1	0	0	0	0	0	465
2100 - 2200	0	0	16	48	104	129	81	31	6	1	0	0	0	0	0	41
2200 - 2300	0	2	10	29	66	101	59	20	1	0	1	0	0	0	0	289
2300 - 2400	4	0	5	26	49	70	45	19	7	1	0	0	0	0	0	22
Session Total	33	31	201	772	2315	3862	2486	796	179	49	16	4	1	2	15	107

10mph Pace Speed	Number in Pace	15th Percentile	50th Percentile	Average	85th Percentile	95th Percentile
33-42	6764 (62.9%)	31	37	37	43	47

Summary

		Southbound, (Movement 1.2)												Total		
TIME	5-14 mph	15-19 mph	20-24 mph	25-29 mph	30-34 mph	35-39 mph	40-44 mph	45-49 mph	50-54 mph	55-59 mph	60-64 mph	65-69 mph	70-74 mph	75-79 mph	80-99 mph	Iotai
Tuesday, March 29, 2022	33	31	201	772	2315	3862	2486	796	179	49	16	4	1	2	15	10762
Session Total	33	31	201	772	2315	3862	2486	796	179	49	16	4	1	2	15	10762

#### AADT

	Total Days	Coverage	ADT	AADT	SD
Total Days	1	0.27	10762.00	10762.00	-
Weekdays	1	0.27	10762.00	10762.00	-
Weekend Days	-	-	-	-	-

### Speed Statistics by Hour

	Total	% Split	Min	Max	Mean	Median		Pero	entile		>PSL	>PSL %
TIME	Total	70 Split	IVIIII	IVIAX	ivicali	iviculaii	15%	50%	85%	95%	(30mph)	(30mph)
0000 - 0100	121	1.12	22	76	39	39	32	39	46	49	107	88
0100 - 0200	59	0.55	14	57	41	40	35	40	47	51	53	90
0200 - 0300	54	0.50	22	81	40	40	31	40	47	52	47	87
0300 - 0400	51	0.47	27	57	44	45	37	45	52	56	49	96
0400 - 0500	69	0.64	4	63	40	41	36	41	45	48	65	94
0500 - 0600	134	1.25	27	57	40	39	35	39	47	51	129	96
0600 - 0700	274	2.55	17	93	40	39	34	39	45	50	259	95
0700 - 0800	493	4.58	19	86	38	38	33	38	43	47	454	92
0800 - 0900	605	5.62	4	95	38	38	34	38	42	46	574	95
0900 - 1000	574	5.33	4	66	38	39	33	39	43	47	533	93
1000 - 1100	578	5.37	9	83	36	36	30	36	42	46	469	81
1100 - 1200	651	6.05	9	108	37	37	32	37	43	47	587	90
1200 - 1300	707	6.57	1	80	36	36	30	36	42	46	582	82
1300 - 1400	716	6.65	4	65	36	36	31	36	42	45	614	86
1400 - 1500	622	5.78	11	89	37	37	30	37	43	47	522	84
1500 - 1600	753	7.00	7	86	37	37	31	37	42	47	655	87
1600 - 1700	746	6.93	8	78	37	38	32	38	42	47	659	88
1700 - 1800	834	7.75	15	89	37	37	31	37	42	47	727	87
1800 - 1900	741	6.89	4	84	36	37	30	37	42	45	617	83
1900 - 2000	584	5.43	4	62	37	37	31	37	43	47	502	86
2000 - 2100	465	4.32	17	56	36	36	30	36	42	45	387	83
2100 - 2200	416	3.87	21	56	36	36	29	36	42	47	331	80
2200 - 2300	289	2.69	18	62	36	37	30	37	42	45	230	80
2300 - 2400	226	2.10	0	55	36	37	29	37	42	48	183	81
		-										
Session Total	10762		0	108	37	37	31	37	43	47	9335	87

#### Bi-Directional Speed & Class Count || Graphical Analysis NB EB



Site 1

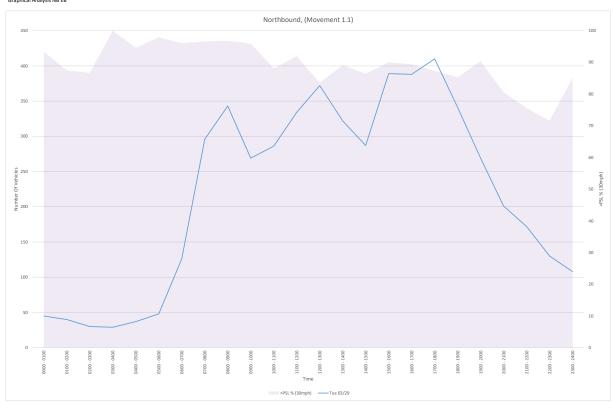
Ellsworth Industrial Blvd NW,
south of Old Chattahoochee Ave NW

Lat/Long 33.797841°, -84.427033°



Click here for Map

0000 - 2400 (Weekday 24h Session) Graphical Analysis NB EB



#### Bi-Directional Speed & Class Count | | Graphical Analysis SB WB



Site 1

Ellsworth Industrial Blvd NW,
south of Old Chattahoochee Ave NW

Lat/Long 33.797841°, -84.427033°



Click here for Map

## 0000 - 2400 (Weekday 24h Session) Graphical Analysis SB WB

