Transportation Analysis

# Project Granite DRI #3298

City of Atlanta, Georgia

May 2021

Prepared for:

West Midtown Acquisition Company, LLC

Prepared by:

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## Kimley »Horn

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#### **Available Upon Request**

Raw Traffic Count Data *Synchro* Capacity Analyses

#### **EXECUTIVE SUMMARY**

This report presents the analysis of the anticipated traffic impacts of the proposed *Project Granite* development located in the City of Atlanta, Georgia. The approximate 36.1-acre site is located south of West Marietta Street, east of Herndon Street, and west of the 950 West Marietta Street development adjacent to Joseph E Lowery Boulevard in the City of Atlanta, Georgia. The site currently consists of approximately 475,000 SF of manufacturing space and 37,500 SF of office space. The existing development is proposed to be demolished and redeveloped with a mix of land uses.

The proposed development will consist of the following land uses and densities contained in **Table 1**. The project is expected to be completed by 2024 (approximately 3 years).

Table 1: Proposed Land Use and Density						
Land Use	Proposed					
Data Center	1,150,000 SF					
Office	640,000 SF					
Retail	35,000 SF					
Restaurant	35,000 SF					
Townhomes	50 units					
Multi-Family Apartments*	350 units					

\*Includes approx. 10% affordable units at 60% AMI

The DRI analysis includes an estimation of the overall vehicle trips projected to be generated by the development, also known as gross trips. Mixed-use, alternative transportation mode, and pass-by reductions to gross trips are also included in the trip generation.

Capacity analyses were performed for the study intersections under the Estimated 2021 conditions, the Projected 2024 No-Build conditions, and the Projected 2024 Build conditions.

- Estimated 2021 conditions represent traffic volumes that were collected in 2018 and grown to 2021 using a 1.5% growth rate or collected in April 2021 and calibrated based on available 2019 GDOT count station data to account for traffic impacts due to COVID.
- Projected 2024 No-Build conditions represent the Estimated 2021 traffic volumes grown for three (3) additional years at 2.0% per year throughout the study network.
- Projected 2024 Build conditions represent the Projected 2024 No-Build conditions plus the addition of the project trips that are anticipated to be generated by the *Project Granite* development.

The intersection of West Marietta Street at Marietta Boulevard (Intersection 1) and the intersection of West Marietta Street at Brady Avenue/8<sup>th</sup> Street (Intersection 4) both operate below their overall LOS standard in either the Existing or No-Build scenario. The southbound approach of Donald Lee Hollowell Parkway (SR 8) at Joseph E Lowery Boulevard (Intersection 5) operates below the approach LOS standard during the PM peak build scenario. Improvements have been identified for the intersections to improve the overall LOS of the intersection. The improvements and LOS summary tables for these intersections are below.

Due to the low level-of-service (LOS) at the following intersections under Estimated 2021 conditions, Projected 2024 No-Build conditions, and/or Projected 2024 Build conditions as indicated, the following intersection improvements are recommended:

- West Marietta Street at Marietta Boulevard (Intersection 1)
  - Restripe the southbound approach for one left-turn lane, one shared through/right-turn lane (No-Build), and two receiving lanes (requires removal of lane channelizing island for Build)
    - With this improvement, also eliminate split phasing for the northbound and southbound approaches
  - Restripe the northbound approach for one left-turn lane, one through lane, one shared through/right-turn lane and one receiving lane (Build)
  - Restripe the eastbound approach for one left-turn lane, one through lane, one right-turn lane, and one receiving lane (No-Build)
  - Restripe the westbound approach for one left-turn lane, one through lane, one right-turn lane, and one receiving lane (No-Build)
- West Marietta Street at Brady Avenue/8<sup>th</sup> Street (Intersection 4)
  - Convert the east leg of the intersection (8<sup>th</sup> Street) to be one-way traffic flow away from the intersection (eastbound only) as contemplated by the Upper Westside CID and Marietta Street Artery Association (No-Build)
    - With this improvement, maintain two-way traffic flow further east of the intersection to allow driveway access for developments north of 8<sup>th</sup> Street
    - While westbound 8<sup>th</sup> Street traffic is relatively small, modifications may impact adjacent properties along 8<sup>th</sup> Street and traffic patterns in the vicinity of the intersection and would need to be coordinated with the City of Atlanta.
- Donald Lee Hollowell Parkway (SR 8) at Joseph E Lowery Boulevard (Intersection 5)
  - Provide one southbound right-turn lane (Build)
    - Due to right-of-way constraints this improvement is not likely to be feasible for construction and is not recommended to be conditioned.
- Site Driveways (Build)
  - Provide one lane entering the site and one lane exiting the site for all site driveways
  - o Provide stop-control for all site driveway approaches

		Marietta Boulevard		Marietta Boulevard			West Marietta Street			West Marietta Street				
			Northbound		Southbound			Eastbound			Westbound		nd	
			L	Т	R	L	L T R			Т	R	L	Т	R
		Overall Std   LOS	D		-	-	-	· F	- (95.7	)		_		-
EXISTING	AN	Approach Std		D			D			D			D	
		Approach LOS		F (223.	5)		F (108.2	<u>'</u> )		D (35.5)			D (47.1)	
		Overall Std   LOS	D					F	- (97.0	)				
	Σ	Approach Std		D			D			D			D	
		Approach LOS		F (136.	7)		F (145.6	i)		D (37.7)			E (60.3)	
		Overall Std   LOS	Е			-		F	(108.9	9)		•		
<u> </u>	M	Approach Std		E			Е		Ĺ	D			D	
		Approach LOS		F (256.	5)		F (127.0	)		D (38.3)			D (47.5)	
8		Overall Std   LOS	Е				•	F	(110.2	<u>2)</u>		•		
N N	M	Approach Std		E			Е		1	D			E	
-		Approach LOS		F (153.	8)		F (170.0	)		D (38.9)			E (66.1)	
		Overall Std   LOS	Е				F			6)				
-	Σ	Approach Std		E		E		D		D				
2	1	Approach LOS		F (248.	9)		F (176.5	5)		D (44.8)			D (50.0)	
IN I	PM	Overall Std   LOS	E					F	(129.9	9)				
		Approach Std		E			Е			D			E	
		Approach LOS		F (151.	6)		F (166.5	i)		D (40.2)			E (130.2	)
	AM	Overall Std   LOS	Е					E	E (61.3	)				
ЪË		Approach Std		E			E			D			D	
۵ ا		Approach LOS		E (77.0	))		E (72.7	)		D (53.1)			D (36.1)	
E Ř	_	Overall Std   LOS	E			-		[	D (35.7	<i>.</i> )				
ΣΞ	PΝ	Approach Std		E			E			D			E	
		Approach LOS		D (39.9)			C (34.0) C (30.2			C (30.2)			D (40.0)	
∢	F	Overall Std   LOS	Е				E (75.9)							
<u>ہ ۵</u>	AN	Approach Std		E			E			D			D	
		Approach LOS		F (108.	4)		E (75.3)	)		E (79.7)			D (39.9)	
D B D B D	-	Overall Std   LOS	E					[	D (39.1	)				
МΡ	РМ	Approach Std		E			E			D			E	
=		Approach LOS		D (39.4	1)		C (34.5	)		D (36.3)			D (46.0)	
B	5	Overall Std   LOS	E	<u> </u>		r		E	= (55.9	)		1		
<u>م</u>	٩N	Approach Std		<u> </u>			E			<u>D</u>			D	
N I		Approach LOS	_	E (57.5	ō)		E (73.9	) _		D (53.2)			C (27.1)	
л Ц Ц	5	Overall Std   LOS	E	<u> </u>				[	ט (39.9 ד	)				
MF	P	Approach Std		E	1		E (ar a						E	
1		Approach LOS		D (41.1	)		D (35.6	)		D (35.2)			D (47.2)	

### Intersection 1 LOS Summary

With the improvements listed above, the intersection of West Marietta Street at Marietta Boulevard (Intersection 1) is projected to operate at or above its overall LOS standard.

		West Marietta Street		Brady Avenue			West Marietta Street			8 <sup>th</sup> Street			
			Northbound		Southbound			Eastbound			Westbound		
			L	T R	L	Т	R	L	Т	R	L	Т	R
(ISTING		Overall Std   LOS	E					C (20.4)					
	AN	Approach Std		E		Е			E			E	
		Approach LOS		A (6.4)	[	D (53.0)			B (11.4)			D (54.0)	
	_	Overall Std   LOS	E		_			E (77.9)			-		
Ê	ΡM	Approach Std		E		Е			E			E	
		Approach LOS		B (10.4)	F	(191.0)			B (11.9)			E (64.0)	
Í		Overall Std   LOS	E					C (21.5)					
9	AM	Approach Std		E		Е			E			E	
In In		Approach LOS		A (6.8)	[	D (53.7)			B (12.7)			D (54.0)	
e B		Overall Std   LOS	Е					F (111.3	)				
ž	PM	Approach Std		E		Е			E			E	
	-	Approach LOS		B (10.6)	F (282.7)		B (12.4)		E (63.9)				
	AM	Overall Std   LOS	E					C (24.4)					
-		Approach Std		E		Е			Е			Е	
		Approach LOS		A (7.1)		Ξ (57.0)			B (14.3)			D (54.0)	
3U	ΡM	Overall Std   LOS	E					F (141.8	)				
-		Approach Std		E		Е			E			Е	
		Approach LOS		B (10.6)	F	(372.9)	)		B (14.0)			E (63.9)	
		Overall Std   LOS	E					B (12.8)					
그 囧	AM	Approach Std		E		Е			E				
۲ ۵		Approach LOS		A (4.2)	(	C (30.2)			A (8.3)				
E Ř	_	Overall Std   LOS	E					C (29.6)					
ΣΞ	PΝ	Approach Std		E		E			E				
		Approach LOS		B (18.2)	[	D (43.4)		C (23.6)					
0	F	Overall Std   LOS	E					B (14.2)					
ب ا	AN	Approach Std		E		E			E				
S F		Approach LOS		A (4.3)	(	C (30.4)			A (9.4)				
BU PR	5	Overall Std   LOS	E	_				D (37.8)	_				
Σ	Р	Approach Std		E		E			E				
		Approach LOS		В (19.2)	[	J (54.5)			D (35.5)				

#### Intersection 4 LOS Summary

With the improvements listed above, the intersection of West Marietta Street at Brady Avenue/8<sup>th</sup> Street (Intersection 4) is projected to operate at or above its overall LOS standard.

		Jose	Joseph E Lowery		Joseph E Lowery Boulevard			Donald Lee Hollowell Parkway (SR 8)			Donald Lee Hollowell Parkway (SR 8)		
			N	orthbound	Southbound			Eastbound			Westbound		d
			L	T R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	Е			D (37.8)							
U	AM	Approach Std		E		Е			Е			Е	
L N		Approach LOS		E (65.5)	Γ	D (44.2)			C (28.8)			B (18.8)	
.SIX		Overall Std   LOS	E					D (38.7)					
Û	ΡM	Approach Std		E		Е			E			E	
		Approach LOS		D (50.9)	E	E (68.2)			B (18.4)			C (32.6)	
		Overall Std   LOS	E					D (41.8)					
٩	AM	Approach Std		E		E			E			E	
0IL		Approach LOS		E (67.8)	D (43.5)		C (34.9)		C (21.4)				
-B	РМ	Overall Std   LOS	E		D (42.9)								
ž		Approach Std		E	E		E			Е			
		Approach LOS		D (52.7)	E	E (69.3)		B (19.7)			D (41.1)		
		Overall Std   LOS	E			D (53.2)							
	٩M	Approach Std		E	E		E		E				
2		Approach LOS		E (76.4)	D (45.4)			D (51.7)		C (26.6)			
BUI		Overall Std   LOS	Е					E (71.5)			•		
-	ΡM	Approach Std		E		Е		E		E			
	_	Approach LOS		E (58.1)	F	(117.0)		C (26.2)			E (78.3)		
		Overall Std   LOS	Е			D (53.9)							
Ö	AM	Approach Std		E		Е		E				Е	
2 S		Approach LOS		E (61.8)	Γ	D (44.4)		E (58.4)		C (30.2)			
PR(		Overall Std   LOS	E					D (46.4)					
Ξ	РМ	Approach Std		E		E			E			E	
	-	Approach LOS		E (62.4)	[	D (54.8)			C (22.1)			D (50.6)	

#### Intersection 5 LOS Summary

With the improvements listed above, the intersection of Donald Lee Hollowell Parkway (SR 8) at Joseph E Lowery Boulevard (Intersection 5) is projected to operate at or above its overall LOS standard. However, due to right-of-way constraints this improvement is not likely to be feasible for construction.

#### **1.0 PROJECT DESCRIPTION**

#### 1.1 Introduction

This report presents the analysis of the anticipated traffic impacts of the proposed *Project Granite* development located in the City of Atlanta, Georgia. The approximate 36.1-acre site is located south of West Marietta Street, east of Herndon Street, and west of the 950 West Marietta Street development adjacent to Joseph E Lowery Boulevard. The project site is currently zoned I-2 (Heavy Industrial), R-4A (Single-Family Residential), and BeltLine Overlay. The site is proposed to be rezoned to MRC-3 (Mixed Residential and Commercial). **Figure 1** provides a location map of the project site. **Figure 2** provides an aerial view of the project site and surrounding area.

The site currently consists of approximately 475,000 SF of manufacturing space and 37,5000 SF of office space. The existing development is proposed to be demolished and redeveloped with a mix of land uses. The proposed development will consist of the following land uses and densities contained in **Table 2**. The project is expected to be completed by 2024 (approximately 3 years).

Table 2: Proposed Land Use and Density						
Land Use	Proposed					
Data Center	1,150,000 SF					
Office	640,000 SF					
Retail	35,000 SF					
Restaurant	35,000 SF					
Townhomes	50 units					
Multi-Family Apartments*	350 units					

\*Includes approx. 10% affordable units at 60% AMI

A reference of the proposed site plan is provided in **Appendix A**. A full-sized site plan consistent with GRTA's Site Plan Guidelines is also being submitted as part of the review package.

The project is considered a Development of Regional Impact (DRI) and is subject to Georgia Regional Transportation Authority (GRTA) and Atlanta Regional Commission (ARC) review due to the project size exceeding 500,000 SF of mixed-use development in a *Maturing Neighborhoods* area per the Atlanta Region's Plan *Unified Growth Policy Map*. The DRI was formally triggered with the filing of the Initial DRI Information (Form 1) on April 14, 2021 by the City of Atlanta. This transportation analysis includes all inputs and methodologies discussed at the DRI Methodology Meeting with GRTA, ARC, and other stakeholders. The inputs and methodologies are outlined in the GRTA Letter of Understanding (LOU).





#### 1.2 Site Access

As currently envisioned, the proposed development will be accessible via five (5) access points:

- 1. **Driveway A** an existing, full-movement driveway located along West Marietta Street approximately 260' east of Herndon Street and operates under side street stop control.
- 2. **Driveway B** an existing, full-movement driveway located along Herndon Street that aligns with Church Street, an existing local street, and operates under side street stop control.
- 3. **Driveway C** an existing, full-movement driveway located along Herndon Street that aligns with Niles Avenue, an existing local street, and operates under side street stop control.
- 4. **Driveway D** an existing, full-movement driveway located along Herndon Street that aligns with Baylor Street, an existing unpaved local street, and operates under side street stop control.
- 5. **Driveway E** an existing, full-movement driveway located along Herndon Street approximately 70 south of Driveway D and operates under side street stop control.

Existing traffic along Church Street, Niles Avenue, and Baylor Street is minimal, so volumes were estimated and included under Projected Build 2024 conditions with the proposed realigned Driveway B, Driveway C, and Driveway D.

#### 1.3 Internal Circulation Analysis

The site consists of three main areas: data center, townhomes, and mixed-use.

The data center area is located in the center to southwest portion of the site and is anticipated to be accessed primarily through Driveway E. However, internal roadways throughout the site allow for access to the data center along the north side as well. The data center area will include controlled access points due to the security measures required by the proposed data centers.

The townhome area is located along the Herndon Street site frontage and is anticipated to be accessed primarily through Driveway C and D. The townhomes are anticipated to be rear loaded using a residential alley behind the townhomes to help reduce the number of curb cuts along Herndon Street.

The mixed-use area is located along the West Marietta Street site frontage and is anticipated to be accessed primarily through Driveway A and B.

The proposed BeltLine corridor located along the eastern frontage of the site is an important feature that will have direct access to the mixed-use area of the site as well as planned access via the new east-west roadway associated with Driveway E.

#### 1.4 Parking

Parking will be provided on-site in individual enclosed parking for the townhomes, parking decks for the mixed-use (retail, restaurant, and apartments), and surface parking lots for the data centers. In addition, some on-street parking will be provided along internal site roadways where space allows.

The current number of total site parking spaces to be provided are listed below in **Table 3**. The site development is currently in progress and the number of parking provided is subject to change.

Table 3: Proposed Parking							
Land Use	Minimum	Maximum	Proposed				
Office	N/A	1,600 spaces	1,500 spaces				
Data Center (Other Non-Residential)	1,917 spaces	N/A	300* spaces				
Retail/Restaurant	117 spaces	N/A	140 spaces				
Residential	400 spaces	N/A	615 spaces				
		Total	2.555 spaces				

\*Variance requested to provide less than minimum parking for the Data Center land use, which is not expected to require significant parking

In addition to standard vehicle parking, the proposed development will include a minimum of 50 bicycle parking spaces, dedicated parking for alternative charging vehicles, and dedicated loading/unloading spaces. Alternative parking will be designed in accordance with City of Atlanta standards and will be coordinated with the City during the permitting process. Other alternative parking options will be considered as design advances.

#### 1.5 Alternative Transportation Facilities

Pedestrian sidewalk facilities are currently provided along the West Marietta Street site frontage. Pedestrian sidewalk facilities are proposed to be provided with the development along the Herndon Street site frontage and all ungated internal roadways. Additionally, the proposed BeltLine corridor is located along the eastern frontage of the site with planned access via the mixed-use area on the sire as well as via the southern east-west roadway, proposed Access E just south of Baylor Street.

The use of alternative transportation modes will be incentivized through dedicated parking for bicycles, vanpool, carpool, and car share. Also, showers and changing facilities will be provided with the office use for employees who walk or bike to work.

Additionally, the project site is located adjacent to a MARTA bus stop that is currently served by routes 1 and 26 seven days a week. The routes provide local service to the Bankhead, Five Points, North Ave, and Ashby MARTA rail stations. The bus stop experienced an average of 10 boardings/10 alightings daily during pre-pandemic conditions in Fall 2019. The bus stop is proposed to be maintained with the development and is projected to increase ridership by approximately 875 boardings/875 alightings daily (assumed 75% of daily alternative mode reduction).



#### 2.0 TRAFFIC ANALYSES, METHODOLOGY AND ASSUMPTIONS

#### 2.1 Study Network Determination

The study area was determined at the methodology meeting with input from GRTA, ARC, and other local agency stakeholders. The study includes the following five (5) intersections described in **Table 4** and is shown visually in **Figure 3**.

Table 4: Intersection Control Summary								
Intersection	Jurisdiction	Control						
1. W Marietta Street at Marietta Boulevard	City of Atlanta	Signal						
2. W Marietta Street at Herndon Street	City of Atlanta	Signal						
3. W Marietta Street at Joseph E Lowery Boulevard	City of Atlanta	Signal						
4. W Marietta Street at Brady Avenue/8 <sup>th</sup> Street	City of Atlanta	Signal						
5. Donald Lee Hollowell Parkway (SR 8) at Joseph E Lowery Boulevard	GDOT	Signal						

#### 2.2 Existing Roadway Facilities

Roadway classification descriptions and estimated Annual Average Daily Traffic (AADT) for roadway segments within the study network are provided in **Table 5** (bolded roadways are adjacent to the site).

Table 5: Roadway Classifications							
Roadway	Lanes	AADT	GDOT Functional Classification				
W Marietta Street	4	14,400	Minor Arterial				
Herndon Street	2	N/A	Local Road				
Church Street	2	N/A	Local Road				
Niles Avenue	2	N/A	Local Road				
Baylor Street	2*	N/A	Local Road <i>(*unpaved)</i>				
Marietta Boulevard	4	16,700	Minor Arterial				
Joseph E Lowery Boulevard	3	9,360	Major Collector				
Donald Lee Hollowell Parkway (SR 8)	4	23,500	Principal Arterial				

#### 2.3 Traffic Data Collection and Calibration

Historical traffic counts collected in 2018 and late 2017 for previous DRIs in the area were used for four (4) of the five (5) study intersections. These traffic counts were grown at 1.5% growth rate to account for background growth from 2018, when the counts were collected, to the current year 2021 as noted in the LOU.

Historical traffic counts were not available for the remaining study intersection of West Marietta Street at Herndon Street (Intersection 2). New traffic counts were collected on Thursday, April 29, 2021 at this location. The newly collected counts were then calibrated using calibration factors to account for the potential impacts of COVID-19 to typical traffic volumes and patterns.

The peak hour adjustment factors were determined by comparing the AM and PM peak volumes at a newly collected average daily traffic (ADT) count to the AM and PM peak ADT volumes previously collected in 2018 at GDOT count station in the same location. The GDOT count station along West Marietta Street east of Herndon Street (Station #121-5061) was used in this comparison. The calibration factors used in this analysis were 2.36 for AM peak hour and 1.40 for PM peak hour. The methodologies used in this analysis for traffic count calibration were approved by GRTA and ARC.

	Table 6: Traffic 0	Count Summ	nary	
	Intersection	Count Date	AM Peak Hour	PM Peak Hour
1.	West Marietta Street at Marietta Boulevard	11/2017	8:00 AM – 9:00 AM	5:00 PM – 6:00 PM
2.	West Marietta Street at Herndon Street	4/2021	8:00 AM – 9:00 AM	5:00 PM – 6:00 PM
3.	West Marietta Street at Joseph E Lowery Boulevard	11/2017	8:00 AM – 9:00 AM	5:00 PM – 6:00 PM
4.	West Marietta Street at Brady Avenue/8th Street	2/2017*	8:00 AM – 9:00 AM	5:00 PM – 6:00 PM
5.	Donald Lee Hollowell Parkway (SR 8) at Joseph E Lowery Boulevard	3/2018	7:45 AM – 8:45 AM	5:00 PM – 6:00 PM

Traffic count peak hours for all the study intersections are shown in **Table 6**.

\*8<sup>th</sup> Street leg is missing in 2017 counts and was supplemented with 2021 calibrated data

The collected peak hour turning movement traffic counts are available upon request.

#### 2.4 Background Growth

Background traffic is defined as expected traffic on the roadway network in future year(s) absent the construction and opening of the proposed *Project Granite* development. Background traffic can include a base growth rate based on historical count data and population growth data as well as trips anticipated from nearby or adjacent other projects.

Based on methodology outlined in the GRTA Letter of Understanding (LOU), a 2.0% per year background traffic growth rate from 2021 to 2024 (3 years) was used for all roadways, with the exception of Herndon Street. No background growth was applied to Herndon Street due to the area accessing Herndon Street being completely built out or to be built out with the proposed development.

The Projected 2024 No-Build conditions represent the Estimated 2021 traffic volumes grown for three (3) years at 2.0% per year throughout the study network.

The Projected 2024 Build conditions represent the project trips generated by the *Project Granite* development (discussed in Section 3.0 and 4.0) added to the Projected 2024 No-Build Conditions.

#### 2.5 Programmed and Planned Projects

Programmed and planned projects near the project site were researched to account for any improvements or modifications within the study network before or by the build-out year of the development. The programmed and planned projects were discussed in the methodology meeting with GRTA, ARC, and other local stakeholders.

No projects were identified to include in the capacity analyses. However, the following projects shown in **Table 7** and **Table 8** are programmed or planned to occur near the development beyond the buildout year of the proposed development or are not anticipated to affect the study network.

	Та	able 7: Prog	rammed Proj	ects			
Project Name	From / To Points:	Sponsor	GDOT PI #	ARC ID # (TIP)	Design FY	ROW / UTL FY	CST FY
Marietta Blvd Resurfacing	Coronet Way/ SR 8	City of Atlanta	<u>0017803</u>	AT-373	Scoping 2021	N/A	N/A
SR 8 Ped Facility	West Lake Ave/ Proctor Creek	City of Atlanta	0010322	AT-240	2011	2021	2023
Northside Dr Signal Upgrades	Tech Way/I-75 Ramps	GDOT	<u>0012823</u>	AT-287	2018	2020	2022
Northside Dr Signal Upgrades	Cameron Alexander Blvd/Wells St	GDOT	<u>0012821</u>	AT-288	2018	2020	2022
JE Boone Blvd Road Diet/ Complete Streets	Mayson Turner Rd/Northside Dr	ARC/ GDOT	N/A	AT-301	N/A	N/A	2030

\*Project information was obtained from GeoPI (GDOT) and the Atlanta Region's Plan (ARC)

	Table 8: I	Planned Proje	ects		
Project Name	From / To Points:	Potential Sponsor	Project ID #	Project Timeline	Planning Document
BeltLine Streetcar Line	Westview Drive/ Bankhead MARTA rail station	MARTA	AR-490F	2050	ARC Fact Sheet
North Ave High Capacity Transit	North Avenue MARTA rail station/Bankhead MARTA rail station along North Avenue.	MARTA	AR-491B	2050	ARC Fact Sheet
Northside Dr High Capacity Transit	I-75 N/Atlanta Metro State College	MARTA	AR-491C	2050	ARC Fact Sheet
W Marietta St Bike Lanes	Marietta Blvd/ throughout area e/o site	City of Atlanta	<u>AT-277A</u> <u>PI#0014993</u>	2030	ARC Fact Sheet
BeltLine Extensions	Various				BeltLine Subarea

In addition to the projects listed in the tables above, the BeltLine trail is proposed to be extended along Marietta Boulevard and is currently under construction.

Available fact sheets for projects can be found in **Appendix D**.

#### 2.6 Level-of-Service Overview

Level-of-service (LOS) is used to describe the operating characteristics of a road segment or intersection in relation to its capacity. LOS is defined as a qualitative measure that describes operational conditions and motorists' perceptions within a traffic stream. The *Highway Capacity Manual* defines six levels-of-service, LOS A through LOS F, with A being the best and F being the worst. LOS analyses were conducted at all intersections within the study network using *Synchro 10*. Existing traffic signal phasing and timing data were retrieved for available intersections.

LOS for signalized intersections is reported for the intersection as a whole. One or more movements at an intersection may experience a low LOS, while the intersection as a whole may operate acceptably.

LOS for unsignalized intersections, with stop control on the minor street only, is reported for the side street approaches and the major street left-turn movements. Low LOS for side street approaches is not uncommon, as vehicles may experience significant delays in turning onto a major roadway.

#### 2.7 Level-of-Service Standards

For the purposes of this traffic analysis, a LOS standard of D was assumed for study intersections located within the Maturing Neighborhoods area. If, however, an intersection or approach currently operates at LOS E or LOS F during an existing peak period, the LOS standard for the overall intersection or approach during that peak period becomes LOS E, consistent with the GRTA LOU. A LOS standard of D was assumed for the intersections listed below:

- 1. W Marietta Street at Marietta Boulevard
- 2. W Marietta Street at Herndon Street

A LOS standard of E was assumed for study intersections located within the *Region Core* area, consistent with the GRTA LOU. A LOS standard of E was assumed for the intersections listed below:

- 1. W Marietta Street at Joseph E Lowery Boulevard
- 2. W Marietta Street at Brady Avenue/8<sup>th</sup> Street
- 3. Joseph E Lowery Boulevard at Donald Lee Hollowell Parkway (SR 8)

#### **3.0** TRIP GENERATION

Gross trips associated with the proposed development were estimated using the *Institute of Transportation Engineers' (ITE) Trip Generation Manual, 10<sup>th</sup> Edition, 2017*, using equations where available. Reductions to gross trips are also considered in the analysis, including mixed-use reductions and alternative transportation mode reductions.

*Mixed-use reductions* occur when a site has a combination of different land uses that interact with one another. For example, people living in a residential development may walk to the restaurants and retail instead of driving off-site or to the site. This reduces the number of vehicle trips that will be made on the roadway, thus reducing traffic congestion.

*Alternative modes reductions* are taken when a site can be accessed by modes other than vehicles (walking, bicycling, transit, etc.). Alternative mode reductions were taken at 17% per the LOU.

**Pass-by reductions** are taken for a site when traffic normally traveling along a roadway may choose to visit a retail or restaurant establishment that is along the vehicle's path. These trips were already on the road and would therefore only be new trips on the driveways. The retail and restaurant establishments proposed for the project are expected to generate pass-by trips.

Table 9: Trip Generation													
L and Lloo	Donaity	D	aily Traffi	C	AM Pea	k Hour	PM Pea	k Hour					
Lanu Use	Density	Total	Enter	Exit	Enter	Exit	Enter	Exit					
160 – Data Center	1,150,000 sf	1,138	569	569	70	57	31	73					
220 – Multi-Family Housing (Low-Rise)	45 units	300	150	150	5	17	18	11					
221 – (Multi-Family Housing (Mid-Rise)	350 units	1,906	953	953	30	87	90	57					
710 – General Office Building	640,000 sf	6,422	3,211	3,211	540	88	106	558					
820 – Shopping Center	35,000 sf	1,322	661	661	20	13	64	69					
932 – High-Turnover (Sit-Down) Restaurant	35,000 sf	3,926	1,963	1,963	191	157	212	130					
Gross Project Tri	ps	15,014	7,057	7,057	856	419	521	898					
Existing Use Trips (To Be	Removed)*	-2,072	-1,036	-1,036	-280	-77	-106	-257					
Mixed-U	se Reductions	-1,276	-638	-638	-97	-97	-96	-96					
Alternative Mo	de Reductions	-1,983	-992	-992	-88	-35	-50	-98					
Pass-	By Reductions	-1,323	-662	-662	-	-	-40	-40					
Net New Trips		8,360	4,180	4,180	391	210	229	407					
Driveway Volum	es	11,755	5,878	5,878	671	287	375	704					

**Table 9** summarizes the gross trip generation, reductions, net trip generation, and driveway volumes for the proposed *Project Granite* development.

\*Existing use trips (to be removed) were based on 475,000 sf of Manufacturing (ITE 140) and 37,500 SF of General Office Building (ITE 710) that were in operation at the time of the counts and have been applied to the adjacent roadway network.

A more detailed trip generation analysis summary table is provided in Appendix B.

#### 4.0 TRIP DISTRIBUTION AND ASSIGNMENT

The distribution of new project trips was based on the project land uses, a review of land use densities and road facilities in the area, engineering judgement, and methodology discussions with GRTA, ARC, and other local stakeholders.

The anticipated distribution and assignment of the trips throughout the study roadway network is shown for residential uses in **Figure 4** and at the site driveways in **Figure 5**. The distribution and assignment for non-residential uses is shown for the study network in **Figure 6** and at the site driveways in **Figure 7**. These trip assignment percentages were applied to the net project trips expected to be generated by the development, and the volumes were assigned to the roadway network. The peak hour project trips are shown by turning movement throughout the study network in **Figure 9** and at the site driveways in **Figure 10**.

Detailed intersection volume worksheets are provided in Appendix C.

#### 5.0 TRAFFIC ANALYSIS

Capacity analyses were performed using *Synchro 10* for the AM and PM peak hours under Estimated 2021 conditions, Projected 2024 No-Build conditions, and Projected 2024 Build conditions. The capacity analyses were performed using methodologies from the *Highway Capacity Manual (HCM), 6<sup>th</sup> Edition* unless otherwise noted. Intersections 1, 3, and 4 were analyzed using *HCM 2000* methodologies under Estimated 2021 conditions, Projected 2024 No-Build conditions, and Projected 2024 Build conditions due to shared laneage and signal phasing constraints. Due to laneage and phasing improvements, Intersection 1 was able to be analyzed using *HCM 6<sup>th</sup> Edition* methodologies under the Projected 2024 No-Build improved and Projected 2024 Build improved scenarios.

These analyses included existing roadway laneage and signal timing data for each of the scenarios. The existing intersection laneage used for all base scenarios is shown in **Figure 11**. The traffic volumes used for each scenario are shown visually in **Figure 12** for Estimated 2021 conditions, **Figure 13** for Projected 2024 No-Build conditions, and **Figure 14** for Projected 2024 Build conditions.

The results of the capacity analyses are presented for each intersection and include projected LOS, delay, and queue lengths.























			Marie	etta Bou	levard	Mari	etta Boule	evard	Wes	t Marietta S	Street	West	Marietta	Street
			N	lorthbou	nd	9	Southbour	nd		Eastbound	ł	V	Vestboun	d
-			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	D		-			F	(88.4)	)	-	-	-	
		Approach Std		D			D			D			D	
	Σ	Approach LOS		F (223.5	5)		F (85.9)			D (35.5)			D (47.1)	
	A	Storage	-	-	130	-	-	-	-	-	200	-	-	-
Q		50th Queue	-	321	0	469	471	-	-	307	-	-	50	-
Ĩ		95th Queue	-	440	0	698	723	-	-	372	-	-	108	-
IIS.		Overall Std   LOS	D					F	(112.7	)				
Ш		Approach Std		D			D			D			D	
	Σ	Approach LOS		F (136.7	')		F (187.4)	)		D (37.7)			E (60.3)	
	Р	Storage	-	-	130	-	-	-	-	-	200	-	-	-
		50th Queue	-	198	0	254	1315	-	-	140	-	-	468	-
		95th Queue	-	307	0	351	1594	-	-	187	-	-	566	-
		Overall Std   LOS	Е					F	(100.4	.)				
		Approach Std		Е			Е			D			D	
LD	Σ	Approach LOS		F (256.5	5)		F (101.2)	)		D (38.3)			D (47.5)	
	A	Storage	-	-	130	-	-	-	-	-	200	-	-	-
		50th Queue	-	353	0	510	545	-	-	334	-	-	53	-
In		95th Queue	-	474	0	765	788	-	-	403	-	-	113	-
-B		Overall Std   LOS	ш					F	(127.0	) )				
Ň		Approach Std		E			E			D			Е	
	Σ	Approach LOS		F (153.8	8)		F (214.7)			D (38.9)			E (66.1)	
	P	Storage	-	-	130	-	-	-	-	-	200	-	-	-
		50th Queue	-	222	0	272	1446	-	-	155	-	-	522	-
		95th Queue	-	334	0	374	1727	-	-	204	-	-	628	-
		Overall Std   LOS	E		-			F	(102.3		-	-	-	
		Approach Std		Е			E			D			D	
	Σ	Approach LOS		F (254.0	))		F (108.8)			D (40.7)			D (49.3)	
	∢	Storage	-	-	130	-	-	-	-	-	200	-	-	-
Δ		50th Queue	-	353	0	557	570	-	-	347	-	-	78	-
JIL		95th Queue	-	474	0	798	814	-	-	419	-	-	146	-
BL		Overall Std   LOS	E	_		1		F	(128.2	2)		1		
		Approach Std					E						E	
	M	Approach LOS		F (152.2	120		F (212.6)			D (40.1)	200		E (73.6)	
	_	Storage	-	-	0	- 302	-	-	-	- 171	200	-	-	-
			-	224	0	30Z	1400	-	-	222	-	-	715	-
		aoin Queue	-	334	U	410	1/31	-	-	222	-	-	715	-

#### 5.1 West Marietta Street at Marietta Boulevard (Intersection 1)

The intersection of West Marietta Street at Marietta Boulevard (Intersection 1) currently operates and is projected to operate at LOS F for the overall intersection. The northbound and southbound approaches are the primary delay contributors at the intersection, likely due to the split phase configuration for those approaches.

By restriping the northbound and southbound approaches to include exclusive left-turn lanes, the split phasing can be eliminated. The recommended lane striping for the northbound approach under the No-Build Improved scenario is one left-turn lane, one through lane, and one right-turn lane. The recommended lane striping for the southbound approach is one left-turn lane, one shared through/right-turn lane, and one receiving lane. Additionally, the eastbound and westbound approaches can be restriped to accommodate one left-turn lane, one through lane, one right-turn lane, and one receiving

			Ma	rietta Boule	evard	Ma	rietta Boule	vard	Wes	t Marietta	Street	Wes	st Marietta S	Street
				Northboun	d		Southboun	d		Eastbound	1		Westbound	k
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	Е					E	E (61.3	)				
•		Approach Std		E			Е			D			D	
Ē	Σ	Approach LOS		E (77.0)			E (72.7)			D (53.1)			D (36.1)	
ò	◄	Storage	-	-	130	-	-	-	-	-	200	-	-	-
Å.		50th Queue	47	374	0	562	143	-	187	446	14	14	88	134
Σ		95th Queue	95	588	0	799	211	-	278	652	52	41	150	222
Q		Overall Std   LOS	Е					[	D (35.7	)				
		Approach Std		Е			E			D			E	
Ъ.	Σ	Approach LOS		D (39.9)			C (34.0)			C (30.2)			D (40.0)	
9	₽	Storage	-	-	130	-	-	-	-	-	200	-	-	-
~		50th Queue	61	128	0	127	522	-	66	73	14	28	284	50
		95th Queue	158	197	0	190	802	-	149	122	66	61	443	146
		Overall Std   LOS	E					E	(75.9	)				
		Approach Std		E			E			D			D	
PROVED A	Σ	Approach LOS		F (108.4)			E (75.3)			E (79.7)			D (39.9)	
	◄	Storage	-	-	130	-	-	-	-	-	200	-	-	-
		50th Queue	49	428	0	640	137	-	192	603	14	22	122	170
		95th Queue	99	638	0	884	202	-	290	838	53	69	245	270
ž		Overall Std   LOS	E			1		Γ	0 (39.1	)				
Q		Approach Std		E			E			D			E	
	Σ	Approach LOS		D (39.4)			C (34.5)			D (36.3)			D (46.0)	
Bl	Δ.	Storage	-	-	130	-	-	-	-	-	200	-	-	-
		50th Queue	61	128	0	146	522	-	66	93	14	41	365	111
		95th Queue	158	197	0	217	802	-	181	148	66	82	586	244
		Overall Std   LOS	E			1		E	E (55.9	)				
		Approach Std		E			E			D			D	
В	Σ	Approach LOS		E (57.5)			E (73.9)			D (53.2)			C (27.1)	
Ш	∢	Storage	-	-	-	-	-	-	-	-	-	-	-	-
8		50th Queue	50	204	-	629	158	-	175	515	13	20	102	142
Ř		95th Queue	101	267	-	873	233	-	260	750	48	68	166	222
Ξ		Overall Std   LOS	Е					Γ	) (39.9	)				
Ō		Approach Std		E			E			D			E	
	Σ	Approach LOS		D (41.1)			D (35.6)			D (35.2)			D (47.2)	
B	٩	Storage	-	-	-	-	-	-	-	-	-	-	-	-
		50th Queue	68	73	-	161	576	-	71	101	15	45	398	123
		95th Queue	170	107	-	235	863	-	186	158	68	88	621	259

lane. With these improvements, the intersection is projected to operate acceptably. The analysis results for the improved conditions at intersection 1 are shown in the table below.

The Build Improved A scenario assumes all improvements listed above for the No-Build Improved scenario are in place. With these improvements, the intersection of West Marietta Street at Marietta Boulevard (Intersection 1) is projected to operate acceptably overall. However, the northbound approach operates below the approach LOS standard during the AM peak hour under the Build Improved A scenario. In order for the northbound and eastbound approaches to operate acceptably, the northbound approach should be restriped as one exclusive left-turn lane, one through lane, and one shared through/right-turn lane. This improvement also requires the removal of the lane channelizing island in the north leg of the intersection to provide two northbound receiving lanes. The results of the additional improvements are shown in the table above as scenario Build Improved B.

The improved intersection laneage is shown in Figure 16.

			He	erndon Str	eet				West	Marietta S	Street	West	Marietta S	Street
				Northboun	nd	S	outhbou	nd	E	Eastbound	t	V	Vestboun	d
-			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	D						A (5.7)					
		Approach Std		D						D			D	
	Σ	Approach LOS		C (24.1)						A (5.1)			A (5.1)	
	A	Storage	-		-					-	-	-	-	
Q		50th Queue	4		-					78	-	-	75	
E		95th Queue	32		-					112	-	-	111	
IS.		Overall Std   LOS	D						A (4.3)					
Ш		Approach Std		D						D			D	
	Σ	Approach LOS		C (24.3)						A (3.6)			A (3.8)	
	P	Storage	-		-					-	-	-	-	
		50th Queue	3		-					0	-	-	0	
		95th Queue	26		-					74	-	-	86	
		Overall Std   LOS	D						A (5.9)					
		Approach Std		D						D			D	
	Σ	Approach LOS		C (24.1)						A (5.3)			A (5.4)	
	A	Storage	-		-					-	-	-	-	
Ą		50th Queue	4		-					85	-	-	83	
IIN		95th Queue	32		-					122	-	-	122	
-B		Overall Std   LOS	D						A (4.3)					
No N		Approach Std		D						D			D	
	⋝	Approach LOS		C (24.3)						A (3.7)			A (3.9)	
	Ы	Storage	-		-					-	-	-	-	
		50th Queue	3		-					0	-	-	0	
		95th Queue	26		-					80	-	-	92	
		Overall Std   LOS	D						C (25.0)					
		Approach Std		D						D			D	
	Σ	Approach LOS		C (28.6)						B (10.5)			D (41.1)	
	A	Storage	-		-					-	-	-	-	
		50th Queue	63		-					131	-	-	217	
		95th Queue	130		-					263	-	-	422	
SU SU		Overall Std   LOS	D						C (21.9)					
		Approach Std		D						D			D	
	≥	Approach LOS		D (48.3)						B (11.9)			B (17.7)	
	4	Storage	-		-					-	-	-	-	
		50th Queue	204		-					113	-	-	162	
		95th Queue	392		-					157	-	-	245	

#### 5.2 West Marietta Street at Herndon Street (Intersection 2)

The intersection of West Marietta Street at Herndon Street (Intersection 2) currently operates and is projected to operate at an acceptable overall LOS under future scenarios. Additionally, the intersection approaches currently operate and are projected to operate at an acceptable LOS under future scenarios.

During meetings with the Upper Westside CID and Howell Station neighborhood, a request was made to remove the eastbound channelized right-turn lane at this intersection and also to consider relocating the Herndon Street signal to the new Site Driveway A east of Herndon Street. Traffic operations will not have significant impact from the removal of the channelized right-turn lane, which was likely installed to serve large trucks entering Herndon Street from West Marietta Street. The traffic signal relocation is further discussed in *Section 5.6 West Marietta Street at Site Driveway A (Intersection 6)*.

			Jose	eph E Lov Soulevard	very				West	Marietta S	Street	West	Marietta S	Street
			N	orthboun	d	ç	Southbour	d	F	asthound	4	v	Vestbound	h
			L	T	R	L	T	R	L	T	R	L	T	R
		Overall Std   LOS	E						C (21.6)					
		Approach Std		E						E			Е	
	Σ	Approach LOS		C (27.8)						C (23.6)			A (7.3)	
	A	Storage	-		-					-	-	-	-	
U		50th Queue	70		-					260	-	-	39	
N I		95th Queue	119		-					352	-	-	60	
IS.		Overall Std   LOS	E						B (19.4)					
Ш		Approach Std		Е						E			Е	
	Σ	Approach LOS		C (26.3)						B (14.6)			B (19.4)	
	┛	Storage	-		-					-	-	-	-	
		50th Queue	37		-					43	-	-	187	
		95th Queue	74		-					74	-	-	279	
		Overall Std   LOS	E						C (23.3)					
		Approach Std		Е						Е			Е	
	Σ	Approach LOS		C (28.3)						C (26.4)			A (7.5)	
	◄	Storage	-		-					-	-	-	-	
P		50th Queue	78		-					286	-	-	42	
IJ.		95th Queue	128		-					436	-	-	66	
н Н Н Н Н		Overall Std   LOS	E						C (24.1)					
ž		Approach Std		E						E			E	
	Σ	Approach LOS		C (26.4)			r			B (14.8)			C (26.9)	
	₽	Storage	-		-					-	-	-	-	
		50th Queue	40		-					48	-	-	207	
		95th Queue	77		-					82	-	-	349	
		Overall Std   LOS	E						C (30.5)					
		Approach Std		E						E			E	
	Σ	Approach LOS		C (33.5)			-	1		D (37.5)			A (8.9)	
	◄	Storage	-		-					-	-	-	-	
~		50th Queue	139		-					350	-	-	63	
Ľ		95th Queue	201		-					510	-	-	89	
BU		Overall Std   LOS	E						D (42.4)			I		
		Approach Std		E						E			E	
	Σ	Approach LOS		C (27.2)						B (16.3)			E (60.9)	
	₽	Storage	-		-					-	-	-	-	
		50th Queue	58		-					91	-	-	270	
		95th Queue	102		-					140	-	-	454	

#### 5.3 West Marietta Street at Joseph E Lowery Boulevard (Intersection 3)

The intersection of West Marietta Street at Joseph E Lowery Boulevard (Intersection 3) currently operates and is projected to operate at an acceptable overall LOS under future scenarios. Additionally, the intersection approaches currently operate and are projected to operate at an acceptable LOS under future scenarios.

			West	Marietta S	Street	Bi	rady Aven	ue	West	Marietta 3	Street		8 <sup>th</sup> Street	
			Nort	hbound (	WB)	9	Southboun	d	Eas	stbound (I	EB)	Wes	stbound (S	SW)
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	E						C (20.4)					
		Approach Std		Е			Е			E			Е	
	Σ	Approach LOS		A (6.4)			D (53.0)			B (11.4)			D (54.0)	
	A	Storage	-	-	-	-	-	-	-	-	-	-	-	-
Q		50th Queue	-	25	-	34	0	-	-	195	-	0	0	-
Ĩ		95th Queue	-	52	-	122	0	-	-	336	-	7	0	-
.SI		Overall Std   LOS	Е						E (77.9)					
Ш		Approach Std		E			E			Е			Е	
	⋝	Approach LOS		B (10.4)			F (191.0)			B (11.9)			E (64.0)	
	Ы	Storage	-	-	-	-	-	-	-	-	-	-	-	-
		50th Queue	-	113	-	348	0	-	-	107	-	0	0	-
		95th Queue	-	146	-	589	0	-	-	145	-	33	0	-
		Overall Std   LOS	E						C (21.5)					
		Approach Std		Е			Е			Е			Е	
	Σ	Approach LOS		A (6.8)			D (53.7)			B (12.7)			D (54.0)	
	AI	Storage	-	-	-	-	-	-	-	-	-	-	-	-
Ą		50th Queue	-	28	-	47	0	-	-	230	-	0	0	-
Б		95th Queue	-	57	-	138	0	-	-	390	-	10	0	-
e B		Overall Std   LOS	E						F (111.3	)		-		
ž		Approach Std		Е			Е			E			Е	
	Σ	Approach LOS		B (10.6)			F (282.7)			B (12.4)			E (63.9)	
	Б	Storage	-	-	-	-	-	-	-	-	-	-	-	-
		50th Queue	0	122	-	459	0	-	-	116	-	0	0	-
		95th Queue	0	157	-	706	0	-	-	159	-	37	0	-
		Overall Std   LOS	Е						C (24.4)					
		Approach Std		Е			E			Е			Е	
	Σ	Approach LOS		A (7.1)			E (57.0)			B (14.3)			D (54.0)	
	A	Storage	-	-	-	-	-	-	-	-	-	-	-	-
		50th Queue	-	33	-	53	-	-	-	263	-	0	-	-
		95th Queue	-	63	-	164	-	-	-	435	-	10	-	-
SUI 8		Overall Std   LOS	E						F (141.8	)				
-		Approach Std		Е			Е			Е			Е	
	Σ	Approach LOS		B (10.6)			F (372.9)			B (14.0)			E (63.9)	
	Р	Storage	-	-	-	-	-	-	-	-	-	-	-	-
		50th Queue	-	125	-	564	-	-	-	156	-	0	-	-
		95th Queue	-	160	-	813	-	-	-	211	-	37	-	-

#### West Marietta Street at Brady Avenue/8th Street (Intersection 4) 5.4

The intersection of West Marietta Street at Brady Avenue/8<sup>th</sup> Street (Intersection 4) is projected to operate at LOS F under both no-build and build future conditions. The southbound and westbound approaches are the primary delay contributors at the intersection. The intersection geometry is complex and has been modeled with the eastbound/westbound mainline as West Marietta Street and Marietta Street, which could be considered the northbound/southbound configuration.

The Upper Westside CID and Marietta Street Artery Association recently proposed the removal of westbound 8<sup>th</sup> Street as an opportunity to modify the intersection to better serve multimodal travelers. While the 8<sup>th</sup> Street westbound traffic is relatively small, the proposed change would need to be coordinated with the City of Atlanta and may impact adjacent property owners along 8th Street as well as traffic patterns in the vicinity of the site. By converting the east leg of the intersection (8th Street) to one-way traffic flow away from the intersection, the westbound turning movements and signal phasing 015170056 29 May 2021

are eliminated. The green time previously used by for the westbound approach can be redistributed to the other approaches which reduces delay. With these improvements, the intersection is projected to operate acceptably. The analysis results for the improved conditions at intersection 1 are shown in the table below.

			Wes	st Marietta	Street	E	Brady Aven	ue	West	Marietta S	treet		8 <sup>th</sup> Street	
			No	orthbound (	WB)		Southboun	d	Ea	stbound (E	B)	Westb	ound ( <mark>Rem</mark>	oved)
-			L	Т	R	L	Т	R	L	Т	R	L	Т	R
	-	Overall Std   LOS	Е						B (12.8	8)				
		Approach Std		E			Е			Е				
	Σ	Approach LOS		A (4.2)			C (30.2)			A (8.3)				
8	A	Storage	1	-	-	-	-	-	-	-	-			
R(		50th Queue	-	13	-	17	-	-	-	106	-			
M		95th Queue	1	36	-	85	-	-	-	246	-			
Q		Overall Std   LOS	Е						C (29.6	S)				
		Approach Std		E			Е			Е				
Ā	Σ	Approach LOS		B (18.2)			D (43.4)			C (23.6)				
ŌN N	Ъ	Storage	-	-	-	-	-	-	-	-	-			
		50th Queue	•	113	-	263	-	-	-	108	-			
		95th Queue	-	160	-	508	-	-	-	165	-			
		Overall Std   LOS	Е						B (14.2	2)				
		Approach Std		E			E			E				
	Σ	Approach LOS		A (4.3)			C (30.4)			A (9.4)				
ίΞ	A	Storage	-	-	-	-	-	-	-	-	-			
õ		50th Queue	-	15	-	17	-	-	-	118	-			
Р		95th Queue	-	41	-	94	-	-	-	285	-			
Σ		Overall Std   LOS	E			_			D (37.8	3)				
Г		Approach Std		E			E			E				
IJ.	Σ	Approach LOS		B (19.2)			D (54.5)			D (35.5)				
Ξ	Ъ	Storage	-	-	-	-	-	-	-	-	-			
		50th Queue	-	116	-	309	-	-	-	145	-			
		95th Queue	-	163	-	565	-	-	-	248	-			

With the improvements listed above, the intersection of West Marietta Street at Brady Avenue/8<sup>th</sup> Street (Intersection 4) is projected to operate at or above its overall LOS standard. The improved intersection laneage is shown in **Figure 16.** 

# 5.5 Donald Lee Hollowell Parkway (SR 8) at Joseph E Lowery Boulevard (Intersection 5)

			Jose	eph E Lo	wery	Jos	eph E Lov	very	Donal	d Lee Ho	llowell	Donal	d Lee Hol	lowell
				30ulevai	nd D		Boulevard	 	Pa	rkway (Sh	(8)	Par	Kway (SR	(8)
					nu P			u P			J Q	v v		J Q
		Overall Std LLOS	E	1	N		I	N	D (37.8)			L	I	N
	·	Approach Std		F			F		D (07.0)	/			F	
	5	Approach LOS		E (65.5	)		D (44 2)			C (28.8)			B (18 8)	
	A	Storage	215	-	-	-	-	-	-	-	50	-	-	-
U		50th Queue	104	392	-	60	129	-	-	542	54	-	90	-
NI		95th Queue	158	604	-	100	199	-	-	664	92	-	126	-
ISI		Overall Std   LOS	E						D (38.7)	)				
ЕX	İ	Approach Std		E			Е			E			Е	
	Σ	Approach LOS		D (50.9	)		E (68.2)			B (18.4)			C (32.6)	
	₫	Storage	215	-	-	-	-	-	-	-	50	-	-	-
		50th Queue	109	229	-	121	342	-	-	168	72	-	549	-
		95th Queue	198	336	-	183	541	-	-	213	116	-	668	-
		Overall Std   LOS	E			-			D (41.8)					
		Approach Std		Е			E			E			Е	
	Σ	Approach LOS		E (67.8	)		D (43.5)			C (34.9)			C (21.4)	
	◄	Storage	215	-	-	-	-	-	-	-	50	-	-	-
Р		50th Queue	110	424	-	63	137	-	-	622	58	-	98	-
3UI		95th Queue	169	675	-	106	210	-	-	797	97	-	137	-
ď		Overall Std   LOS	E			r			D (42.9)			1		
ž		Approach Std		E	<u>,</u>		E			E			E	
	Σ	Approach LOS	0.15	D (52.7	)		E (69.3)			B (19.7)			D (41.1)	
		Storage	215	-	-	-	-	-	-	-	50	-	-	-
		50th Queue	118	247	-	129	376	-	-	183	78	-	634	-
		95th Queue	231	358	-	194	595	-	- D (50.0)	231	123	-	818	-
		Overall Std   LOS	E						D (53.2)	)				
	_	Approach Sid			\									
	AR	Approach LOS Storago	215	E (70.4	)		D (45.4)			D (51.7)	50		C (20.0)	
		50th Queue	110	-	-	- 75	- 158	-	-	- 776	58	-	- 102	-
Ą		95th Oueue	169	720	-	140	238	-	-	916	97	_	145	-
IIN		Overall Std LLOS	F	120		110	200		E (71.5)	010	01		110	
B		Approach Std	-	E			F			E			E	
	5	Approach LOS		E (58.1	)		F (117.0)			C (26.2)			E (78.3)	
	đ	Storage	215	-	-	-	-	-	-	-	50	-	-	-
		50th Queue	121	261	-	168	546	-	-	204	75	-	643	-
		95th Queue	253	371	-	246	775	-	-	263	119	-	831	-

The intersection of Donald Lee Hollowell Parkway (SR 8) at Joseph E Lowery Boulevard (Intersection 5) currently operates and is projected to operate at or above the overall LOS standard under future scenarios. The southbound and westbound approaches at the intersection are projected to operate at LOS F during the build scenario PM peak hour.

The proposed development is not anticipated to add trips to either of the movements with queues exceeding available storage length highlighted in the table above.

Construction of a southbound right-turn lane would allow the intersection to operate acceptably. However, due to right-of-way constraints this improvement is not likely to be feasible for construction. Several alternative improvements were explored to identify if a more feasible alternative could mitigate the low LOS, however they each had significant right-of-way impacts with less benefit to operations. Because the overall intersection operates acceptably during the PM peak hour, the southbound approach operates acceptably during the AM peak hour without the right-turn lane, and due to the significant right-of-way impacts anticipated with its construction, we recommend this improvement not be conditioned.

The analysis results for the improved conditions at Intersection 5 are shown in the table below.

			Jo	seph E Lov Boulevard	very I	Jo	seph E Lov Boulevard	very I	Don P	ald Lee Hol arkway (SR	llowell 8 8)	Don P	ald Lee Hol arkway (SR	lowell 8)
				Northboun	d		Southboun	d		Eastbound	ł		Westbound	ł
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	Е					[	D (53.9	)				
		Approach Std		Е			Е			Е			E	
VED	Σ	Approach LOS		E (61.8)			D (44.4)			E (58.4)			C (30.2)	
	A	Storage	215	-	-	-	-	-	-	-	50	-	-	-
Ő		50th Queue	110	447	-	75	134	0	-	774	58	-	102	-
ЪF		95th Queue	169	656	-	186	205	9	-	914	97	-	144	-
Σ		Overall Std   LOS	Е					[	D (46.4	)				
Р		Approach Std		Е			Е			E			E	
I.	Σ	Approach LOS		E (62.4)			D (54.8)			C (22.1)			D (50.6)	
Bl	Ъ	Storage	215	-	-	-	-	-	-	-	50	-	-	-
		50th Queue	119	274	-	169	244	97	-	202	75	-	635	-
		95th Queue	172	375	-	282	348	183	-	278	127	-	865	-

With the improvements listed above, the intersection of Donald Lee Hollowell Parkway (SR 8) at Joseph E Lowery Boulevard (Intersection 5) is projected to operate at or above its overall and approach LOS standards. However, as noted above, right-of-way constraints may make the improvement infeasible for construction.

The improved intersection laneage is shown in Figure 16.

				Driveway A	4				Wes	t Marietta S	Street	Wes	t Marietta S	Street
				Northboun	d		Southbound	d		Eastbound	k		Westbound	k
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	D						(94.3)					
		Approach Std		D						D			D	
	Σ	Approach LOS		F (>120)						A (0.0)			A (6.6)	
	◄	Storage	•		-					-	-	-	-	
~		50th Queue	-		-					-	-	-	-	
		95th Queue	350		-					-	-	43	-	
3U		Overall Std   LOS	D						(13.1)					
-		Approach Std		D						D			D	
	Σ	Approach LOS F (99.2)								A (0.0)			A (2.5)	
	٩	Storage	-		-					-	-	-	-	
		50th Queue	-		-					-	-	-	-	
	95th Queue 258				-					-	-	25	-	

#### 5.6 West Marietta Street at Site Driveway A (Intersection 6)

The intersection of West Marietta Street at Site Driveway A (Intersection 6) is projected to operate acceptably, but at a poor LOS for the northbound approach under the build conditions. The intersection is proposed to operate under stop-control with stop control for the one-lane northbound approach only. Long delays are common at unsignalized intersections and interconnectivity is provided through the site to give drivers an option to utilize the signal at Herndon Avenue. Therefore, long delays are considered acceptable at this location. A configuration with two lanes exiting the site was contemplated to help reduce delay at the intersection by separating left-turns from right-turning vehicles. Delay improves, but continues to operate at LOS F. However, it is unlikely that the City of Atlanta will allow a two-lane approach for a side street without the incorporation of a traffic signal. The recommended lane configuration for Driveway A is one lane entering the site and one lane exiting the site.

As noted in Section 5.1 West Marietta Street at Herndon Street (Intersection 1), discussions with the Upper Westside CID and Howell Station neighborhood included a request to consider the relocation of the existing traffic signal at Herndon Street to the new Site Driveway A. This request was made after the Methodology Meeting, and therefore was not included in the scope of the traffic study as outlined in the LOU. The relocation of the signal is anticipated to be discussed at a future date with the applicant, the City of Atlanta and relevant stakeholders.

			H	lerndon Str	eet	F	lerndon Stre	eet	(	Church Stre	et		Driveway E	3
				Northboun	d		Southboun	d		Eastbound	k		Westbound	ł
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
	-	Overall Std   LOS	D						(5.7)					
		Approach Std					D						D	
	Σ	Approach LOS					A (5.6)						A (9.4)	
	A	Storage				-								-
~		50th Queue				-								-
Ľ		95th Queue				25								25
ЗU	Overall Std   LOS D								(6.5)					
-		Approach Std					D						D	
	Σ	Approach LOS					A (5.1)						B (11.4)	
	Ъ	Storage				-								-
		50th Queue				-								-
		95th Queue				25								50

#### 5.7 Herndon Street at Site Driveway B (Intersection 7)

The intersection of Herndon Street at Site Driveway B (Intersection 7) is projected to operate at an acceptable LOS overall and for each approach under the Projected 2024 Build conditions. The intersection is proposed to operate under two-way stop-control with stop control for the eastbound and westbound approaches. The recommended lane configuration for Driveway B is one lane entering the site and one lane exiting the site.

			Н	lerndon Stre	eet	H	lerndon Stre	eet		Niles Avenu	Je		Driveway C	)
				Northboun	d		Southboun	d		Eastbound	k		Westbound	k
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	D						(0.4)					
		Approach Std					D						D	
	Σ	Approach LOS					A (0.2)						A (8.7)	
	A	Storage				•								-
		50th Queue				-								-
		95th Queue				0								0
зU		Overall Std   LOS	D						(0.5)					
-		Approach Std					D						D	
	Σ	S Approach LOS					A (0.7)						A (8.8)	
	P	Storage				•								-
		50th Queue				•								-
		95th Queue				0								0

#### 5.8 Herndon Street at Site Driveway C (Intersection 8)

The intersection of Herndon Street at Site Driveway C (Intersection 8) is projected to operate at an acceptable LOS overall and for each approach under the build conditions. The intersection is proposed to operate under two-way stop-control with stop control for the eastbound and westbound approaches. The recommended lane configuration for Driveway C is one lane entering the site and one lane exiting the site.

#### 5.9 Herndon Street at Site Driveway D (Intersection 9)

			F	lerndon Str	eet	ŀ	lerndon Str	eet	E	Baylor Aven	ue		Driveway D	)
				Northboun	d		Southboun	d		Eastbound	b		Westbound	k
			L	Т	R	L	Т	R	L	Т	R	L	Т	R
		Overall Std   LOS	D						(0.2)					
		Approach Std					D						D	
	Σ	Approach LOS					A (0.1)						A (8.7)	
	A	Storage				-								-
		50th Queue				-								-
		95th Queue				0								0
3U		Overall Std   LOS	D						(0.3)					
-		Approach Std		-			D						D	
	Σ	Approach LOS					A (0.5)						A (8.8)	
	P	Storage				•								-
		50th Queue				-								-
		95th Queue				0								0

The intersection of Herndon Street at Site Driveway D (Intersection 9) is projected to operate at an acceptable LOS overall and for each approach under the build conditions. The intersection is proposed to operate under two-way stop-control with stop control for the eastbound and westbound approaches. The recommended lane configuration for Driveway D is one lane entering the site and one lane exiting the site.

			Н	lerndon Str	eet	H	lerndon Str	eet					Driveway E	
				Northboun	d		Southboun	d		Eastbound	b		Westbound	k
			Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
	_	Overall Std   LOS	D						(3.3)					
		Approach Std					D						D	
	Σ	Approach LOS					A (3.9)						A (8.7)	
	۲	Storage				•								-
		50th Queue				-								-
		95th Queue				25								25
3UI		Overall Std   LOS	D						(4.6)					
		Approach Std					D						D	
	Σ	Approach LOS					A (3.2)						A (8.7)	
	₫	Storage				-								-
		50th Queue				-								-
		95th Queue				25								25

#### 5.10 Herndon Street at Site Driveway E (Intersection 10)

The intersection of Herndon Street at Site Driveway E (Intersection 10) is projected to operate at an acceptable LOS overall and for each approach under the build conditions. The intersection is proposed to operate under two-way stop-control with stop control for the westbound approach only. The recommended lane configuration for Driveway E is one lane entering the site and one lane exiting the site.





![](_page_42_Figure_0.jpeg)

![](_page_43_Figure_0.jpeg)

![](_page_44_Figure_0.jpeg)

![](_page_44_Picture_1.jpeg)

![](_page_45_Figure_0.jpeg)

# **Proposed Site Plan**

![](_page_47_Figure_0.jpeg)

# **Trip Generation Analysis**

Trip Generation Analysis (10th	Ed. with <i>2nd Edition Handbook</i> Daily Project Granite DRI #3298 City of Atlanta, GA	IC & 3rd I	Edition A	M/PM I	C)			
Land Use	Density	Daily	AN	1 Peak H	our PN	I Peak H	our	
		Trips	Total	In	Out	Total	In	Out
Proposed Site Trips								
160 Data Center	1,150,000 s.f.	1,138	127	70	57	104	31	73
220 Multi-Family Housing (Low-Rise)	45 d.u.	300	22	5	17	29	18	11
221 Multi-Family Housing (Mid-Rise)	350 d.u.	1,906	117	30	87	147	90	57
710 General Office Building	640,000 s.f.	6,422	628	540	88	664	106	558
820 Shopping Center	35,000 s.f. gross leasable area	1,322	33	20	13	133	64	69
932 High-Turnover (Sit-Down) Restaurant	35,000 s.f.	3,926	348	191	157	342	212	130
	Total Site Trips	15,014	1,275	856	419	1,419	521	898
Redevelopment Trips								
140 Manufacturing	475,000 s.f.	-1,662	-295	-227	-68	-318	-99	-219
710 General Office Building	37,500 s.f.	-410	-62	-53	-9	-45	-7	-38
	Total Redevelopment Trips	-2,072	-357	-280	-77	-363	-106	-257
Proposed Gross Trips		12,942	918	576	342	1.056	415	641
Residential Trips		1,902	100	25	75	131	81	50
Mixed-Use Reductions		-480	-20	-2	-18	-46	-29	-17
Alternative Mode Reductions		-242	-14	-4	-10	-14	-9	-6
Adjusted Residential Trips		1,180	66	19	47	71	43	27
Office Trine		5.526	450	200	(2	40.4	70	415
Office Trips		5,536	452	389	03 27	494	/9	415
Alternative Mode Reductions		-130	-//	-40	-37	-10	-0	-10
Adjusted Office Trips		4 440	311	290	22	397	61	336
rajusted office rips		1,110	511	270		571	01	550
Retail Trips		1,140	24	15	9	99	48	51
Mixed-Use Reductions		-154	-12	-7	-5	-62	-33	-29
Alternative Mode Reductions		-167	-2	-1	-1	-6	-3	-4
Pass By Reductions (Based on ITE Rates)		-277	0	0	0	-11	-6	-6
Adjusted Retail Trips		542	10	7	3	20	6	12
Restaurant Trins		3 384	251	138	113	255	158	97
Mixed-Use Reductions		-456	-85	-48	-37	-68	-28	-40
Alternative Mode Reductions		-498	-28	-15	-13	-32	-22	-10
Pass By Reductions (Based on ITE Rates)		-1,046	0	0	0	-67	-34	-34
Adjusted Restaurant Trips		1,384	138	75	63	88	74	13
Industrial Trips		980	91	50	41	77	23	54
Truck Trips (0% of Industrial Trips)		0	0	0	0	0	0	0
Car Trips (100% of Industrial Trips)		980	91	50	41	77	23	54
Alternative Mode Reductions		-167	-15	-9	-7	-13	-4	-9
Adjusted Car Trips		813	76	41	34	64	19	45
Adjusted Industrial Trips		813	76	41	34	64	19	45
Mixed-Use Reductions - TOTAL		-1,276	-194	-97	-97	-192	-96	-96
Alternative Mode Reductions - TOTAL		-1,984	-123	-88	-35	-146	-50	-98
Pass-By Reductions - TOTAL		-1,323	0	0	0	-78	-40	-40
New Net Trips		8,359	601	391	210	640	229	407
Driveway Volumes		11,754	958	671	287	1,081	375	704
		<u> </u>						

Note: All daily trips were calculated based on rates or equations on a Weekday, peak hour trips were calculated based on rates or equations on a Weekday for the peak hour of the adjacent street traffic (AM - one hour between 7AM and 9AM, PM - one hour between 4PM and 6PM)

Equation - ITE Trip Generation Manual, 10th Edition

Rate - ITE Trip Generation Manual, 10th Edition

## Intersection Volume Worksheets

## INTERSECTION VOLUME DEVELOPMENT INTERSECTION #1 Marietta Blvd at West Marietta St

viarietta	Biva	at	west	Mar	ietta	2

Image: books with the series of th							AM	I PEAK HOL	JR								
Image: Note of the strate of the s			Marie	tta Blvd			Mariet	ta Blvd			West M	larietta St			West M	arietta St	
U-TurnU-Tu			North	nbound			South	bound			East	bound			West	bound	
Observe Districtive Volume         0         59         79         90         93         90         93         90         93         90         93         90         93         90         93         90         93         90         93         90         93         90         93         90         93 <th< th=""><th></th><th>U-Turn</th><th>Left</th><th>Through</th><th>Right</th><th>U-Turn</th><th>Left</th><th>Through</th><th>Right</th><th>U-Turn</th><th>Left</th><th>Through</th><th>Right</th><th>U-Turn</th><th>Left</th><th>Through</th><th>Right</th></th<>		U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Pedestring colspan="4">IIIAnd Stort Rate2.0%<	Observed 2018 Traffic Volumes	0	59	392	27	0	558	221	89	0	245	495	73	0	16	100	354
Conflicting heavy vhicksII<	Pedestrians			1				0				4				0	
demy whick is which is a sector of a sector	Conflicting Pedestrians		4		0		0		4		0		1		1		0
Heary leasy le	Heavy Vehicles	0	24	12	3	0	8	13	29	0	33	10	19	0	2	10	12
Peck broin Adjustment         Image: Display in the second s	Heavy Vehicle %	2%	41%	3%	11%	2%	2%	6%	33%	2%	13%	2%	26%	2%	13%	10%	3%
Adjusted 201 Volume1.05	Peak Hour Factor		0	.98			0.	98			0	.98			0	.98	
Adjusted 2021 Volumes00241011011011 <th< td=""><td>Adjustment</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td><td>1.05</td></th<>	Adjustment	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Anal Growth Rate Growth Rate Gr	Adjusted 2021 Volumes	0	62	412	28	0	586	232	93	0	257	520	77	0	17	105	372
Annal growth Rate Growth Factor2.0%																	
Grown Fractor Grown Fractor D224 No-Build Tariffic Trip Distribution NUT Trip Distribution NUT Trip Distribution NUT Trip Distribution NUT Trip Distribution NUT Distribution NUT Pip Distribution OUT Pip Distribution OUT Pip Distribution OUT Pip Distribution O	Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
2024 No-Build Taffic06643730606222669902735208200018110935Trip Distribution OUT5%120%1-120%1120%111<	Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Trip Distribution N Trip Distribution OUT         Image: Normal Sector Sect	2024 No-Build Traffic	0	66	437	30	0	622	246	99	0	273	552	82	0	18	111	395
Trip Distribution N         Image: Constraint of the second s																	
Trip Distribution OUT         Image of the second seco	Trip Distribution IN				5%		20%					20%					
Ware local Crips         0         0         2         0         8         0         0         0         8         0         0         8         0         0         8         0         0         8         0         0         8         0         0         8         0         0         8         0         0         8         0	Trip Distribution OUT														5%	20%	20%
New problem in the problem in t	Warehouse Car Trips	0	0	0	2	0	8	0	0	0	0	8	0	0	2	7	7
Trip Distribution IN Trip Distribution OUT Residential Trips         Image: model intermediate intermed																	
Trip Distribution OUT Residential TripsImage of the second of the secon	Trip Distribution IN				5%		15%					10%					
Residential Trips0000300002000 <td>Trip Distribution OUT</td> <td></td> <td>5%</td> <td>10%</td> <td>15%</td>	Trip Distribution OUT														5%	10%	15%
Trip Distribution OUT         Image: marked mar	Residential Trips	0	0	0	1	0	3	0	0	0	0	2	0	0	2	5	7
Trip Distribution NV Trip Distribution OVT OPICe TripImage: Sign of the sign of t																	
Trip Distribution OUT         Image: Description of Description OUT         Image: De	Trip Distribution IN				5%		20%					20%					
Office Trips0010150580005800144Trip Distribution NU Trip Distribution QUT Real Trips15%120%1120%120%120%120%120%120%120%120%120%120%111 </td <td>Trip Distribution OUT</td> <td></td> <td>5%</td> <td>20%</td> <td>20%</td>	Trip Distribution OUT														5%	20%	20%
Vertication out in the point of th	Office Trips	0	0	0	15	0	58	0	0	0	0	58	0	0	1	4	4
Trip Distribution IN Trip Distribution OUT         Image: Ima																	
Trip Distribution QUT Retail Trips     Image: Constraint of the symbol of	Trip Distribution IN				5%		20%					20%					
Retail Trips         0         0         0         0         1         0         0         0         1         0         0         0         1         0         0         0         0         1         0         0         0         0         0         0         1         0         0         0         0         0         0         0         0         0         0         0         0         0         0         1         0 <t< td=""><td>Trip Distribution OUT</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>5%</td><td>20%</td><td>20%</td></t<>	Trip Distribution OUT														5%	20%	20%
Vertication of the probability of	Retail Trips	0	0	0	0	0	1	0	0	0	0	1	0	0	0	1	1
Trip Distribution IN Trip Distribution QUT Restaurant Trips     Image: Constant Strip Strips     Image: Constant Strip Strips     Strip Strip Strips     Strip Strip Strip Strips     Strip St		_				_				_				_			
Trip Distribution QUT Restaurant Trips         Image: mark of the state of th	Trip Distribution IN				5%		20%					20%					
Restaurant Trips         0         0         0         4         0         15         0         0         15         0         0         3         13         13           Pass-Py Distribution IN Pass-Py Distribution OUT         Image: Constraint of the state of t	Trip Distribution OUT														5%	20%	20%
Pass-By Distribution IN Pass-By Distribution OUT         Image: Constraint of the image: Constraintof the image: Constraint of the image: Constraint of the image: C	Restaurant Trips	0	0	0	4	0	15	0	0	0	0	15	0	0	3	13	13
Pass-By Distribution IN Pass-By Distribution OUT         I																	
Pass-By Distribution OUT         Image: Constraint of the state	Pass-By Distribution IN																
Pass-By Trips         0         <	Pass-By Distribution OUT																
O         O         O         O         Second Harmonic Action         Second Harmonicon Action         Second Harmonic Action <td>Pass-By Trips</td> <td>0</td>	Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips         0         0         0         22         0         85         0         0         0         84         0         0         8         30         32           2024 Build Traffic         0         66         437         52         0         707         246         99         0         273         636         82         0         26         141         427           2024 Heavy Vehicle %         2%         41%         3%         6%         2%         6%         33%         2%         13%         2%         26%         2%         9%         8%         3%																	
O         66         437         52         O         707         246         99         O         273         636         82         O         26         141         427           2024 Heavy Vehicle %         2%         41%         3%         6%         2%         6%         33%         2%         13%         2%         26%         2%         3%         3%         3%         3%         2%         13%         2%         26%         2%         8%         3%	Total Project Trips	0	0	0	22	0	85	0	0	0	0	84	0	0	8	30	32
2024 Build Traffic         0         66         437         52         0         707         246         99         0         273         636         82         0         26         141         427           2024 Heavy Vehicle %         2%         41%         3%         6%         2%         6%         33%         2%         13%         2%         26%         2%         8%         3%		_				_				_				_			
2024 Heavy Vehicle % 2% 41% 3% 6% 2% 2% 6% 33% 2% 13% 2% 26% 2% 9% 8% 3%	2024 Build Traffic	0	66	437	52	0	707	246	99	0	273	636	82	0	26	141	427
	2024 Heavy Vehicle %	2%	41%	3%	6%	2%	2%	6%	33%	2%	13%	2%	26%	2%	9%	8%	3%

						PIV	1 PEAK HOU	JR								
		Marie	tta Blvd			Marie	tta Blvd			West M	arietta St			West M	arietta St	
		North	bound			South	nbound			East	bound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2018 Traffic Volumes	0	64	212	16	0	271	559	152	0	122	137	200	0	46	372	460
Pedestrians			3				0				2				0	
Conflicting Pedestrians		2		0		0		2		0		3		3		0
Heavy Vehicles	0	23	2	3	0	4	5	32	0	19	5	26	0	1	5	8
Heavy Vehicle %	2%	36%	2%	19%	2%	2%	2%	21%	2%	16%	4%	13%	2%	2%	2%	2%
Peak Hour Factor		0.	.96			0	.96			0	.96			0	.96	
Adjustment	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Adjusted 2021 Volumes	0	67	223	17	0	285	587	160	0	128	144	210	0	48	391	483
Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	71	237	18	0	302	623	170	0	136	153	223	0	51	415	513
	_				_				_				_			
Trip Distribution IN				5%		20%					20%					
Trip Distribution OUT														5%	20%	20%
Warehouse Car Trips	0	0	0	1	0	4	0	0	0	0	4	0	0	2	9	9
Trip Distribution IN				5%		15%					10%					
Trip Distribution OUT														5%	10%	15%
Residential Trips	0	0	0	2	0	6	0	0	0	0	4	0	0	1	3	4
Trip Distribution IN				5%		20%					20%					
Trip Distribution OUT														5%	20%	20%
Office Trips	0	0	0	3	0	12	0	0	0	0	12	0	0	17	67	67
Trip Distribution IN				5%		20%					20%					
Trip Distribution OUT														5%	20%	20%
Retail Trips	0	0	0	0	0	1	0	0	0	0	1	0	0	1	2	2
						n		n		n		n				
Trip Distribution IN				5%		20%					20%					
Trip Distribution OUT														5%	20%	20%
Restaurant Trips	0	0	0	4	0	15	0	0	0	0	15	0	0	1	3	3
					-				-				-			
Pass-By Distribution IN																
Pass-By Distribution OUT																
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
							1									
Total Project Trips	0	0	0	10	0	38	0	0	0	0	36	0	0	22	84	85
	-															
2024 Build Traffic	0	71	237	28	0	340	623	170	0	136	189	223	0	73	499	598
2024 Heavy Vehicle %	2%	36%	2%	12%	2%	2%	2%	21%	2%	16%	3%	13%	2%	2%	2%	2%

## INTERSECTION VOLUME DEVELOPMENT INTERSECTION #2 Herndon St at West Marietta St

						AM	PEAK HOU	JR								
		Hern	don St							West M	arietta St			West Ma	arietta St	
		North	bound			South	bound			East	ound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	6	0	19	0	0	0	0	0	0	377	7	0	18	338	0
Pedestrians			0				0				0				)	
Conflicting Pedestrians		0		0	(	)		0	(	)		0	(	)		0
Heavy Vehicles	0	1	0	1	0	0	0	0	0	0	45	1	0	1	48	0
Heavy Vehicle %	2%	17%	2%	5%	2%	2%	2%	2%	2%	2%	12%	14%	2%	6%	14%	2%
Peak Hour Factor		0.	.97			0.	97			0.	97			0.	97	
Adjustment	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36
Adjusted 2021 Volumes	0	14	0	45	0	0	0	0	0	0	890	17	0	42	798	0
		r														
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	0.0%	2.0%	0.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	14	0	45	0	0	0	0	0	0	944	18	0	45	847	0
Trip Distribution IN											15%	30%		25%		
Trip Distribution OUT		40%		25%											5%	
Warehouse Car Trips	0	20	0	13	0	0	0	0	0	0	9	18	0	15	3	0
			r								r					
Trip Distribution IN											10%	20%		30%		
Trip Distribution OUT		25%		30%											5%	
Residential Trips	0	19	0	23	0	0	0	0	0	0	3	6	0	9	4	0
			r								r					
Trip Distribution IN											15%	30%		25%		
Trip Distribution OUT		40%		25%											5%	
Office Trips	0	19	0	12	0	0	0	0	0	0	66	132	0	110	2	0
	-		T.				1				I					
Trip Distribution IN											15%	30%		25%		
Trip Distribution OUT		40%		25%											5%	
Retail Trips	0	3	0	2	0	0	0	0	0	0	2	4	0	3	0	0
					-		-		-		-		-			-
Trip Distribution IN											15%	30%		25%		
Trip Distribution OUT		40%		25%				-					-		5%	-
Restaurant Trips	0	43	0	27	0	0	0	0	0	0	19	38	0	32	5	0
							1				I			500/	500/	
Pass-By Distribution IN		500/												50%	-50%	
Pass-By Distribution OUT		50%				0		0		0		0		0		
Pass-By Trips	U	U	U	U	U	U	0	U	U	U	U	U	U	U	U	U
Tatal Brainst Trias	0	104	0	77	0	0	0	0	0	0	00	100	0	100	14	0
Total Project Trips	0	104	0		U	U	0	U	U	0	99	198	U	169	14	U
2024 Build Traffia		110	0	122	•	•	0	0	•	0	1.042	210	•	214	001	0
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	1,045	210	2%	214	1/1%	2%
2024 HEavy VEHILIE /0	270	270	270	2./0	270	2/0	270	2/0	270	2/0	11/0	270	270	270	14/0	270

PM PEAK HOUR

Herndon St Northbound										West M	arietta St			West M	arietta St	
		North	nbound			South	bound		Eastbound					West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	7	0	19	0	0	0	0	0	0	457	8	1	17	490	0
Pedestrians			0				0				0				0	
Conflicting Pedestrians		0		0		0		0		0		0		0		0
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	19	0	0	0	19	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	4%	2%	2%	2%	4%	2%
Peak Hour Factor		0	.94	-		0	.94			0	.94			0	.94	
Adjustment	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4
Adjusted 2021 Volumes	0	10	0	27	0	0	0	0	0	0	640	11	1	24	686	0
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	0.0%	2.0%	0.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	10	0	27	0	0	0	0	0	0	679	12	1	25	728	0
	-	1	1	1		1		1		1	450/	202/		250/	1	1
Trip Distribution IN	-	100/		250/							15%	30%		25%	50/	
Werehouse Con Tries	0	40%	0	25%	0	0	0	0	0	0	4	0	0	7	5%	0
warehouse car trips	0	20	U	10	U	U	U	U	U	U	4	0	U	/	3	U
Trip Distribution IN	1										10%	20%		30%		
Trip Distribution OUT		25%		30%											5%	
Residential Trips	0	11	0	14	0	0	0	0	0	0	7	14	0	21	2	0
Trip Distribution IN	-										15%	30%		25%		
Trip Distribution OUT		40%		25%											5%	
Office Trips	0	192	0	120	0	0	0	0	0	0	13	26	0	22	24	0
Trip Distribution IN											15%	30%		25%		
Trip Distribution OUT		40%		25%											5%	
Retail Trips	0	12	0	8	0	0	0	0	0	0	3	7	0	6	2	0
		1													1	
Trip Distribution IN	-	100/		250/							15%	30%		25%	50/	
	-	40%		25%		•		•		•	10	20		22	5%	
Restaurant Trips	0	18	0	12	0	0	0	0	0	0	19	38	0	32	2	0
Pass-By Distribution IN	1													50%	-50%	
Pass-By Distribution OUT		50%														
Pass-By Trips	0	20	0	0	0	0	0	0	0	0	0	0	0	20	-20	0
Total Project Trips	0	279	0	170	0	0	0	0	0	0	46	93	0	108	13	0
	- · ·	2.5		1/0	Ŭ		, v	ÿ	Ŭ		-10		, v	100		
2024 Build Traffic	0	289	0	197	0	0	0	0	0	0	725	105	1	133	741	0
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	4%	2%	2%	2%	4%	2%

#### INTERSECTION VOLUME DEVELOPMENT INTERSECTION #3 St

Joseph E Lowery B	ilvd at West Marietta S
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						AN	I PEAK HOU	JR								
		Joseph E L	owery Blvd			Joseph E L	owery Blvd			West M	arietta St			West M	arietta St	
		North	bound			South	bound			East	ound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2018 Traffic Volumes	0	252	0	215	0	0	0	0	0	0	908	205	0	137	227	0
Pedestrians			0				0				1				D	
Conflicting Pedestrians		1		0		D		1		0		0	(	0		0
Heavy Vehicles	0	9	0	0	0	0	0	0	0	0	13	3	0	1	10	0
Heavy Vehicle %	2%	4%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	4%	2%
Peak Hour Factor		0.	96			0	.96			0.	96			0.	96	
Adjustment	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Adjusted 2021 Volumes	0	265	0	226	0	0	0	0	0	0	953	215	0	144	238	0
Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	281	0	240	0	0	0	0	0	0	1,011	228	0	153	253	0
Trip Distribution IN		30%													25%	
Trip Distribution OUT											25%	30%				
Warehouse Car Trips	0	12	0	0	0	0	0	0	0	0	9	10	0	0	10	0
Trip Distribution IN		30%													40%	
Trip Distribution OUT											40%	30%				
Residential Trips	0	6	0	0	0	0	0	0	0	0	19	14	0	0	8	0
Trip Distribution IN		30%													25%	
Trip Distribution OUT											25%	30%				
Office Trips	0	87	0	0	0	0	0	0	0	0	6	7	0	0	73	0
					_											
Trip Distribution IN		30%													25%	
Trip Distribution OUT											25%	30%				
Retail Trips	0	2	0	0	0	0	0	0	0	0	1	1	0	0	2	0
					-											
Trip Distribution IN		30%													25%	
Trip Distribution OUT											25%	30%				
Restaurant Trips	0	23	0	0	0	0	0	0	0	0	16	19	0	0	19	0
Pass-By Distribution IN																
Pass-By Distribution OUT																
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		1														
Total Project Trips	0	130	0	0	0	0	0	0	0	0	51	51	0	0	112	0
2024 Build Traffic	0	411	0	240	0	0	0	0	0	0	1,062	279	0	153	365	0
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	3%	2%

						PM	I PEAK HOU	JR								
		Joseph E L	owery Blvd			Joseph E L	owery Blvd			West M	arietta St			West M	arietta St	
		North	bound			South	bound			East	bound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2018 Traffic Volumes	0	166	0	166	0	0	0	0	0	0	259	206	1	456	775	0
Pedestrians			1				0				3				0	
Conflicting Pedestrians		3		0		0		3		0		1		1		0
Heavy Vehicles	0	3	0	1	0	0	0	0	0	0	4	2	0	1	6	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.98			0	.98			0	.98			0.	98	
Adjustment	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Adjusted 2021 Volumes	0	174	0	174	0	0	0	0	0	0	272	216	1	479	814	0
Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	185	0	185	0	0	0	0	0	0	289	229	1	508	864	0
	_				_				_				_			
Trip Distribution IN		30%													25%	
Trip Distribution OUT											25%	30%				
Warehouse Car Trips	0	6	0	0	0	0	0	0	0	0	11	14	0	0	5	0
Trip Distribution IN		30%													40%	
Trip Distribution OUT											40%	30%				
Residential Trips	0	13	0	0	0	0	0	0	0	0	11	8	0	0	17	0
Trip Distribution IN		30%													25%	
Trip Distribution OUT											25%	30%				
Office Trips	0	18	0	0	0	0	0	0	0	0	84	101	0	0	15	0
Trip Distribution IN		30%													25%	
Trip Distribution OUT											25%	30%				
Retail Trips	0	2	0	0	0	0	0	0	0	0	3	4	0	0	2	0
Trip Distribution IN		30%													25%	
Trip Distribution OUT											25%	30%				
Restaurant Trips	0	22	0	0	0	0	0	0	0	0	3	4	0	0	19	0
Pass-By Distribution IN																
Pass-By Distribution OUT																
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	61	0	0	0	0	0	0	0	0	112	131	0	0	58	0
																-
2024 Build Traffic	0	246	0	185	0	0	0	0	0	0	401	360	1	508	922	0
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%

## INTERSECTION VOLUME DEVELOPMENT INTERSECTION #4 Marietta St NW at West Marietta St

viarietta	Sτ	IN VV	at	west	iviar	ietta	ŝ

Mariella Si Mariella Si Mariella Si Mariella Mariell							AM	Ι ΡΕΑΚ ΗΟΙ	JR								
Normal			Mariet	ta St NW			Brady /	Ave NW			West M	arietta St			8th 9	it NW	
U-TurnLeftThroughRightU-TurnLeftThroughRightU-TurnLeftThroughRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightRightU-TurnLeftThroughRightZDecketsins1030001100<			North	nbound			South	bound			East	bound			West	bound	
Observe SQ21 Traffic Volumes         0         150         0         0         1         0         0         1         0         1         0         1         0         1         0         1         0         1         0         1         1         1         1         1         1         0 <t< th=""><th></th><th>U-Turn</th><th>Left</th><th>Through</th><th>Right</th><th>U-Turn</th><th>Left</th><th>Through</th><th>Right</th><th>U-Turn</th><th>Left</th><th>Through</th><th>Right</th><th>U-Turn</th><th>Left</th><th>Through</th><th>Right</th></t<>		U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Productional product of the series	Observed 2021 Traffic Volumes	0	152	27	0	0	1	30	243	0	274	38	516	0	1	16	2
Configure productions heavy vehicle %         Image of the set of t	Pedestrians			0				1				1				3	
new yieldis heavy yieldis provide factor         0         3         0         0         1         0         0         1         0         7         0	Conflicting Pedestrians		1		3		3		1		1		0		0		1
new yelloc % peak low Factor         2%         <	Heavy Vehicles	0	3	0	0	0	0	1	0	0	1	0	7	0	0	0	0
Peak Horis Pricing         Description         Description <thdescription< th=""></thdescription<>	Heavy Vehicle %	2%	2%	2%	2%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Adjustment         1.06         1.06         1.06         1.06         1.06         1.06         1.06         1.06         2.36         3.36	Peak Hour Factor		0	.89			0.	.89			0	.89			0	89	
Adjusted 2021 Volumes         0         161         29         0         200         547         0         2         38         5           Annual Growth Rate Growth Factor         2.0%         0 <td>Adjustment</td> <td>1.06</td> <td>1.06</td> <td>1.06</td> <td>2.36</td> <td>1.06</td> <td>2.36</td> <td>1.06</td> <td>1.06</td> <td>1.06</td> <td>1.06</td> <td>2.36</td> <td>1.06</td> <td>2.36</td> <td>2.36</td> <td>2.36</td> <td>2.36</td>	Adjustment	1.06	1.06	1.06	2.36	1.06	2.36	1.06	1.06	1.06	1.06	2.36	1.06	2.36	2.36	2.36	2.36
Annual Growth Rate Growth Factor         2.0%         1.06	Adjusted 2021 Volumes	0	161	29	0	0	2	32	258	0	290	90	547	0	2	38	5
Annual Growth Rate Growth Factor         2.0%																	
Grown Fractor         1.06         1.0         0.0         0.0	Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
2024 Build Traffic         0         17.1         31         0         0         2         34         274         0         308         96         580         0         2         40         5           Trip Distribution N Trip Distribution OUT         5%         1         1         1         20%         1	Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
Trip Distribution IN Trip Distribution OUT         5%         I         I         I         20%         I         I         I         I         I           Warehouse Car Trips         0         2         0 <td>2024 No-Build Traffic</td> <td>0</td> <td>171</td> <td>31</td> <td>0</td> <td>0</td> <td>2</td> <td>34</td> <td>274</td> <td>0</td> <td>308</td> <td>96</td> <td>580</td> <td>0</td> <td>2</td> <td>40</td> <td>5</td>	2024 No-Build Traffic	0	171	31	0	0	2	34	274	0	308	96	580	0	2	40	5
Trip Distribution IN Trip Distribution OUT       S%       Image: Constraint of the second sec																	
Trip Distribution OUT       Image: Constraint of the second	Trip Distribution IN		5%						20%								
Warehouse Car Trips         0         2         0         0         0         0         8         0         7         0         2         0         0         0         0           Trip Distribution IV Trip Distribution OUT         10%         1         10%         1 </td <td>Trip Distribution OUT</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>20%</td> <td></td> <td>5%</td> <td></td> <td></td> <td></td> <td></td>	Trip Distribution OUT										20%		5%				
Trip Distribution IN       10%       Image: constraint of the second sec	Warehouse Car Trips	0	2	0	0	0	0	0	8	0	7	0	2	0	0	0	0
Trip Distribution IN Trip Distribution OUT       Image: Constraint of the second																	
Trip Distribution OUT       Image of the second secon	Trip Distribution IN		10%						30%								
Residential Trips         0         2         0         0         0         0         6         0         14         0         5         0         0         0         0         0           Trip Distribution OUT         5%           20%          20%         5%	Trip Distribution OUT										30%		10%				
Trip Distribution NU       5%       Image: Sign of the sign of th	Residential Trips	0	2	0	0	0	0	0	6	0	14	0	5	0	0	0	0
Signification IN Trip Distribution OUT       Signification IN In Distribution OUT       Signification IN Integration IN Trip Distribution OUT       Signification IN Integration IN Integration IN Trip Distribution OUT       Signification IN Integration IN Integration IN Trip Distribution OUT       Signification IN Integration Integration Integration Integration Integration Integration Integrat																	
Trip Distribution OUT       Image: Constraint of the second	Trip Distribution IN		5%						20%								
Office Trips       0       15       0       0       0       0       0       58       0       4       0       1       0       0       0       0       0         Trip Distribution NU       5%         20%        20%   <	Trip Distribution OUT										20%		5%				
Trip Distribution IN Trip Distribution OUT       5%       Image: Constraint of the second sec	Office Trips	0	15	0	0	0	0	0	58	0	4	0	1	0	0	0	0
Signal Constraint     Signal Constra																	
Trip Distribution OUT       Image: Constraint of the second	Trip Distribution IN		5%						20%								
Retail Trips         0 <t< td=""><td>Trip Distribution OUT</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>20%</td><td></td><td>5%</td><td></td><td></td><td></td><td></td></t<>	Trip Distribution OUT										20%		5%				
Trip Distribution IN       5%       Image: Signature of the signatex of the signature of the signatex of the s	Retail Trips	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0
Trip Distribution IN     5%     I     I     I     I     I     I     I     I       Trip Distribution OUT     0     4     0     0     0     0     0     13     0     3     0     0     0     0       Pass-By Distribution OUT     I										-							
Trip Distribution OUT       Image: Constraint of the second	Trip Distribution IN		5%						20%								
Restaurant Trips       0       4       0       0       0       0       15       0       13       0       3       0       0       0       0       0         Pass-By Distribution IN Pass-By Distribution OUT	Trip Distribution OUT										20%		5%				
Pass-By Distribution IN Pass-By Distribution OUT         Image: Constraint of the second	Restaurant Trips	0	4	0	0	0	0	0	15	0	13	0	3	0	0	0	0
Pass-By Distribution N         Image: Constraint of the stress of th																	
Pass-By Distribution OUT     O	Pass-By Distribution IN																
Pass-By Trips         0         <	Pass-By Distribution OUT																
Total Project Trips         0         23         0         0         0         0         88         0         39         0         11         0         0         0         0           2024 Build Traffic         0         194         31         0         0         2         34         362         0         347         96         591         0         2         40         5	Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips         0         23         0         0         0         0         88         0         39         0         11         0         0         0         0           Total Project Trips         0         23         0         0         0         0         88         0         39         0         11         0         0         0         0           Total Project Trips           Total Project Trips         0         10         0																	
2024 Build Traffic 0 194 31 0 0 2 34 362 0 347 96 591 0 2 40 5	Total Project Trips	0	23	0	0	0	0	0	88	0	39	0	11	0	0	0	0
2024 Build Traffic         0         194         31         0         0         2         34         362         0         347         96         591         0         2         40         5																	
	2024 Build Traffic	0	194	31	0	0	2	34	362	0	347	96	591	0	2	40	5
2024 Heavy Vehicle % 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2	2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	3%	2%	2%	2%	2%	2%	2%	2%	2%	2%

						PM	I PEAK HOU	JR								
		Mariet	ta St NW			Brady	Ave NW			West M	arietta St			8th 9	it NW	
		North	nbound			South	bound			East	bound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	468	51	3	0	3	28	548	0	154	56	220	0	3	35	5
Pedestrians			0				1				2				1	
Conflicting Pedestrians		2		1		1		2		1		0		כ		1
Heavy Vehicles	0	0	0	0	0	0	0	6	0	0	0	2	0	0	0	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.94	n		0.	.94			0	.94			0	94	
Adjustment	1.06	1.06	1.06	1.4	1.06	1.4	1.06	1.06	1.06	1.06	1.4	1.06	1.4	1.4	1.4	1.4
Adjusted 2021 Volumes	0	496	54	4	0	4	30	581	0	163	78	233	0	4	49	7
			1			r		1				1			r.	
Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	526	57	4	0	4	32	617	0	173	83	247	0	4	52	7
			1								1					
Trip Distribution IN		5%						20%								
Trip Distribution OUT										20%		5%				
Warehouse Car Trips	0	1	0	0	0	0	0	4	0	9	0	2	0	0	0	0
					-						1		-			
Trip Distribution IN		10%						30%								
Trip Distribution OUT										30%		10%				
Residential Trips	0	4	0	0	0	0	0	13	0	8	0	3	0	0	0	0
			1								1					-
Trip Distribution IN		5%						20%								
Trip Distribution OUT										20%		5%				
Office Trips	0	3	0	0	0	0	0	12	0	67	0	17	0	0	0	0
	1		1	1			1				1					
Trip Distribution IN		5%						20%								
Trip Distribution OUT										20%		5%		0	0	0
Retail Trips	0	0	0	0	0	0	0	1	0	2	0	1	0	0	0	0
The provide second	1	50/						200/			1					
Trip Distribution IN		5%						20%		200/		59/				
Protection COT	0		0	0	0	0	0	15	0	20%	0	5%	0	0	0	0
Restaurant mps	U	4	U	U	U	U	U	15	0	3	U	1	U	U	U	U
Page By Distribution IN	1	I		1			1				1					
Pass-By Distribution OUT																
Pass-By Distribution OUT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1 033-by 111ps	U	U	U	U	U	U	U	U	U	U	U	U	U	U	U	U
Total Project Trips	0	12	0	0	0	0	0	45	0	89	0	24	0	0	0	0
				, v	, v	. <u> </u>	, , , , , , , , , , , , , , , , , , ,		Ŭ		, v		, v	, v	. <u> </u>	•
2024 Build Traffic	0	538	57	4	0	4	32	662	0	262	83	271	0	4	52	7
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
*Green text represents movements assoc	ciated with th	e 2021 count	s collected to	account for t	he 8th street	leg not being	included in th	ne original 20	17 counts				•			

## INTERSECTION VOLUME DEVELOPMENT INTERSECTION #5 Joseph E Lowery Blvd at Donald Lee Hollowell Pkwy

						AM	PEAK HOU	JR								
		Joseph E L	owery Blvd			Joseph E L	owery Blvd			Donald Lee H	Iollowell Pkwy			Donald Lee H	ollowell Pkwy	r
		North	bound			South	bound			East	bound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2018 Traffic Volumes	0	164	371	60	0	99	152	13	0	96	1,113	104	0	21	240	93
Pedestrians			4				1				3				3	
Conflicting Pedestrians		3		3	3	3		3		1		4		4		1
Heavy Vehicles	0	0	10	0	0	8	4	0	0	4	20	2	0	0	12	1
Heavy Vehicle %	2%	2%	3%	2%	2%	8%	3%	2%	2%	4%	2%	2%	2%	2%	5%	2%
Peak Hour Factor		0.	.97			0.	97			0	97			0.	97	
Adjustment	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Adjusted 2021 Volumes	0	172	390	63	0	104	160	14	0	101	1,169	109	0	22	252	98
Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	183	414	67	0	110	170	15	0	107	1,241	116	0	23	267	104
Trip Distribution IN			5%							15%						10%
Trip Distribution OUT						10%	5%	15%								
Warehouse Car Trips	0	0	2	0	0	3	2	5	0	6	0	0	0	0	0	4
Trip Distribution IN			5%							5%						20%
Trip Distribution OUT						20%	5%	5%								
Residential Trips	0	0	1	0	0	9	2	2	0	1	0	0	0	0	0	4
Trip Distribution IN			5%							15%						10%
Trip Distribution OUT						10%	5%	15%								
Office Trips	0	0	15	0	0	2	1	3	0	44	0	0	0	0	0	29
Trip Distribution IN			5%							15%						10%
Trip Distribution OUT						10%	5%	15%								
Retail Trips	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
Trip Distribution IN			5%							15%						10%
Trip Distribution OUT						10%	5%	15%								
Restaurant Trips	0	0	4	0	0	6	3	9	0	11	0	0	0	0	0	8
		n														
Pass-By Distribution IN																
Pass-By Distribution OUT																
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
			1													
Total Project Trips	0	0	22	0	0	20	8	19	0	63	0	0	0	0	0	46
2024 Build Traffic	0	183	436	67	0	130	178	34	0	170	1,241	116	0	23	267	150
2024 Heavy Vehicle %	2%	2%	3%	2%	2%	7%	3%	2%	2%	3%	2%	2%	2%	2%	5%	2%

						PN	I PEAK HO	JR								
		Joseph E	Lowery Blvd			Joseph E L	owery Blvd			Donald Lee H	lollowell Pkwy	/		Donald Lee H	Iollowell Pkwy	
		Nort	hbound			South	bound			East	bound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2018 Traffic Volumes	0	162	217	41	0	178	228	134	0	11	531	136	0	71	1,060	55
Pedestrians			7				6				5				5	
Conflicting Pedestrians		5		5		5		5		6		7		7		6
Heavy Vehicles	0	1	2	0	0	4	0	1	0	0	9	2	0	1	17	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		(	).95			0	.95			0	.95			0	.95	
Adjustment	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Adjusted 2021 Volumes	0	170	228	43	0	187	239	141	0	12	558	143	0	75	1,113	58
Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	180	242	46	0	198	254	150	0	13	592	152	0	80	1,181	62
	_				_				_				_			
Trip Distribution IN			5%							15%						10%
Trip Distribution OUT						10%	5%	15%								
Warehouse Car Trips	0	0	1	0	0	5	2	7	0	3	0	0	0	0	0	2
Trip Distribution IN			5%							5%						20%
Trip Distribution OUT						20%	5%	5%								
Residential Trips	0	0	2	0	0	5	1	1	0	2	0	0	0	0	0	9
Trip Distribution IN			5%							15%						10%
Trip Distribution OUT						10%	5%	15%								
Office Trips	0	0	3	0	0	34	17	50	0	9	0	0	0	0	0	6
													_			
Trip Distribution IN			5%							15%						10%
Trip Distribution OUT						10%	5%	15%								
Retail Trips	0	0	0	0	0	1	1	2	0	1	0	0	0	0	0	1
													_			
Trip Distribution IN			5%							15%						10%
Trip Distribution OUT						10%	5%	15%								
Restaurant Trips	0	0	4	0	0	1	1	2	0	11	0	0	0	0	0	7
													_			
Pass-By Distribution IN																
Pass-By Distribution OUT																
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	10	0	0	46	22	62	0	26	0	0	0	0	0	25
	_															
2024 Build Traffic	0	180	252	46	0	244	276	212	0	39	592	152	0	80	1,181	87
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%

## INTERSECTION VOLUME DEVELOPMENT INTERSECTION #6 Driveway A at West Marietta St NW

						AM	PEAK HOU	IR								
		Drive	way A							West Mar	etta St NW			West Mari	etta St NW	
		North	bound			South	bound			East	ound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	0	0					0	0	396	0	0	0	356	0
Pedestrians																
Conflicting Pedestrians																
Heavy Vehicles	0	0	0	0					0	0	46	0	0	0	49	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	12%	2%	2%	2%	14%	2%
Peak Hour Factor		0.	.97			0.	97			0.	97			0.	97	
Adjustment	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36
Adjusted 2021 Volumes	0	0	0	0					0	0	935	0	0	0	840	0
Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	0	0					0	0	992	0	0	0	891	0
Trip Distribution IN												15%		30%	25%	
Trip Distribution OUT		5%		30%							25%					
Warehouse Car Trips	0	3	0	15	0	0	0	0	0	0	13	9	0	18	15	0
Trip Distribution IN												10%		40%	30%	
Trip Distribution OUT		5%		40%							30%					
Residential Trips	0	4	0	30	0	0	0	0	0	0	23	3	0	12	9	0
Trip Distribution IN												15%		30%	25%	
Trip Distribution OUT		5%		30%							25%					
Office Trips	0	2	0	14	0	0	0	0	0	0	12	66	0	132	110	0
					_				_							
Trip Distribution IN												15%		30%	25%	
Trip Distribution OUT		5%		30%							25%					
Retail Trips	0	0	0	2	0	0	0	0	0	0	2	2	0	4	3	0
Trip Distribution IN												15%		30%	25%	
Trip Distribution OUT		5%		30%							25%					
Restaurant Trips	0	5	0	32	0	0	0	0	0	0	27	19	0	38	32	0
			1													
Pass-By Distribution IN											-50%	50%				
Pass-By Distribution OUT				50%												
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
							,									
Total Project Trips	0	14	0	93	0	0	0	0	0	0	77	99	0	204	169	0
2024 Build Traffic	0	14	0	93					0	0	1,069	99	0	204	1,060	0
2024 Heavy Vehicle %	2%	2%	2%	2%					2%	2%	11%	2%	2%	2%	12%	2%

						PM	PEAK HOU	IR								
		Drive	way A							West Mar	ietta St NW			West Ma	rietta St NW	
		North	bound			South	bound			East	bound			Wes	tbound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	0	0					0	0	476	0	0	0	508	0
Pedestrians																
Conflicting Pedestrians																
Heavy Vehicles	0	0	0	0					0	0	19	0	0	0	19	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	4%	2%	2%	2%	4%	2%
Peak Hour Factor		0	.94			0.	94			0	.94			C	.94	
Adjustment	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4
Adjusted 2021 Volumes	0	0	0	0					0	0	666	0	0	0	711	0
Annual Growth Rate	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	0	0					0	0	707	0	0	0	755	0
Trip Distribution IN												15%		30%	25%	
Trip Distribution OUT		5%		30%							25%					
Warehouse Car Trips	0	3	0	19	0	0	0	0	0	0	16	4	0	8	7	0
· ·			•										-			
Trip Distribution IN												10%		40%	30%	
Trip Distribution OUT		5%		40%							30%					
Residential Trips	0	2	0	18	0	0	0	0	0	0	14	7	0	28	21	0
			•													
Trip Distribution IN												15%		30%	25%	
Trip Distribution OUT		5%		30%							25%					
Office Trips	0	24	0	144	0	0	0	0	0	0	120	13	0	26	22	0
			•										-			
Trip Distribution IN												15%		30%	25%	
Trip Distribution OUT		5%		30%							25%					
Retail Trips	0	2	0	9	0	0	0	0	0	0	8	3	0	7	6	0
•													-			
Trip Distribution IN												15%		30%	25%	
Trip Distribution OUT		5%		30%							25%					
Restaurant Trips	0	2	0	14	0	0	0	0	0	0	12	19	0	38	32	0
													-			
Pass-By Distribution IN											-50%	50%				
Pass-By Distribution OUT				50%												
Pass-By Trips	0	0	0	20	0	0	0	0	0	0	-20	20	0	0	0	0
	•															
Total Project Trips	0	33	0	224	0	0	0	0	0	0	150	66	0	107	88	0
					•				•							
2024 Build Troffic	0	33	0	224					0	0	857	66	0	107	843	0
2024 Bullu Hallic	•										0.07		-	107	0.40	-

## INTERSECTION VOLUME DEVELOPMENT INTERSECTION #7 Herndon St at Church St

						AN	I PEAK HOU	JR								
		Hern	don St			Hern	don St			Chu	rch St			Drive	way B	
		North	nbound			South	bound			East	bound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	25	0	0	0	25	0	0	10	0	0	0	0	0	0
Pedestrians																
Conflicting Pedestrians																
Heavy Vehicles	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	8%	2%	2%	2%	8%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.97			0.	.97			0	.97			0	97	
Adjustment	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	1	1	1	1	2.36	2.36	2.36	2.36
Adjusted 2021 Volumes	0	0	59	0	0	0	59	0	0	10	0	0	0	0	0	0
					_				_				_			
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	59	0	0	0	59	0	0	10	0	0	0	0	0	0
Trip Distribution IN						45%	10%									
Trip Distribution OUT			10%													55%
Warehouse Car Trips	0	0	5	0	0	27	6	0	0	0	0	0	0	0	0	28
	-				_				_				_			
Trip Distribution IN						35%	15%									
Trip Distribution OUT			15%													40%
Residential Trips	0	0	11	0	0	10	4	0	0	0	0	0	0	0	0	30
	_															
Trip Distribution IN						55%										
Trip Distribution OUT																65%
Office Trips	0	0	0	0	0	243	0	0	0	0	0	0	0	0	0	31
		n		n												
Trip Distribution IN						55%										
Trip Distribution OUT																65%
Retail Trips	0	0	0	0	0	7	0	0	0	0	0	0	0	0	0	5
	-	n		n												
Trip Distribution IN						55%										
Trip Distribution OUT																65%
Restaurant Trips	0	0	0	0	0	70	0	0	0	0	0	0	0	0	0	70
			1				1									
Pass-By Distribution IN																
Pass-By Distribution OUT	-															
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		1		1												
Total Project Trips	0	0	16	0	0	357	10	0	0	0	0	0	0	0	0	164
	-															
2024 Build Traffic	0	0	75	0	0	357	69	0	0	10	0	0	0	0	0	164
2024 Heavy Vehicle %	2%	2%	6%	2%	2%	2%	7%	2%	2%	2%	2%	2%	2%	2%	2%	2%

						PM	PEAK HOU	JR								
		Hern	don St			Hern	don St			Chu	rch St		1	Drive	eway B	
		North	nbound			South	bound			East	bound			Wes	tbound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	26	0	0	0	25	0	0	10	0	0	0	0	0	0
Pedestrians		·										•				
Conflicting Pedestrians																
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.94			0.	94			0	.94			C	.94	
Adjustment	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1	1	1	1	1.4	1.4	1.4	1.4
Adjusted 2021 Volumes	0	0	36	0	0	0	35	0	0	10	0	0	0	0	0	0
					-							r				r
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	36	0	0	0	35	0	0	10	0	0	0	0	0	0
	-	1	Т		-						1					
Trip Distribution IN						45%	10%									
Trip Distribution OUT			10%													55%
Warehouse Car Trips	0	0	6	0	0	12	3	0	0	0	0	0	0	0	0	35
		1	T	l.						l.	L					
Trip Distribution IN						35%	15%									
Trip Distribution OUT			15%													40%
Residential Trips	0	0	7	0	0	25	11	0	0	0	0	0	0	0	0	18
		1	1				-								1	
Trip Distribution IN						55%										
Trip Distribution OUT																65%
Office Trips	0	0	0	0	0	48	0	0	0	0	0	0	0	0	0	311
		1	1				-								1	
Trip Distribution IN						55%										
Trip Distribution OUT		-	-	-				-	-	-		-		-	-	65%
Retail Trips	0	0	0	0	0	12	0	0	0	0	0	0	0	0	0	20
			1		1		1								1	
Trip Distribution IN						55%										650/
Trip Distribution OUT						70		•		•						65%
Restaurant Trips	0	0	0	0	0	70	0	0	0	0	0	0	0	0	0	30
Dage Du Distribution (N													1		1	
Pass-By Distribution IN																
Pass-By Distribution OUT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-By Trips	U	U	0	U	U	U	U	U	U	U	U	U	U	U	U	U
Total Project Trips	0	0	13	0	0	167	14	0	0	0	0	0	0	0	0	414
Total Hoject Hips			15	0	0	107	-14	5	0	0		0		0	0	414
2024 Build Traffic	0	0	49	0	0	167	49	0	0	10	0	0	0	0	0	414
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
J,, ,	270	270			270	270	2/0	270							270	270

## INTERSECTION VOLUME DEVELOPMENT INTERSECTION #8 Herndon St at Niles Ave

н	er	n	aor	13	π	at	. IN	lies	A

Interview         <		AM PEAK HOUR           Herndon St         Niles Ave         Driveway C																
Image: stratement of the		Herndon St         Herndon St         Niles Ave         Driveway C           Northbound         Southbound         Eastbound         Westbound           IJ-Turn         Left         Through         Right         Li-Turn         Left         Through         Right																
Union         Leth         Two of the state          The bitchick of the state<			Nort	hbound			South	nbound			East	bound			West	bound		
Oheery Barle Contractional problem Contractional problem Contractio		U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	
Image: The second seco	Observed 2021 Traffic Volumes	0	0	25	0	0	0	25	0	0	10	0	0	0	0	0	0	
Conflicting herry Vehicles herry Vehicles New	Pedestrians			1				1				1						
den0002002000<	Conflicting Pedestrians						n				r							
neary leasy l	Heavy Vehicles	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0	
Peak Houring Adjustment         2.36         2	Heavy Vehicle %	2%	2%	8%	2%	2%	2%	8%	2%	2%	2%	2%	2%	2%	2%	2%	2%	
Adjusted 201Q.26Q.26Q.26Q.26Q.26Q.26Q.26Q.26Q.1IIIQ.36Q.36Q.36Q.36Q.36Adjusted 201OneOO	Peak Hour Factor		(	0.97			0	.97			0	.97			0	.97		
Adjust 2021 Volumes00000000000000Armal Growth Rate Growth Factor0.0%0.	Adjustment	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	1	1	1	1	2.36	2.36	2.36	2.36	
Anal Growth Rate Growth Rate Growth Rate Growth Rate Growth Rate Growth Rate Try Distribution N         0.0%        <	Adjusted 2021 Volumes	0	0	59	0	0	0	59	0	0	10	0	0	0	0	0	0	
Annal Growth Rate Growth Factor0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% Loo0.0% 							r		r		r		r					
Growfinction1.00 <td>Annual Growth Rate</td> <td>0.0%</td> <td>2.0%</td> <td>2.0%</td> <td>2.0%</td> <td>2.0%</td>	Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%	
202 Model Artific000100<	Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06	
Trip Distribution IN         Image: Normal Sector Sect	2024 No-Build Traffic	0	0	59	0	0	0	59	0	0	10	0	0	0	0	0	0	
Implementation N         Implementation N<							r		r		r		r					
Implicitation OUT Warehouse Car rigisImplicitation OUT 0Implicitation OUT 0 <td>Trip Distribution IN</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>10%</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Trip Distribution IN							10%										
Warehold Car TripsOOO<	Trip Distribution OUT			10%														
Trip Distribution NU Trip Distribution OUT         Image: marked mar	Warehouse Car Trips	0	0	5	0	0	0	6	0	0	0	0	0	0	0	0	0	
Int D istribution IN Trip D istribution OUT Top D istribution OUT Top D istribution OUT 							n		n		r		n					
The bishbution OUT Residential TripsImage: Bishbution OUT 00 <td>Trip Distribution IN</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>10%</td> <td>5%</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Trip Distribution IN						10%	5%										
Residenti Trips000	Trip Distribution OUT			5%													10%	
Trip Distribution NI Trip Distribution OUT         Image: Normal System Sys	Residential Trips	0	0	4	0	0	3	1	0	0	0	0	0	0	0	0	8	
The Distribution IN Trip Distribution OUTImage and the Description of the Description OUTImage and the Descr						_				-								
Trip Distribution OUT         Image         Image<	Trip Distribution IN																	
Office Trips000 <th< td=""><td>Trip Distribution OUT</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>	Trip Distribution OUT																	
Trip Distribution N Trip Distribution OUT         Image: Normal System Syst	Office Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Trip Distribution IN Trip Distribution OUT         Image: Im						_												
Trip Distribution OUT         Image	Trip Distribution IN																	
Retail Trips         0 <t< td=""><td>Trip Distribution OUT</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	Trip Distribution OUT																	
Trip Distribution N         Image: Second Secon	Retail Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Trip Distribution IN         Image: Boot Strip Distribution OUT         Image: Boot Strip Distrip DistripDistrip DistripDistrip Distr						-												
Trip Distribution OUT     Image: Description OUT     Image: Descrip	Trip Distribution IN																	
Restaurant Trips         0	Trip Distribution OUT																	
Pass-By Distribution N         Image: Normal Symbol Sy	Restaurant Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Pass-By Distribution IN         Image: Constraint of the state o						_												
Pass-by Distribution OUT         Image: Constraint of the straint of the strain	Pass-By Distribution IN																	
Pass-By Trips         0         <	Pass-By Distribution OUT																	
Operation         Operation <t< td=""><td>Pass-By Trips</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td></t<>	Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total Project Trips         0         0         9         0         0         3         7         0         0         0         0         0         0         0         0         0         0         0         0         0         0         0         8           Co24 Build Traffic         O <th cols<="" td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>_</td><td></td><td></td><td></td><td>_</td><td></td><td></td><td></td></th>	<td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>_</td> <td></td> <td></td> <td></td> <td>_</td> <td></td> <td></td> <td></td>										_				_			
O248 build Traffic         O         O         68         O         O         3         66         O	Total Project Trips	0	0	9	0	0	3	7	0	0	0	0	0	0	0	0	8	
2024 Build Traffic         0         68         0         0         3         66         0         0         10         0         0         0         0         0         8           2024 Heavy Vehicle %         2%         2%         7%         2%         7%         2%		-								_								
2024 Heavy Vehicle % 2% 2% 7% 2% 2% 7% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2% 2%	2024 Build Traffic	0	0	68	0	0	3	66	0	0	10	0	0	0	0	0	8	
	2024 Heavy Vehicle %	2%	2%	7%	2%	2%	2%	7%	2%	2%	2%	2%	2%	2%	2%	2%	2%	

						PM	I PEAK HOL	JR								
		Hern	don St		Ι	Hern	don St			Nile	s Ave			Drive	way C	
		North	bound			South	bound			East	bound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	26	0	0	0	25	0	0	10	0	0	0	0	0	0
Pedestrians				•						•					•	
Conflicting Pedestrians																
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.94			0.	.94			0	.94			0	.94	
Adjustment	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1	1	1	1	1.4	1.4	1.4	1.4
Adjusted 2021 Volumes	0	0	36	0	0	0	35	0	0	10	0	0	0	0	0	0
					-											
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	36	0	0	0	35	0	0	10	0	0	0	0	0	0
							•				•					
Trip Distribution IN							10%									
Trip Distribution OUT			10%													
Warehouse Car Trips	0	0	6	0	0	0	3	0	0	0	0	0	0	0	0	0
Trip Distribution IN						10%	5%									
Trip Distribution OUT			5%													10%
Residential Trips	0	0	2	0	0	7	4	0	0	0	0	0	0	0	0	5
Trip Distribution IN																
Trip Distribution OUT																
Office Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN																
Trip Distribution OUT																
Retail Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
					-				_				_			
Trip Distribution IN																
Trip Distribution OUT																
Restaurant Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
					-				_				_			
Pass-By Distribution IN																
Pass-By Distribution OUT																
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		1	1			r	1				1	r				
Total Project Trips	0	0	8	0	0	7	7	0	0	0	0	0	0	0	0	5
2024 Build Traffic	0	0	44	0	0	7	42	0	0	10	0	0	0	0	0	5
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%

### INTERSECTION VOLUME DEVELOPMENT INTERSECTION #9 Herndon St at Baylor St

пеі	nu	UII	sι	aι	Dd	yiui	э

						AN	Ι ΡΕΑΚ ΗΟΙ	JR								
		Hern	idon St			Hern	Bay	lor St			Drive	way D				
		North	nbound			South	bound			East	ound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	25	0	0	0	25	0	0	10	0	0	0	0	0	0
Pedestrians																
Conflicting Pedestrians																
Heavy Vehicles	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	8%	2%	2%	2%	8%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.97			0	.97			0	97			0	97	
Adjustment	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	1	1	1	1	2.36	2.36	2.36	2.36
Adjusted 2021 Volumes	0	0	59	0	0	0	59	0	0	10	0	0	0	0	0	0
				r			r									
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	59	0	0	0	59	0	0	10	0	0	0	0	0	0
Trip Distribution IN							10%									
Trip Distribution OUT			10%													
Warehouse Car Trips	0	0	5	0	0	0	6	0	0	0	0	0	0	0	0	0
Trip Distribution IN						5%										
Trip Distribution OUT																5%
Residential Trips	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	4
Trip Distribution IN																
Trip Distribution OUT																
Office Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN																
Trip Distribution OUT																
Retail Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN																
Trip Distribution OUT																
Restaurant Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
											r					
Pass-By Distribution IN																
Pass-By Distribution OUT																
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	5	0	0	1	6	0	0	0	0	0	0	0	0	4
	-															
2024 Build Traffic	0	0	64	0	0	1	65	0	0	10	0	0	0	0	0	4
2024 Heavy Vehicle %	2%	2%	7%	2%	2%	2%	7%	2%	2%	2%	2%	2%	2%	2%	2%	2%

						PM	PEAK HOL	JR								
		Hern	idon St			Hern	don St			Bay	lor St			Drive	eway D	
		North	nbound			South	bound			East	bound			Wes	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	26	0	0	0	25	0	0	10	0	0	0	0	0	0
Pedestrians																
Conflicting Pedestrians																
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0	.94	n		0	94			0	.94	n		C	.94	
Adjustment	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1	1	1	1	1.4	1.4	1.4	1.4
Adjusted 2021 Volumes	0	0	36	0	0	0	35	0	0	10	0	0	0	0	0	0
	- 1	1	1	1			1			1	1	1		1	1	
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	36	0	0	0	35	0	0	10	0	0	0	0	0	0
Taia Distrikution IN	- 1	1			-		100/	-						1	1	1
Trip Distribution IN			109/				10%									-
Warehouse Car Trins	0	0	10%	0	0	0	2	0	0	0	0	0	0	0	0	0
warehouse car mps	0	0	0	0	0	U	3	U	0	0	0	0	0	U	0	0
Trip Distribution IN						5%										1
Trip Distribution OUT																5%
Residential Trips	0	0	0	0	0	4	0	0	0	0	0	0	0	0	0	2
			1													
Trip Distribution IN																
Trip Distribution OUT																
Office Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN																
Trip Distribution OUT																
Retail Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
				1								1	1	1		1
Trip Distribution IN																
Trip Distribution OUT	-		-			•		•		•						-
Restaurant Trips	U	U	0	0	U	U	U	U	0	U	U	U	0	U	0	0
Page By Distribution IN	1		1	1					-		1	1	1	1	1	1
Pass-By Distribution OUT																-
Pass-By Distribution COT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1 035 Dy 111p5	-				5	5		5						0		
Total Project Trips	0	0	6	0	0	4	3	0	0	0	0	0	0	0	0	2
2024 Build Traffic	0	0	42	0	0	4	38	0	0	10	0	0	0	0	0	2
2024 Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%

#### INTERSECTION VOLUME DEVELOPMENT INTERSECTION #10 Herndon St at

						AM	PEAK HOL	JR								
		Hern	don St			Hern	don St							Drive	way E	
		North	bound			South	bound			East	ound			West	ound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	25	0	0	0	25	0					0	0	0	0
Pedestrians																
Conflicting Pedestrians																
Heavy Vehicles	0	0	2	0	0	0	2	0					0	0	0	0
Heavy Vehicle %	2%	2%	8%	2%	2%	2%	8%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0.	.97			0.	97			0.	97			0.	97	
Adjustment	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36	2.36
Adjusted 2021 Volumes	0	0	59	0	0	0	59	0					0	0	0	0
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	59	0	0	0	59	0					0	0	0	0
Trip Distribution IN						10%										
Trip Distribution OUT																10%
Warehouse Car Trips	0	0	0	0	0	6	0	0	0	0	0	0	0	0	0	5
Trip Distribution IN																
Trip Distribution OUT																
Residential Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN																
Trip Distribution OUT																
Office Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN																
Trip Distribution OUT																
Retail Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	-				-											
Trip Distribution IN																
Trip Distribution OUT																
Restaurant Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
					_											
Pass-By Distribution IN																
Pass-By Distribution OUT																
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	0	0	0	6	0	0	0	0	0	0	0	0	0	5
2024 Build Traffic	0	0	59	0	0	6	59	0					0	0	0	5
2024 Heavy Vehicle %	2%	2%	8%	2%	2%	2%	8%	2%					2%	2%	2%	2%

PM	PEAK	HOUR

	Herndon St				Herndon St							Driveway E				
		North	bound			South	bound			East	ound			West	bound	
	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right	U-Turn	Left	Through	Right
Observed 2021 Traffic Volumes	0	0	26	0	0	0	25	0	0	0	0	0	0	0	0	0
Pedestrians																
Conflicting Pedestrians																
Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0.	.94			0	94			0	94			0	.94	
Adjustment	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4	1.4
Adjusted 2021 Volumes	0	0	36	0	0	0	35	0	0	0	0	0	0	0	0	0
Annual Growth Rate	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%	2.0%
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.06	1.06	1.06	1.06	1.06	1.06	1.06	1.06
2024 No-Build Traffic	0	0	36	0	0	0	35	0	0	0	0	0	0	0	0	0
Trip Distribution IN	1	1		[	1	1.0%		[	1			1	1		1	
Trip Distribution AUT						1070										10%
Warehouse Car Trips	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	6
warehouse car mps	Ū	0	0	0	0	5	Ū	U	U	Ū	U	U	Ū	0	0	Ū
Trip Distribution IN																
Trip Distribution OUT																
Residential Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN	1			-				-				1			1	
Trip Distribution OUT																
Office Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN																
Trip Distribution OUT																
Retail Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trip Distribution IN																
Trip Distribution OUT																
Restaurant Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Page Ry Distribution IN	1															
Pass-by Distribution III	-															
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
					•								•			
Total Project Trips	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	6
2024 Build Traffic	0	0	36	0	0	2	35	0	0	0	0	0	0	0	0	6
2024 Beavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
2024 Heavy Venicie /6	2/0	2/0	270	2.70	2/0	2/0	2/0	2/0	2/0	2/0	270	270	270	2/0	2/0	2/0

# **Programmed Project Fact Sheets**

AR-490F	Atlanta Region's Plan RTP (2	020) PROJECT FACT SHEET
Short Title	ATLANTA STREETCAR - NORTHWEST BELTLINE CORRIDOR FROM NEAR INTERSECTION OF WESTVIEW DRIVE AT LANGHORN STREET TO MARTA BANKHEAD RAIL STATION	ake Ave WW
GDOT Project No.	N/A	N Toole Bird WW
Federal ID No.	N/A	
Status	Long Range	Martin Cuther King Jr Dr NW
Service Type	Transit / Rail Capital	
Sponsor	MARTA	20 402 Westview Dr SW
Jurisdiction	Regional - Central	0 0.5 1 Miles
Analysis Level	In the Region's Air Quality Conformity Analysis	Sinte Ave Silv
Existing Thru Lane	N/A LCI	Network Year 2050
Planned Thru Lane	N/A Flex	Corridor Length TBD miles
Detailed Description a	and Justification	
This project constructs a ne MARTA Bankhead heavy rai	ew streetcar line along the Beltline corridor between the inter il station	section of Westview Drive and Langhorn Street to the

Phas	se Status & Funding	Status	FISCAL	TOTAL PHASE	BREAKDOWN	OF TOTAL PHAS	E COST BY FUNI	DING SOURCE		
Info	rmation		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE		
ALL	New Starts		LR 2041- 2050	\$96,900,000	\$33,915,000	\$0,000	\$0,000	\$62,985,000		
				\$96,900,000	\$33,915,000	\$0,000	\$0,000	\$62,985,000		

 SCP: Scoping
 PE: Preliminary engineering / engineering / design / planning
 PE-OV: GDOT oversight services for engineering
 ROW: Right-of-way Acquistion

 UTL: Utility relocation
 CST: Construction / Implementation
 ALL: Total estimated cost, inclusive of all phases
 ROW: Right-of-way Acquistion

AR-491B	Atlanta Region's Plan RTP (2	020) PROJECT FACT SHEET
Short Title	NORTH AVENUE CORRIDOR HIGH CAPACITY PREMIUM TRANSIT SERVICE FROM MARTA NORTH AVENUE RAIL STATION TO MARTA BANKHEAD RAIL STATION	Romer and the second se
GDOT Project No.	N/A	Provide And Provid
Federal ID No.	N/A	
Status	Long Range	Joseph E Boone Blvd NW
Service Type	Transit / Bus Capital	
Sponsor	MARTA	Martin of ther Aing Jr. Dr. NW
Jurisdiction	City of Atlanta	0 0.5 1 Miles 154
Analysis Level	In the Region's Air Quality Conformity Analysis	EastExpy
Existing Thru Lane	N/A LCI	Network Year 2050
Planned Thru Lane	N/A Flex	Corridor Length TBD miles
Detailed Description a	and Justification	
This project will provide hig heavy rail stations.	h capacity premium transit service along the North Avenue c	orridor between MARTA's North Avenue and Bankhead

Phas	se Status & Funding	Status	FISCAL	TOTAL PHASE	BREAKDOWN	OF TOTAL PHAS	E COST BY FUNI	DING SOURCE
Info	rmation		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE
ALL	Local Jurisdiction/Municipality Funds		LR 2041- 2050	\$62,900,000	\$0,000	\$0,000	\$0,000	\$62,900,000
				\$62,900,000	\$0,000	\$0,000	\$0,000	\$62,900,000

 SCP: Scoping
 PE: Preliminary engineering / engineering / design / planning
 PE-OV: GDOT oversight services for engineering
 ROW: Right-of-way Acquistion

 UTL: Utility relocation
 CST: Construction / Implementation
 ALL: Total estimated cost, inclusive of all phases
 ROW: Right-of-way Acquistion

AR-491C	Atlanta Region's Plan RTP (20	020) PROJECT FACT SHEET
Short Title	NORTHSIDE DRIVE CORRIDOR HIGH CAPACITY PREMIUM TRANSIT SERVICE FROM ATLANTA METROPOLITAN STATE COLLEGE TO I-75 NORTH	S-Inman 9 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
GDOT Project No.	N/A	emetery Voseph E Boone Blvd NW
Federal ID No.	N/A	
Status	Long Range	Westview Company
Service Type	Transit / Bus Capital	Centerna A
Sponsor	MARTA	Castade Aven Ave SW stream W
Jurisdiction	City of Atlanta	
Analysis Level	In the Region's Air Quality Conformity Analysis	
Existing Thru Lane	N/A LCI	Network Year 2050
Planned Thru Lane	N/A Flex	Corridor Length TBD miles
Detailed Description a	and Justification	
This project will provide hig State College area.	gh capacity premium transit service along the Northside Drive	e corridor between I-75 north and the Atlanta Metropolitan

Phase Status & Funding Status		FISCAL	TOTAL PHASE	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE				
Information		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE	
ALL	New Starts		LR 2041- 2050	\$167,000,000	\$58,450,000	\$0,000	\$0,000	\$108,550,000
				\$167,000,000	\$58,450,000	\$0,000	\$0,000	\$108,550,000

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases ROW: Right-of-way Acquistion

AT-240	Atlanta Region's Plan RTP (20	020) PROJECT FACT SHEET
Short Title	US 78/278/SR 8 (D.L. HOLLOWELL PARKWAY) PEDESTRIAN FACILITY - PHASE A FROM WEST LAKE AVENUE/FLORENCE PLACE TO PROCTOR CREEK (WEST OF GARY AVENUE)	NW Francis, PLN WW
GDOT Project No.	0010322	₽ ₽ > Blyss Ave NW. ₹
Federal ID No.	N/A	St NV
Status	Programmed	North
Service Type	Last Mile Connectivity / Joint Bike-Ped Facilities	MAN MAN DE TRANSPORT
Sponsor	City of Atlanta	r sth
Jurisdiction	City of Atlanta	0 250 500 Feet
Analysis Level	Exempt from Air Quality Analysis (40 CFR 93)	Copyright 2005 Aero Surveys of Georgia, Inc. Reproduced by permission of the copyright owner. Contact http://www.aeroatlas.com
Existing Thru Lane	4 LCI X	Network Year TBD
Planned Thru Lane	4 <b>Flex</b>	Corridor Length 0.8 miles

#### **Detailed Description and Justification**

The proposed improvements would construct a 9-foot multi-use path (6-foot sidewalks and 4-foot one way bike pair) along Donald Lee Hollowell and add streetscape trees, pedestrian and street lighting inside a 6-foot tree planting zone along Donald Lee Hollowell from West Lake Ave./Florence Place to Proctor Creek (west of Gary Avenue). The proposed improvements to this project would also realign West Lake Avenue with Florence Place. The proposed improvements would also re-stripe Chappell Road in order to align through movements across Donald Lee Hollowell Parkway, eliminating the existing conflicting lane alignments. The proposed improvements would also add a dedicated left turn lane on Chappell Road, add dedicated left turn lanes with adequate storage along Donald Lee Hollowell, add a dedicated right turn lane to westbound Donald Lee Hollowell, and improve the right turn radius on southbound Dobbs Street.

Phas	se Status & Funding	Status	FISCAL	TOTAL PHASE	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
Information			YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE
PE	STP - Urban (>200K) (ARC)	AUTH	2011	\$698,000	<del>\$469,833</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$228,167</del>
ROW	Local Jurisdiction/Municipality Funds		2021	\$1,373,213	\$0,000	\$0,000	\$0,000	\$1,373,213
UTL	Local Jurisdiction/Municipality Funds		2023	\$998,589	\$0,000	\$0,000	\$0,000	\$998,589
CST	Surface Transportation Block Grant (STBG) Program - Urban (>200K) (ARC) - LCI Setaside for Implementation		2023	\$3,695,069	\$2,956,055	\$739,014	\$0,000	\$0,000
				\$6,764,871	\$3,425,888	\$739,014	\$0,000	\$2,599,969

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases

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For additional information about this project, please call (404) 463-3100 or email transportation@atlantaregional.com.

![](_page_65_Picture_7.jpeg)

AT-277A	Atlanta Region's Plan RTP (2	020) PROJECT FACT SHEET
Short Title	CYCLE ATLANTA PHASE 1.0 - IMPLEMENTATION AT VARIOUS LOCATIONS	Collier Rd NW Band Burd Burd Burd Burd Burd Burd Burd Bur
GDOT Project No.	0014993	W 278 Ponce de Leon Aven
Federal ID No.	N/A	Voseph 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
Status	Programmed	]
Service Type	Last Mile Connectivity / Pedestrian Facility	20 <b>1001a</b> 402
Sponsor	City of Atlanta	Glenn Comile Strists
Jurisdiction	City of Atlanta	chiefer a state of the state of
Analysis Level	In the Region's Air Quality Conformity Analysis	Avon Ave SW 2
Existing Thru Lane Planned Thru Lane	5/4/3         LCI           4/3/2         Flex         X	Network Year2030Corridor Length4.6

#### **Detailed Description and Justification**

This project will install the bicycle facilities identified in the ARC funded Cycle Atlanta: Phase 1.0 study. These facilities will support the existing and planned compact development in the central core of the city, as well as within the Atlanta BeltLine Planning Area by supporting cycling as a mode of transportation between varied land uses. Projects include (1) protected bike lanes on Mangum/Walker/Peters/Lee - part of Corridor A, (2) bike lanes and buffered bike lanes on R. McGill Blvd - part of Corridor C, and (3) the Bicycle Boulevard/Neighborway along Woodward Avenue - part of Corridor D. The projects add 4.6 miles of high quality bicycle facilities to Atlanta's network and make key connections within the 31-mile Phase 1.0 network. Portions of this project are located in Equitable Target Areas.

Phase Status & Funding Status		FISCAL	TOTAL PHASE	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE				
Information		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE	
PE	TAP - Urban (>200K) (ARC)	AUTH	2017	\$237,500	<del>\$190,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$47,500</del>
CST	Local Jurisdiction/Municipality Funds		2021	\$2,950,000	\$0,000	\$0,000	\$0,000	\$2,950,000
				\$3,187,500	\$190,000	\$0,000	\$0,000	\$2,997,500

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases

<b>AT-287</b>	Atlanta Region's Plan RTP (2020) PROJECT FACT SHEET									
Short Title	US 19/41/SR 3 (NORTHSIDE DRIVE) SIGNAL UPGRADES AT 13 LOCATIONS	heo Ave NW Ta and the Ave NW T								
GDOT Project No.	0012823	2 78								
Federal ID No.	N/A	W Mariotta St. 14th StNW 0								
Status	Programmed	Interest								
Service Type	Roadway / Operations & Safety	Georgia								
Sponsor	GDOT									
Jurisdiction	City of Atlanta	0 0.25 0.5 Miles								
Analysis Level	Exempt from Air Quality Analysis (40 CFR 93)	Norva								
Existing Thru Lane		Network Year TBD								
Planned Thru Lane	6 Flex	Corridor Length N/A miles								
Detailed Description	and Justification									

Signal upgrades on SR 3 (Northside Drive) and Hemphill Avenue at SR 9 in the City of Atlanta and Georgia Tech area. Total corridor length is approximately 2.5 miles, with 11 signal upgrades: North Avenue, Donald Lee Hollowell Parkway NW, Marietta Street, 10th Street, 14th Street, 17th Street, Deering Road, Bellemeade Avenue, I-75 SB, I-75 NB, and at Hemphill Avenue/14th Street.

Phase Status & Funding Status			FISCAL	TOTAL PHASE	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOUR			
Info	rmation		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE
PE	STP - Urban (>200K) (ARC)	AUTH	2014	\$325,000	<del>\$325,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>
PE	Surface Transportation Block Grant (STBG) Program Flex (GDOT)	AUTH	2018	\$106,000	<del>\$106,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>
ROW	Surface Transportation Block Grant (STBG) Program - Urban (>200K) (ARC)	AUTH	2020	\$466,140	<del>\$466,140</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>
UTL	Congestion Mitigation & Air Quality Improvement (CMAQ)		2022	\$497,831	\$497,831	\$0,000	\$0,000	\$0,000
CST	Congestion Mitigation & Air Quality Improvement (CMAQ)		2022	\$2,018,316	\$2,018,316	\$0,000	\$0,000	\$0,000
				\$3,413,287	\$3,413,287	\$0,000	\$0,000	\$0,000

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases

AT-288	Atlanta Region's Plan RTP (2	020) PROJECT FACT SHEET
Short Title	US 41/SR 3 (NORTHSIDE DRIVE) AND US 19 (14TH STREET) SIGNAL UPGRADES AT 11 LOCATIONS	Joseph E. Lowery. Blvd. NW Joseph E. Lowery. Blvd. NW James, P. Brandey, Dr. NW Surrat Ave NW Wahut St. NW Wahut St. NW
GDOT Project No.	0012821	And
Federal ID No.	N/A	Atlanta College
Status	Programmed	Hore house
Service Type	Roadway / Operations & Safety	ells Ave SW See man Status St 4002 00 00
Sponsor	GDOT	Park StSW 20 - alter winter all States and States
Jurisdiction	City of Atlanta	Dak St SW 0 0.25 0 0.6 Millies we st SN Park S 5 5 5 5 F F F F ST Fick
Analysis Level	Exempt from Air Quality Analysis (40 CFR 93)	Glenn SLSW
Existing Thru Lane		Network Year TBD
Planned Thru Lane	4 Flex	Corridor Length 4.6 miles
Detailed Description a	and Justification	

US 41/SR 3 at: North Ave, Donald Lee Hollowell Pkwy NW, Marietta St, 10th St, 14th St, 17th St, Deering Rd, Bellemeade Ave, I?75 SB, and I?75 NB and Hemphill at US 19/14th St

Phas	se Status & Funding	Status	FISCAL	TOTAL PHASE	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
Info	rmation		YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE
PE	STP - Urban (>200K) (ARC)	AUTH	2014	\$360,035	<del>\$360,035</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>
PE	Surface Transportation Block Grant (STBG) Program Flex (GDOT)	AUTH	2018	\$113,000	<del>\$113,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>
ROW	Surface Transportation Block Grant (STBG) Program - Urban (>200K) (ARC)	AUTH	2020	\$980,220	<del>\$980,220</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>
UTL	Congestion Mitigation & Air Quality Improvement (CMAQ)		2022	\$124,848	\$124,848	\$0,000	\$0,000	\$0,000
CST	Congestion Mitigation & Air Quality Improvement (CMAQ)		2022	\$2,059,771	\$2,059,771	\$0,000	\$0,000	\$0,000
				\$3,637,874	\$3,637,874	\$0,000	\$0,000	\$0,000

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases

AT-301	Atlanta Region's Plan RTP (2020) PROJECT FACT SHEET							
Short Title	JOSEPH E BOONE BOULEVARD ROAD DIET AND COMPLETE STREET IMPROVEMENTS FROM NORTHSIDE DRIVE TO MAYSON TURNER ROAD							
GDOT Project No.	N/A							
Federal ID No.	N/A							
Status	Programmed	NO IMAGE AVAILABLE						
Service Type	Last Mile Connectivity / Complete Street Retrofit	]						
Sponsor	ARC/GDOT	]						
Jurisdiction	City of Atlanta	]						
Analysis Level	In the Region's Air Quality Conformity Analysis	] []						
Existing Thru Lane	4/2 LCI	Network Year 2030						
Planned Thru Lane	3/2 Flex	Corridor Length 1.21 miles						
Detailed Description a	and Justification							
This project will reduce roa	dway capacity on Joseph E Boone Boulevard, providing spac	e for new bicycle lanes and sidewalk improvements.						

Phase Status & Funding Status		FISCAL	L TOTAL PHASE	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE				
Information			YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE
PE	Local Jurisdiction/Municipality Funds	AUTH	2020	\$442,539	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$0,000</del>	<del>\$442,539</del>
CST	Local Jurisdiction/Municipality Funds		2021	\$8,900,000	\$0,000	\$0,000	\$0,000	\$8,900,000
· · ·				\$9,342,539	\$0,000	\$0,000	\$0,000	\$9,342,539

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases ROW: Right-of-way Acquistion

![](_page_69_Picture_3.jpeg)

![](_page_69_Picture_4.jpeg)

AT-373	Atlanta Region's Plan RTP (2	020) PROJECT FACT SHEET			
Short Title	MARIETTA BOULEVARD ROAD RECONSTRUCTION FROM CORONET WAY TO DONALD LEE HOLLOWELL PKWY	All Collier Roll			
GDOT Project No.	0017803				
Federal ID No.	N/A	P Contra Biscontin			
Status	Programmed				
Service Type	Other / Scoping	Johnson no sta			
Sponsor	City of Atlanta				
Jurisdiction	City of Atlanta	0 0.5 1 Miles 78 78 73			
Analysis Level	Exempt from Air Quality Analysis (40 CFR 93)	Letter Williams			
Existing Thru Lane	N/A LCI	Network Year TBD			
Planned Thru Lane	N/A Flex	Corridor Length 3.5 miles			
Detailed Description a	and Justification				
This project will cover scopi Hollowell Parkway NW. The major North-South connecti	ng for a proposed reconstruction and resurfacing of the Mari Pavement Condition Index (PCI) score varies from 37 to 58. on for the City of Atlanta.	etta Boulevard corridor between Coronet Way NW and DL The corridor is on the Regional Freight Route and is a			

Phase Status & Funding Sta		Status	FISCAL	FISCAL TOTAL PHASE	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
Information			YEAR	COST	FEDERAL	STATE	BONDS	LOCAL/PRIVATE
SCP	Surface Transportation Block Grant (STBG) Program - Urban (>200K) (ARC)		2021	\$1,000,000	\$800,000	\$0,000	\$0,000	\$200,000
				\$1,000,000	\$800,000	\$0,000	\$0,000	\$200,000

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquistion UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases