



Transportation Analysis

Sugarloaf Crossing DRI #2472

Gwinnett County, Georgia

Report Prepared:

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TABLE OF CONTENTS

Executive Summary	1
1.0 Project Description	4
1.1 Introduction	4
1.2 Site Plan Review	8
1.3 Site Access	8
1.4 Bicycle and Pedestrian Facilities	8
1.5 Transit Facilities	8
2.0 Traffic Analyses, Methodology and Assumptions	9
2.1 Growth Rate	9
2.2 Traffic Data Collection	9
2.3 Detailed Intersection Analysis	9
3.0 Study Network	10
3.1 Gross Trip Generation	10
3.2 Trip Distribution	10
3.3 Level-of-Service Standards	10
3.4 Study Network Determination	11
3.5 Existing Roadway Facilities	13
4.0 Trip Generation	13
5.0 Trip Distribution and Assignment	14
6.0 Traffic Analysis	17
6.1 Existing 2015 Conditions	17
6.2 Projected 2020 No-Build Conditions	19
6.3 Projected 2020 Build Conditions	22
7.0 Ingress/Egress Analysis	26
8.0 Identification of Programmed Projects	27
9.0 Internal Circulation Analysis	29
10.0 Compliance with Comprehensive Plan Analysis	29

LIST OF TABLES

Table 1	Proposed Land Uses.....	4
Table 2	Peak Hour Summary	9
Table 3	Gross Trip Generation.....	10
Table 4	Intersection Control Summary	11
Table 5	Roadway Classification	13
Table 6	Net Trip Generation.....	14
Table 7	Existing 2015 Intersection Levels-of-Service	17
Table 8	Projected 2020 No-Build Intersection Levels-of-Service	19
Table 9	Projected 2020 No-Build Intersection Levels-of-Service IMPROVED	20
Table 10	Projected 2020 Build Intersection Levels-of-Service.....	22
Table 11	Projected 2020 Build Intersection Levels-of-Service IMPROVED	24
Table 12	Programmed Improvements	27

LIST OF FIGURES

Figure 1	Site Location Map.....	5
Figure 2	Site Aerial.....	6
Figure 3	Site Aerial (SR 8 Frontage)	7
Figure 4	Study Intersections.....	12
Figure 5	Project Traffic Assignment.....	15
Figure 6	Project Trips	16
Figure 7	Existing 2015 Conditions	18
Figure 8	Projected 2020 No-Build Conditions	21
Figure 9	Projected 2020 Build Conditions.....	25
Figure 10	Programmed Projects.....	28

LIST OF APPENDICES

Appendix A	Site Photo Log
Appendix B	Land Use and Zoning Maps
Appendix C	Proposed Site Plan
Appendix D	Trip Generation Analysis
Appendix E	Intersection Volume Worksheets
Appendix F	Programmed Project Fact Sheets

Available Upon Request

Synchro Capacity Analyses
Raw Traffic Count Data

EXECUTIVE SUMMARY

This report presents the analysis of the anticipated traffic impacts of the proposed Sugarloaf Crossing DRI development located in Gwinnett County, Georgia, between the cities of Lawrenceville and Dacula. The approximate 159.87-acre site is bordered by SR 316 (University Parkway) to the northeast and by SR 8 / US 29 (Winder Highway) to the northwest. The proposed development will be a mixed-use development consisting of approximately 1,100,000 square feet of general office space and 275,000 square feet of retail space.

The project is a Development of Regional Impact (DRI) and is subject to Georgia Regional Transportation Authority (GRTA) and Atlanta Regional Commission (ARC) review due to the project size exceeding 500,000 SF of mixed-use development in a Developing Suburbs area type. The DRI for this development is a rezoning with Gwinnett County. The DRI was formally triggered with the filing of the Initial DRI Information (Form 1) on February 9, 2015.

The proposed redevelopment project is expected to be completed by 2020 (approximately 5 years), and this analysis will consider the full build-out of the proposed site in 2020. The proposed site will consist of the following land uses and densities:

General Office:	1,100,000 SF
Retail:	275,000 SF

Capacity analyses were performed throughout the study network for the 2015 Existing conditions, the projected 2020 No-Build conditions, and the projected 2020 Build conditions.

- Existing 2015 conditions represent traffic volumes that were collected in March 2015 by performing AM and PM peak hour turning movement counts.
- Projected 2020 No-Build conditions represent the existing traffic volumes grown for five (5) years at 1.5 percent per year throughout the study network.
- Projected 2020 Build conditions represent the projected 2020 No-Build conditions with the addition of the project trips that are anticipated to be generated by the Sugarloaf Crossing development. Also included, is the one (1) proposed site access driveway, to tie into one of the existing study network intersections.

Based on the Existing 2015 conditions (present conditions; i.e. excludes background traffic growth and excludes the Sugarloaf Crossing DRI project traffic), none of the study intersections operate below the acceptable level-of-service (LOS) standard of D.

Based on the projected 2020 No-Build conditions (includes background traffic growth but excludes the Sugarloaf Crossing DRI project traffic), the following recommended improvements result in all study intersections operating at or above their level-of-service standard (LOS D, or LOS E, where applicable):

- SR 316 (University Parkway) at Cedars Road (Int. #1)
 - Construct an exclusive left-turn lane along the Cedars Road northbound approach.
 - Construct an exclusive left-turn lane along the Cedars Road southbound approach.

- SR 316 (University Parkway) at SR 8 / US 29 (Winder Highway) (Int. #3)
 - Construct a second through lane along the Winder Highway northbound approach.
 - Construct a second through lane along the Winder Highway southbound approach.
- SR 316 (University Parkway) at Harbins Road (Int. #4)
 - Construct a second through lane along the Harbins Road northbound approach.
 - Construct a second through lane along the Harbins Road southbound approach.

Based on the projected 2020 Build conditions (includes background traffic growth and includes the Sugarloaf Crossing DRI project traffic plus the site access driveway), the following recommended improvements result in all study intersections operating at or above their level-of-service standard (LOS D, or LOS E, where applicable). Please note that the following improvements are IN ADDITION TO the improvements associated with the projected 2020 No-Build conditions:

- SR 316 (University Parkway) at SR 8 / US 29 (Winder Highway) (Int. #3)
 - Construct an interchange to eliminate at-grade crossings and related delays.
 - Provide four through lanes along SR 8 at the new interchange (four-lane bridge across SR 316).
 - Provide dual left-turn lanes and a single channelized right-turn lane along each of the SR 316 off-ramp approaches to SR 8.
 - Provide a single southbound left-turn lane and a single northbound channelized right-turn lane along the SR 8 approaches to each of the SR 316 on-ramps.

Or, alternatively (not recommended):

- Construct a third through lane along the SR 316 eastbound approach.
- Construct a third through lane along the SR 316 westbound approach.
- Construct one additional northbound left-turn lane along SR 8 to SR 316, resulting in dual left-turn lanes.
- Construct one additional westbound left-turn lane along SR 316 to SR 8, resulting in dual left-turn lanes.
- Convert the existing northbound channelized right-turn along SR 8 from yield-control to continuous free-flow. Construct an eastbound receiving lane along SR 316, departing from SR 8, to accommodate this free-flow right-turn implementation.
- Convert the existing southbound channelized right-turn along SR 8 from yield-control to continuous free-flow. Construct a westbound receiving lane along SR 316, departing from SR 8, to accommodate this free-flow right-turn implementation.

An interchange is recommended due to the significant roadway geometry (intersection laneage) improvements otherwise required at this intersection. An interchange (grade separation) was once identified as a TIA project, and has been suggested in the City of Dacula's Comprehensive Plan Update (2014).

- Roadway segment of SR 8 / US 29 (Winder Highway) between Alcovy Industrial Blvd. / Proposed Site Driveway (Int. #8) and SR 316 (University Parkway) (Int. #3)
 - Widen SR 8 between Alcovy Industrial Blvd. / Proposed Site Driveway and SR 316 from a 2-lane section to a 4-lane section, to serve the projected Sugarloaf Crossing DRI project traffic between its access point (site driveway) and SR 316.
 - At the southern end of this segment, the additional northbound through travel lane begins with the recommended westbound free-flow right-turn from the Proposed Site Driveway onto SR 8 (see details for Int. #8 below).
- SR 8 / US 29 (Winder Highway) at Alcovy Industrial Blvd. / Proposed Site Driveway (Int. #8)
 - Install a traffic signal (when warranted). It should be noted that for all requests for new traffic signals, an alternative solution that considers a roundabout is required to be investigated, per GDOT policy.
 - Construct an exclusive northbound left-turn lane along SR 8 to Alcovy Industrial Blvd.
 - Construct an exclusive northbound channelized, yield-controlled right-turn lane along SR 8 to the Proposed Site Driveway.
 - Construct exclusive dual southbound left-turn lanes along SR 8 to the Proposed Site Driveway, and convert the existing southbound shared left-turn/through lane to be a through-only lane.
 - Construct an exclusive eastbound left-turn lane along Alcovy Industrial Blvd to SR 8, and convert the existing approach lane to be a through-only lane (while preserving the channelized right-turn).
 - Construct the Proposed Site Driveway to have dual ingress lanes (to receive the recommended dual southbound left-turn lanes) and dual egress lanes.
 - Provide one exclusive left-turn lane, one through lane, and one channelized, continuous free-flow right-turn add lane along the Proposed Site Driveway westbound approach to SR 8. It is recommended that the right-turn lane be formed from the outside egress through travel lane exiting the site.

1.0 PROJECT DESCRIPTION

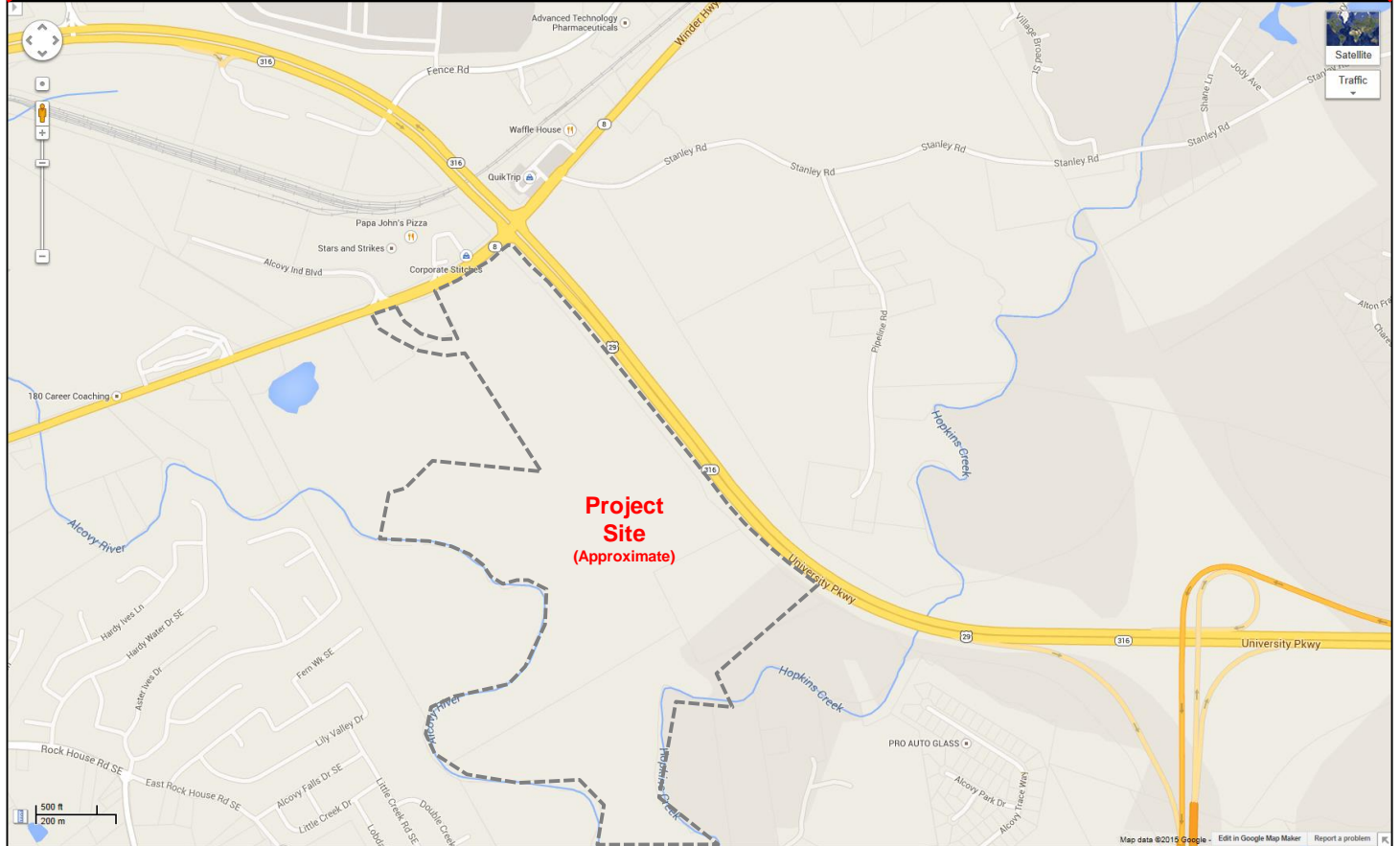
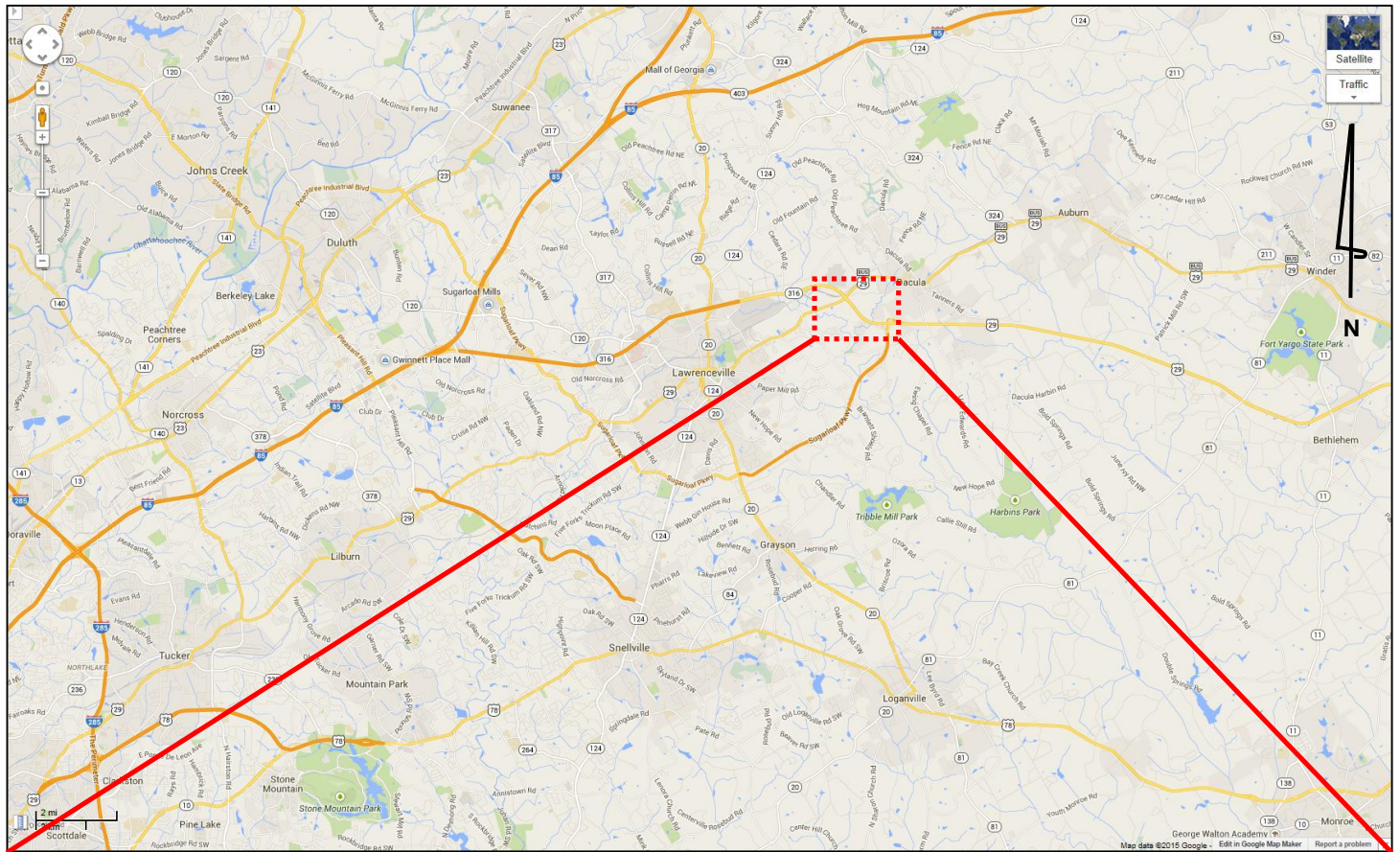
1.1 Introduction

This report presents the analysis of the anticipated traffic impacts of the Sugarloaf Crossing DRI mixed-use development located in Gwinnett County, Georgia, between the cities of Lawrenceville and Dacula. The approximate 159.87-acre site is bordered by SR 316 (University Parkway) to the northeast and by SR 8 / US 29 (Winder Highway) to the northwest. The project will exceed 500,000 square feet of mixed-use development in a Developing Suburbs area type, therefore, the proposed development is a Development of Regional Impact (DRI) and is subject to Georgia Regional Transportation Authority (GRTA) and Atlanta Regional Commission (ARC) review.

Figure 1 provides the site location of the Sugarloaf Crossing DRI project, and **Figure 2** provides an aerial view of the project site and surrounding area. **Figure 3** provides a zoomed-in aerial of the project site frontage along Winder Highway. Field review photographs taken within the vicinity of the study network are located in the site photo log in Appendix A. The Gwinnett County Existing Land Use Map, the Gwinnett County Future Development Map, and ARC's *PLAN 2040 Unified Growth Policy Map* are included in Appendix B.

The proposed project is expected to be completed by 2020, and this analysis will consider the full build-out of the proposed site in 2020. A summary of the proposed land-use and density can be found below in **Table 1**.

Table 1 Proposed Land Uses	
General Office	1,100,000 SF
Retail	275,000 SF







1.2 Site Plan Review

The proposed development is approximately a 159.87-acre site in Gwinnett County. The project site is bordered by SR 316 (University Parkway) to the northeast, by SR 8 / US 29 (Winder Highway) to the northwest, and roughly by the Alcovy River and Hopkins Creek to the southwest and southeast. The project will include eleven office buildings, one retail building, plus some formal green spaces as functional areas for office users.

The property is currently an undeveloped land tract which is zoned to the Agriculture-Residence District (RA-200) and General Business District (C-2) classifications. The zoning classification is proposed to be changed to Regional Mixed-Use District (MU-R). ARC's *PLAN 2040 Unified Growth Policy Map* identifies the project site as being in a Developing Suburbs area.

In the northern corner of the project site, right-of-way will be reserved for future SR 316 eastbound entrance and exit ramps. These ramps will serve the envisioned interchange along SR 316 at SR 8 / US 29 (Winder Highway).

A reference of the proposed site plan can be found in Appendix C. A full-sized site plan consistent with GRTA's Site Plan Guidelines is also being submitted as part of the review package.

1.3 Site Access

As currently envisioned, the proposed development will be served by one (1) full-movement driveway along SR 8 / US 29 (Winder Highway). The proposed site driveway will be the fourth leg of the existing unsignalized intersection of Winder Highway at Alcovy Industrial Boulevard.

The proposed site driveway on Winder Highway provides vehicular access to the entire development. Internal private roadways throughout the site provide access to all buildings and parking facilities. See the referenced site plan in Appendix C for a visual representation of vehicular access and circulation throughout the proposed development.

The site driveway and internal roadways mentioned above provide access to all parking on the site. Parking will be provided throughout the development as follows:

Parking Provided:	3,850 spaces total (1,100 for retail + 2,750 for office)
Parking Required:	3,850 spaces total

1.4 Bicycle and Pedestrian Facilities

Pedestrian facilities (sidewalks) and bicycle facilities do not currently exist along the project site frontage. Sidewalks currently exist along the northwest side of SR 8 / US 29 (Winder Highway) in the vicinity of project site (along the existing shopping center development frontage). There are currently no sidewalks along SR 316. There are currently no bicycle facilities (bike lanes/paths) in the vicinity of the project site. According to the DRI site plan, pedestrian facilities (sidewalks) are proposed along most roadways internal to the DRI project site, and trails are proposed throughout the DRI project site.

1.5 Transit Facilities

There are no direct transit routes located within the vicinity of the project site, and therefore, there were no alternative mode reductions taken.

2.0 TRAFFIC ANALYSES, METHODOLOGY AND ASSUMPTIONS

2.1 Growth Rate

Background traffic is defined as expected traffic on the roadway network in future year(s) absent the construction and opening of the proposed project. Background traffic can include a base growth rate based on historical count data as well as population growth data and estimates as well as trips anticipated from nearby or adjacent other projects. Based on methodology outlined in the GRTA Letter of Understanding (LOU), a 1.5 percent per year background traffic growth rate was used for all roadways. This background growth rate was used to account for other development activity in the area.

2.2 Traffic Data Collection

Weekday peak hour turning movement counts were collected on Tuesday, March 10, 2015 at ten (10) intersections during the AM and PM peak periods. The morning and afternoon peak hours varied some between the intersections. Peak hours for all intersections are shown in **Table 2**.

Table 2 Peak Hour Summary		
Intersection	AM Peak Hour	PM Peak Hour
1. SR 316 (University Pkwy) at Cedars Road	7:00-8:00	4:15-5:15
2. SR 316 (University Pkwy) at Hurricane Trail	6:45-7:45	4:15-5:15
3. SR 316 (University Pkwy) at SR 8 / US 29 (Winder Hwy)	6:45-7:45	4:45-5:45
4. SR 316 (University Pkwy) at Harbins Road	6:45-7:45	5:00-6:00
5. SR 8 / US 29 (Winder Hwy) at SR 124 (Scenic Hwy) / E Pike St	7:30-8:30	4:30-5:30
6. SR 8 / US 29 (Winder Hwy) at Sweet Gum Road	7:15-8:15	5:00-6:00
7. SR 8 / US 29 (Winder Hwy) at Cedars Road	6:45-7:45	5:00-6:00
8. SR 8 / US 29 (Winder Hwy) at Alcovy Industrial Blvd	7:30-8:30	4:45-5:45
9. SR 8 / Bus US 29 (Winder Hwy) at Circle Road	6:45-7:45	4:45-5:45
10. SR 8 / Bus US 29 (Winder Hwy) at Broad St / McMillan St	6:45-7:45	5:15-6:15

The collected peak hour turning movement traffic counts are available upon request.

2.3 Detailed Intersection Analysis

Level-of-service (LOS) is used to describe the operating characteristics of a road segment or intersection in relation to its capacity. LOS is defined as a qualitative measure that describes operational conditions and motorists' perceptions within a traffic stream. The *Highway Capacity Manual* defines six levels-of-service, LOS A through LOS F, with A being the best and F being the worst. Level-of-service analyses were conducted at all intersections within the study network using *Synchro Professional, Version 8.0*.

Existing traffic signal phasing and timing data (from current EPAC reports) was provided by Gwinnett County, and utilized in the *Synchro* model.

Levels-of-service for signalized intersections are reported for the intersection as a whole. One or more movements at an intersection may experience a low level-of-service, while the intersection as a whole may operate acceptably.

Levels-of-service for unsignalized intersections, with stop control on the minor street only, are reported for the side street approaches. Low levels-of-service for side street approaches are not uncommon, as vehicles may experience significant delays in turning onto a major roadway.

3.0 STUDY NETWORK

3.1 Gross Trip Generation

Traffic for the proposed land uses and densities were calculated using methodology contained in the *Institute of Transportation Engineers' (ITE) Trip Generation Manual, Ninth Edition*. Gross trips generated are displayed below in **Table 3**.

Table 3 Gross Trip Generation							
Land Use (Intensity)	ITE Code	Daily Traffic		AM Peak Hour		PM Peak Hour	
		Enter	Exit	Enter	Exit	Enter	Exit
General Office (1,100,000 SF)	710	4,061	4,061	1,147	156	223	1,087
Retail (275,000 SF)	820	6,554	6,554	179	110	566	614
Total Gross Trips		10,615	10,615	1,326	266	789	1,701

3.2 Trip Distribution

The directional distribution and assignment of new project trips was based on the project land uses, a review of the land use densities and road facilities in the area, engineering judgment, and methodology discussions with the Georgia Regional Transportation Authority (GRTA), Atlanta Regional Commission (ARC), Georgia Department of Transportation (GDOT), and Gwinnett County.

3.3 Level-of-Service Standards

For the purposes of this traffic analysis, a level-of-service standard of D was assumed for all intersections and segments within the study network. If, however, an intersection or segment currently operates at LOS E or LOS F during an existing peak period, the LOS standard for that peak period becomes LOS E, consistent with the GRTA Letter of Understanding.

3.4 Study Network Determination

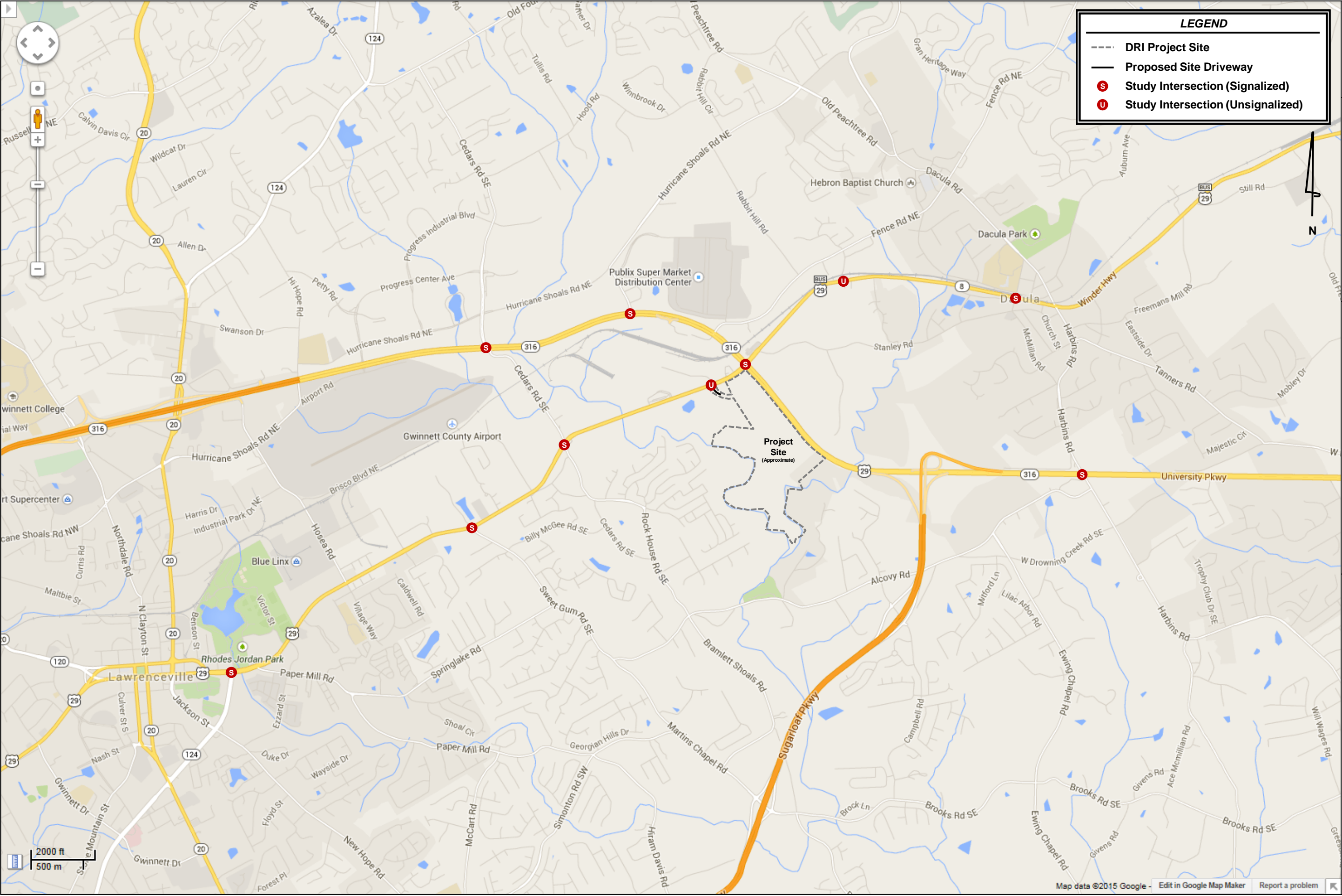
A general study area was determined using the GRTA 7% rule. This rule recommends that all intersections and segments be analyzed which are impacted to the extent that the traffic from the proposed site is 7% or more of the service volume of the facility (at a previously established LOS standard, typically LOS D) be considered for analysis. The study area was agreed upon during methodology discussions with GRTA, ARC, GDOT, and Gwinnett staff, and includes the following twelve (12) intersections in **Table 4**.

The study network includes eight (8) signalized intersections and two (2) two-way stop controlled intersections as noted in Table 4. The site location and study intersections can be found in **Figure 4**.

Table 4 Intersection Control Summary	
Intersection	Control
1. SR 316 (University Pkwy) at Cedars Road	Signal
2. SR 316 (University Pkwy) at Hurricane Trail	Signal
3. SR 316 (University Pkwy) at SR 8 / US 29 (Winder Hwy)	Signal
4. SR 316 (University Pkwy) at Harbins Road	Signal
5. SR 8 / US 29 (Winder Hwy) at SR 124 (Scenic Hwy) / E Pike St	Signal
6. SR 8 / US 29 (Winder Hwy) at Sweet Gum Road	Signal
7. SR 8 / US 29 (Winder Hwy) at Cedars Road	Signal
8. SR 8 / US 29 (Winder Hwy) at Alcovy Industrial Blvd	TWSC
9. SR 8 / Bus US 29 (Winder Hwy) at Circle Road	TWSC
10. SR 8 / Bus US 29 (Winder Hwy) at Broad St / McMillan St	Signal

*Note: TWSC = Two-Way Stop Control

Each of the above listed intersections was analyzed for the Existing 2015 conditions, the projected 2020 No-Build conditions, and the projected 2020 Build conditions. The projected 2020 No-Build conditions represent the existing traffic volumes grown for five (5) years at 1.5 percent per year throughout the study network. The projected 2020 Build conditions add the project trips associated with the Sugarloaf Crossing development to the projected 2020 No-Build conditions.



3.5 Existing Roadway Facilities

Roadway classification descriptions for the entire study area are provided in **Table 5** (bolded roadways run adjacent to the site).

Table 5 Roadway Classification			
Roadway	No. of Lanes	Posted Speed Limit (MPH)	Functional Classification
SR 8 / US 29 (Winder Hwy)	2-4	35-45	Minor Arterial
SR 316 (University Pkwy)	4	55-65	Principal Arterial
Alcovy Industrial Blvd	2	35	Local Road
Cedars Road	2-4	40	Local Road
Sweet Gum Road	2	45	Major Collector
SR 124 (Scenic Hwy)	4	35	Principal Arterial
Hurricane Trail	3	35	Local Road
Circle Road	2	25	Local Road
Broad Street / McMillan Road	2	25	Local Road
Harbins Road	2	45-50	Minor Arterial

4.0 TRIP GENERATION

As stated previously, gross trips associated with the proposed development were estimated using the *Institute of Transportation Engineers' (ITE) Trip Generation Manual, Ninth Edition, 2012*, using equations where available. Trip generation for this proposed development is calculated based upon the following land uses: General Office Building (ITE 710) and Shopping Center / Retail (ITE 820).

Mixed-use vehicle trip reductions were taken according to the *ITE Trip Generation Handbook, Third Edition, 2014*. Total internal capture and vehicle trip reduction between the land uses is expected to be 15.9% for the weekday, 9.5% for the AM peak hour, and 4.6% for the PM peak hour as a result of the anticipated interaction between the office and retail uses within the proposed development.

Alternative transportation mode (walking, bicycle, and transit) reductions were not applied for this study.

Pass-by reductions were determined according to the *ITE Trip Generation Handbook, Third edition, 2014*. Per ITE guidance, the pass-by trip reduction rate for the proposed retail land use is 34% for the PM peak hour. However, per GRTA's DRI Technical Guidelines, the total pass-by trips associated with the development may be limited to 15% of the adjacent roadway's traffic volume. Based on traffic count data collected in March 2015, 15% of the adjacent roadway's traffic volume is the limiting factor for pass-by trip reduction (results in a pass-by trip reduction rate of 13.6% for the PM peak hour). It should be noted that pass-by trips are not new trips to the roadway network, rather, they are vehicles already travelling along the existing roadway network that stop to visit the retail land uses. No pass-by reductions were taken for the AM peak hour as pass-by trips are minimal in the morning for retail land uses.

The total (net) trips generated and analyzed in this report are listed in **Table 6**.

Table 6 Net Trip Generation						
	Daily Traffic		AM Peak Hour		PM Peak Hour	
	Enter	Exit	Enter	Exit	Enter	Exit
Gross Project Trips	10,615	10,615	1,326	266	789	1,701
<i>Mixed-Use Reduction</i>	-1,686	-1,686	-76	-76	-57	-57
<i>Alternative More Reduction</i>	-0	-0	-0	-0	-0	-0
<i>Pass-By Reduction (Limited by GRTA 15% Rule)</i>	-836	-836	-0	-0	-77	-77
Net New Trips	8,093	8,093	1,250	190	655	1,567

A more detailed trip generation analysis summary table is provided in Appendix D.

5.0 TRIP DISTRIBUTION AND ASSIGNMENT

New trips were distributed onto the roadway network using the percentages developed as described in *Section 3.2* of this report, and as agreed to during methodology discussions with GRTA, ARC, GDOT, and Gwinnett County staff.

Figure 5 displays the anticipated distribution and assignment of project trips throughout the study roadway network. These trip assignment percentages were applied to the net new trips expected to be generated by the development, and the volumes were assigned to the roadway network. The combined peak hour project trips by turning movement throughout the study network, anticipated to be generated by the proposed Sugarloaf Crossing development, are shown on **Figure 6**.

Detailed intersection volume worksheets can also be found in Appendix E.

LEGEND

Turning Movement

Existing Traffic Signal

XX% Project Trip IN Assignments

(XX%) Project Trip OUT Assignments

Intersection Reference Number

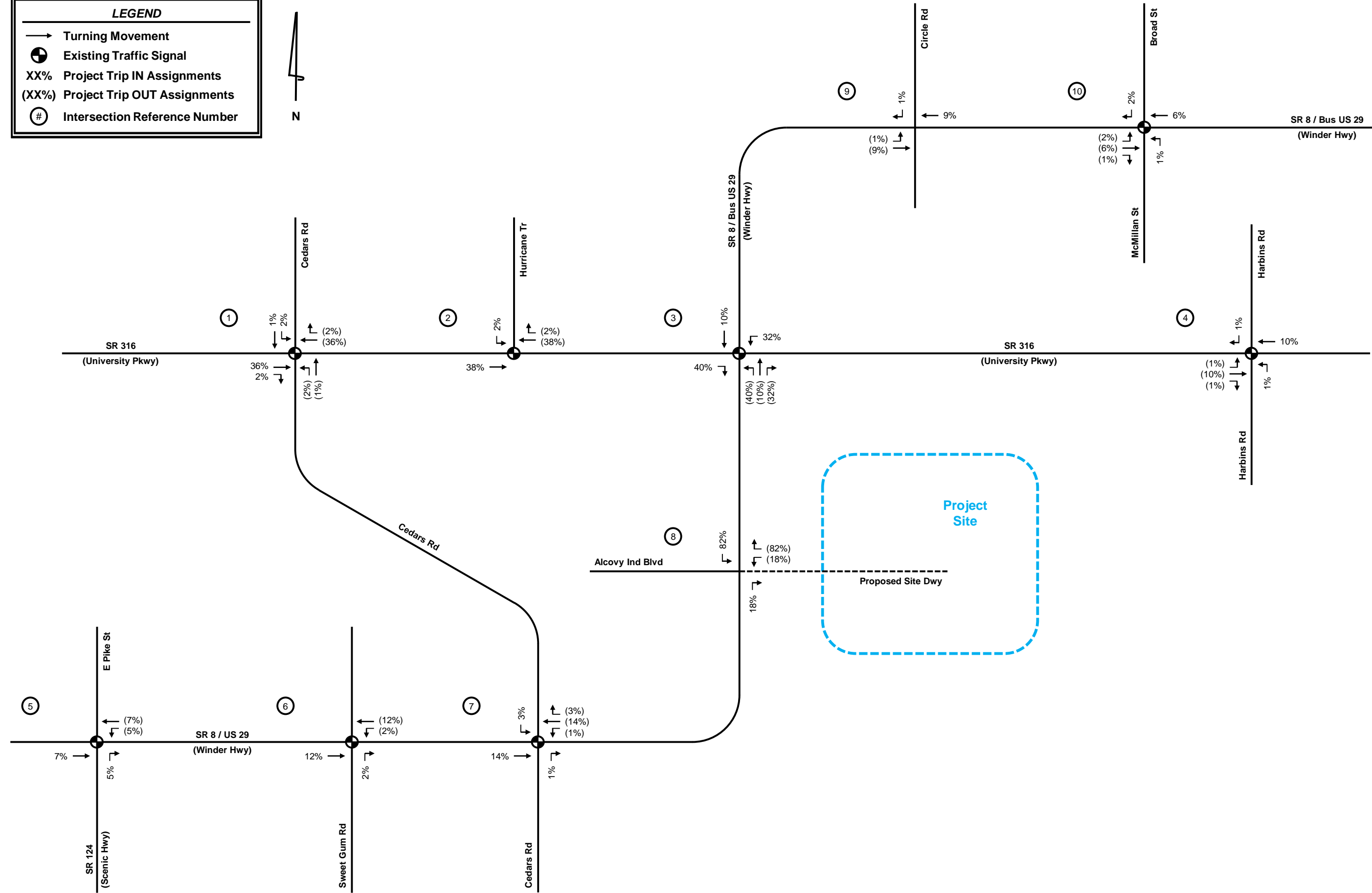


Figure 5

Project Trip Distribution & Assignment

Sugarloaf Crossing DRI #2472 Transportation Analysis

LEGEND

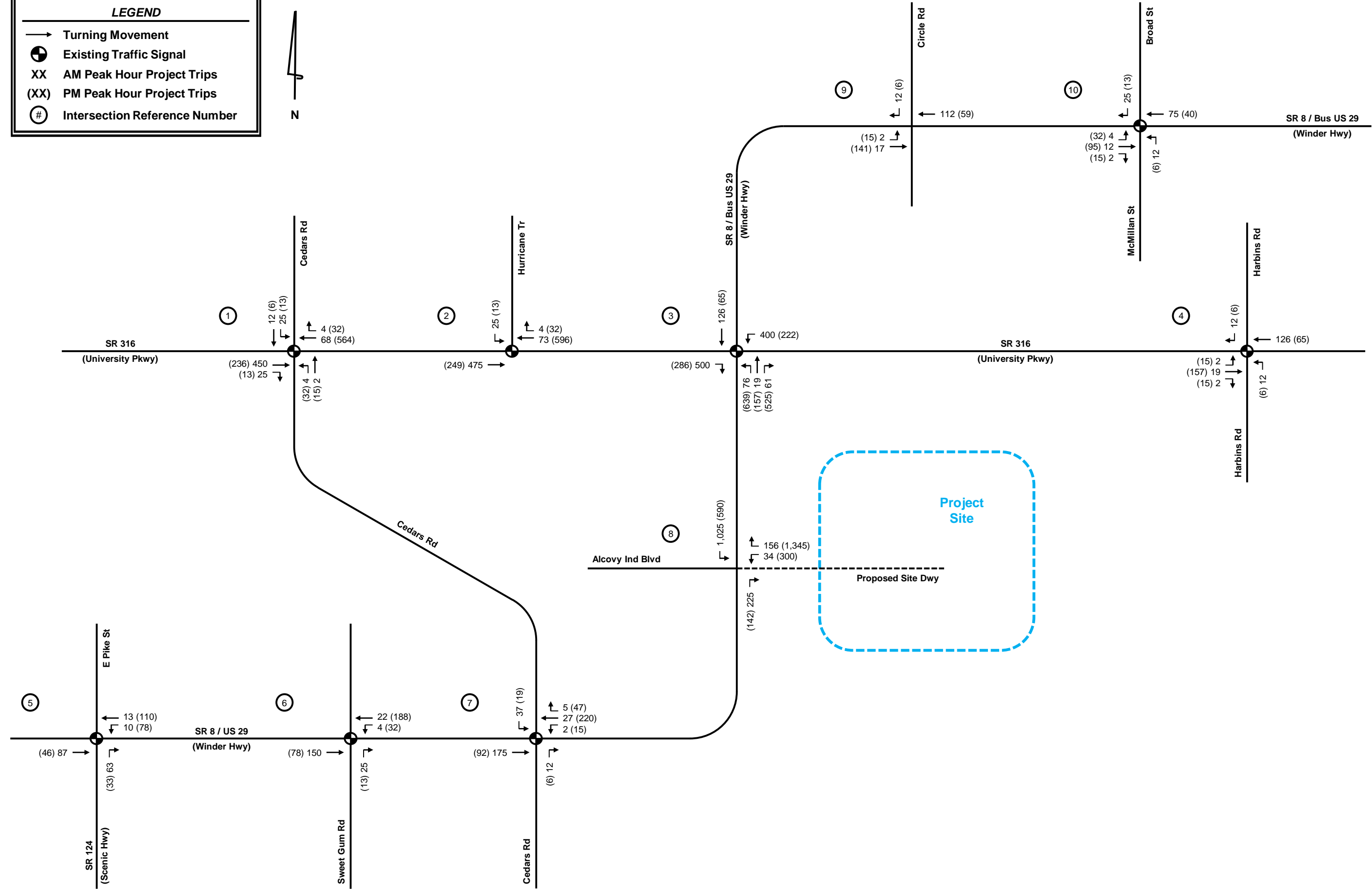
Turning Movement

Existing Traffic Signal

XX AM Peak Hour Project Trips

(XX) PM Peak Hour Project Trips

Intersection Reference Number



6.0 TRAFFIC ANALYSIS

6.1 Existing 2015 Conditions

The observed existing peak hour traffic volumes were entered into *Synchro 8.0*, and capacity analyses were performed for the AM and PM peak hours. The existing peak hour traffic volumes are displayed in **Figure 7**, and the results of the capacity analyses for the Existing 2015 conditions are shown in **Table 7**. Detailed *Synchro* analysis reports are available upon request.

Table 7 Existing 2015 Intersection Levels-of-Service LOS (delay in seconds)				
Intersection	Control	LOS Std.	AM Peak Hour	PM Peak Hour
1. SR 316 (University Pkwy) at Cedars Road	Signal	AM – D PM – E	D (37.5)	E (61.8)
2. SR 316 (University Pkwy) at Hurricane Trail	Signal	D	B (17.1)	B (11.6)
3. SR 316 (University Pkwy) at SR 8 / US 29 (Winder Hwy)	Signal	D	D (43.1)	D (48.7)
4. SR 316 (University Pkwy) at Harbins Road	Signal	AM – E PM – D	E (65.6)	D (47.0)
5. SR 8 / US 29 (Winder Hwy) at SR 124 (Scenic Hwy) / E Pike St	Signal	D	C (27.2)	D (38.9)
6. SR 8 / US 29 (Winder Hwy) at Sweet Gum Road	Signal	D	D (35.4)	B (14.7)
7. SR 8 / US 29 (Winder Hwy) at Cedars Road	Signal	D	C (25.2)	B (18.8)
8. SR 8 / US 29 (Winder Hwy) at Alcovy Industrial Blvd /	EB Stop NBL Yield	D	C (15.5) A (0.2)	C (19.6) A (0.0)
9. SR 8 / Bus US 29 (Winder Hwy) at Circle Road*	SB Stop EBL Yield	D	C (16.0) A (9.4)	C (15.0) A (8.1)
10. SR 8 / Bus US 29 (Winder Hwy) at Broad St / McMillan St	Signal	D	C (24.7)	C (21.9)

**Note: the southern leg of Circle Road at Winder Highway (Intersection #9) is gated closed, and no vehicles were observed on this leg, thus, LOS for the northbound approach is not reported.*

As shown in Table 7, all study intersections currently operate at or above their acceptable level-of-service standard during the AM and PM peak hours for the Existing 2015 conditions. Therefore, there are no recommended improvements for the Existing 2015 conditions scenario.

LEGEND

→

Existing Roadway Laneage

⊕

Existing Traffic Signal

STOP

Existing STOP Control

XX

AM Peak Hour Traffic Volumes

(XX)

PM Peak Hour Traffic Volumes

⊗

Intersection Reference Number

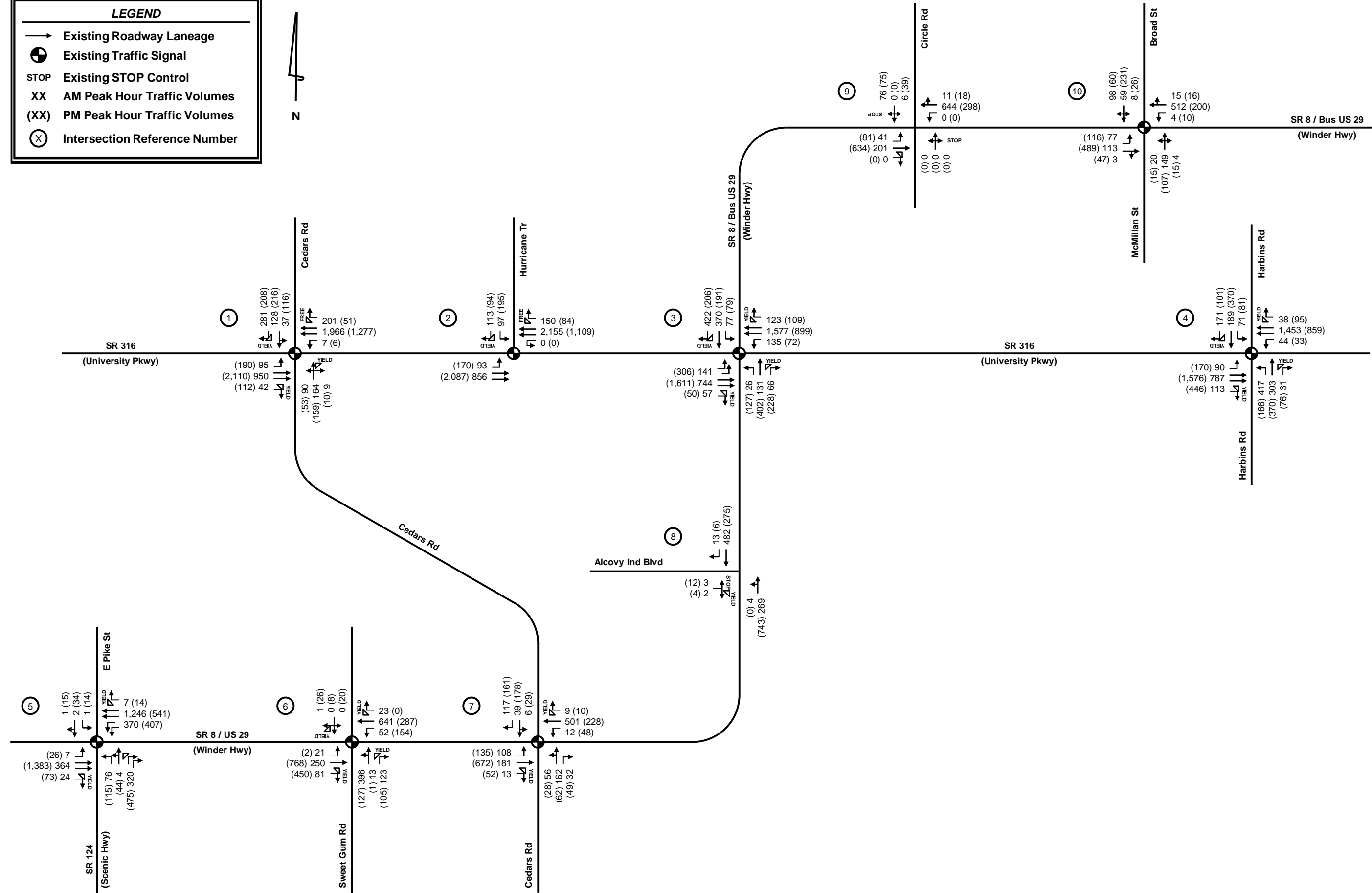


Figure 7

Existing 2015 Conditions

Sugarloaf Crossing
DRI #2472
Transportation Analysis

6.2 Projected 2020 No-Build Conditions

To account for growth in the vicinity of the proposed development, the existing traffic volumes were increased for five (5) years at 1.5 percent per year throughout the study network. These volumes were entered into *Synchro 8.0*, and capacity analyses were performed. The projected 2020 No-Build conditions were analyzed using existing roadway geometry and existing intersection control types.

The intersection laneage and traffic volumes for the projected 2020 No-Build conditions are shown in **Figure 8**. The results of the capacity analyses for the projected 2020 No-Build conditions with existing laneage and control types are shown in **Table 8**. Detailed *Synchro* analysis reports are available upon request.

Table 8 Projected 2020 No-Build Intersection Levels-of-Service LOS (delay in seconds)				
Intersection	Control	LOS Std.	AM Peak Hour	PM Peak Hour
1. SR 316 (University Pkwy) at Cedars Road	Signal	AM – D PM – E	E (56.6)	F (91.8)
2. SR 316 (University Pkwy) at Hurricane Trail	Signal	D	C (20.5)	B (12.5)
3. SR 316 (University Pkwy) at SR 8 / US 29 (Winder Hwy)	Signal	D	D (46.8)	E (56.7)
4. SR 316 (University Pkwy) at Harbins Road	Signal	AM – E PM – D	F (81.5)	E (62.1)
5. SR 8 / US 29 (Winder Hwy) at SR 124 (Scenic Hwy) / E Pike St	Signal	D	C (27.9)	D (42.0)
6. SR 8 / US 29 (Winder Hwy) at Sweet Gum Road	Signal	D	D (42.4)	B (16.8)
7. SR 8 / US 29 (Winder Hwy) at Cedars Road	Signal	D	C (25.9)	B (19.3)
8. SR 8 / US 29 (Winder Hwy) at Alcovy Industrial Blvd /	EB Stop NBL Yield	D	C (16.6) A (0.2)	C (22.1) A (0.0)
9. SR 8 / Bus US 29 (Winder Hwy) at Circle Road*	SB Stop EBL Yield	D	C (17.4) A (9.6)	C (16.3) A (8.2)
10. SR 8 / Bus US 29 (Winder Hwy) at Broad St / McMillan St	Signal	D	C (26.2)	C (23.1)

As shown in Table 8, three (3) intersections are projected to operate below their acceptable level-of-service standard during the AM Peak Hour and/or PM Peak Hour. Based on the projected 2020 No-Build conditions, the following improvements result in the below-listed intersections operating at or above their LOS standard:

- SR 316 (University Parkway) at Cedars Road (Int. #1)
 - Construct an exclusive left-turn lane along the Cedars Road northbound approach.
 - Construct an exclusive left-turn lane along the Cedars Road southbound approach.
- SR 316 (University Parkway) at SR 8 / US 29 (Winder Highway) (Int. #3)
 - Construct a second through lane along the Winder Highway northbound approach.
 - Construct a second through lane along the Winder Highway southbound approach.
- SR 316 (University Parkway) at Harbins Road (Int. #4)
 - Construct a second through lane along the Harbins Road northbound approach.
 - Construct a second through lane along the Harbins Road southbound approach.

The recommended roadway improvements are also illustrated on **Figure 8**. The results of the capacity analyses for the projected 2020 No-Build conditions with the roadway improvements stated above are shown in **Table 9**.

Table 9 Projected 2020 No-Build Intersection Levels-of-Service – IMPROVED <i>LOS (delay in seconds)</i>				
Intersection	Control	LOS Std.	AM Peak Hour	PM Peak Hour
1. SR 316 (University Pkwy) at Cedars Road	Signal	AM – D PM – E	C (31.4)	D (36.0)
3. SR 316 (University Pkwy) at SR 8 / US 29 (Winder Hwy)	Signal	D	D (49.0)	D (43.4)
4. SR 316 (University Pkwy) at Harbins Road	Signal	AM – E PM – D	E (64.3)	D (40.8)

LEGEND

Existing Roadway Laneage

Recommended Laneage for Improved No-Build Conditions

Existing Traffic Signal

STOP

Existing STOP Control

XX

AM Peak Hour Traffic Volumes

(XX)

PM Peak Hour Traffic Volumes

#

Intersection Reference Number

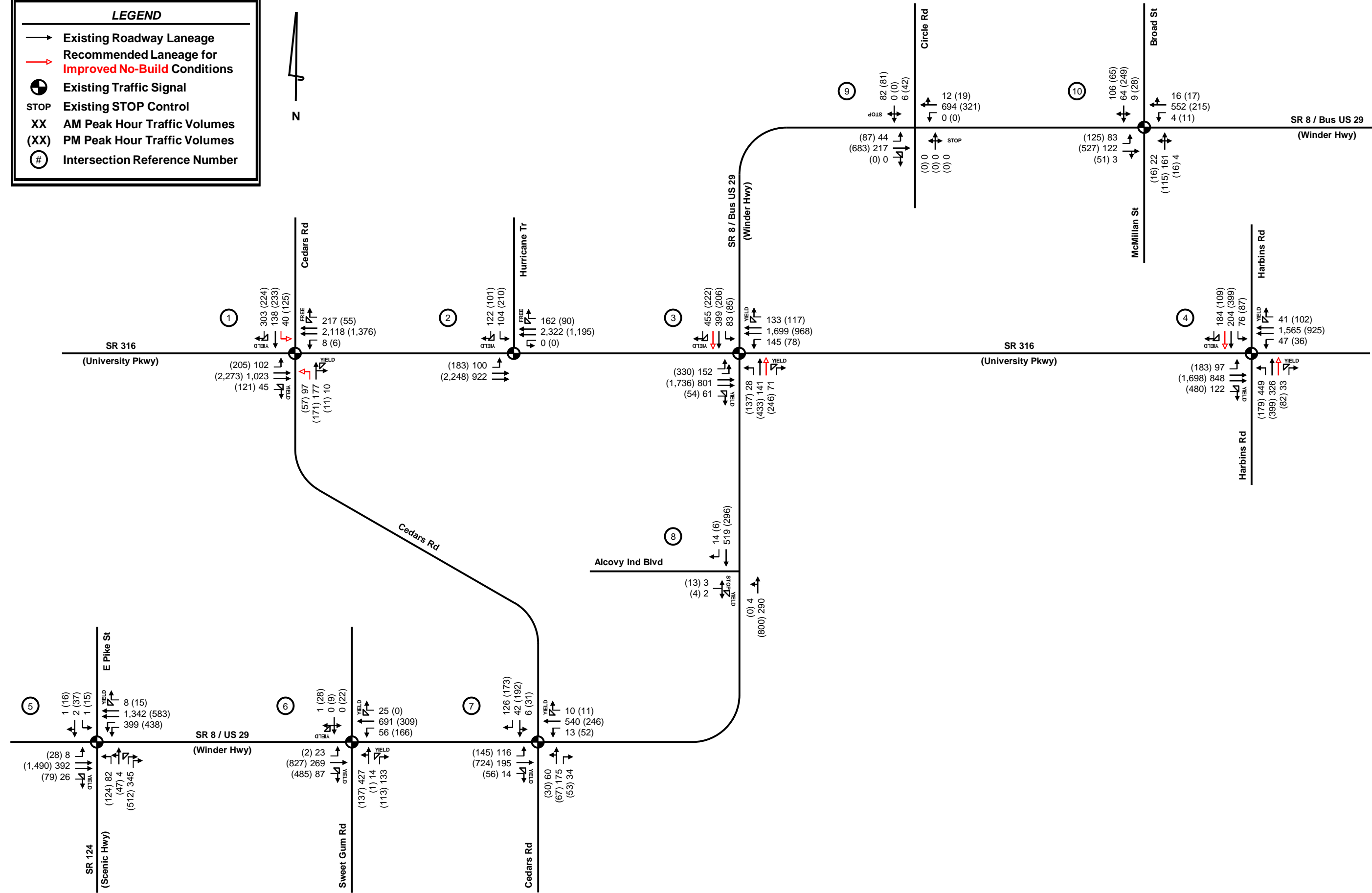


Figure 8

Projected 2020 No-Build Conditions

Sugarloaf Crossing DRI #2472 Transportation Analysis

6.3 Projected 2020 Build Conditions

The traffic associated with the proposed Sugarloaf Crossing DRI development was added to the projected 2020 No-Build volumes. These volumes were then entered into *Synchro* 8.0, and capacity analyses were performed. The projected 2020 Build conditions were analyzed using existing roadway geometry and existing intersection control types.

The intersection laneage and traffic volumes used for the projected 2020 Build conditions are shown in **Figure 9**. The results of the capacity analyses for the projected 2020 Build conditions with existing laneage and control types are shown in **Table 10**. Detailed *Synchro* analysis reports are available upon request.

Table 10 Projected 2020 Build Intersection Levels-of-Service LOS (delay in seconds)				
Intersection	Control	LOS Std.	AM Peak Hour	PM Peak Hour
1. SR 316 (University Pkwy) at Cedars Road	Signal	AM – D PM – E	E (79.7)	F (189.9)
2. SR 316 (University Pkwy) at Hurricane Trail	Signal	D	C (21.4)	B (19.5)
3. SR 316 (University Pkwy) at SR 8 / US 29 (Winder Hwy)	Signal	D	F (130.8)	F (226.9)
4. SR 316 (University Pkwy) at Harbins Road	Signal	AM – E PM – D	F (91.3)	E (73.4)
5. SR 8 / US 29 (Winder Hwy) at SR 124 (Scenic Hwy) / E Pike St	Signal	D	C (28.1)	D (45.0)
6. SR 8 / US 29 (Winder Hwy) at Sweet Gum Road	Signal	D	D (42.9)	B (19.9)
7. SR 8 / US 29 (Winder Hwy) at Cedars Road	Signal	D	C (24.5)	C (21.1)
8. SR 8 / US 29 (Winder Hwy) at Alcovy Industrial Blvd / Proposed Site Driveway	EB Stop WB Stop NBL Yield SBL Yield	D	F (Error) F (Error) A (0.1) F (93.0)	F (Error) F (Error) A (0.0) E (46.4)
9. SR 8 / Bus US 29 (Winder Hwy) at Circle Road*	SB Stop EBL Yield	D	C (21.6) B (10.2)	C (19.8) A (8.5)
10. SR 8 / Bus US 29 (Winder Hwy) at Broad St / McMillan St	Signal	D	C (31.2)	C (25.0)

* Error = excessive delays (outside *Synchro* limitations) due to stop control on minor street only with heavy major street volume.

As shown in Table 10, four (4) intersections are projected to operate below their acceptable level-of-service standard during the AM Peak Hour and/or PM Peak Hour. Following implementation of the improvements recommended in the projected 2020 No-Build conditions analysis, two (2) intersections are still projected to operate below their acceptable level-of-service standard during the AM Peak Hour and/or PM Peak Hour.

Based on the projected 2020 Build conditions, the following improvements result in the below-listed intersections operating at or above their LOS standard (please note that the following improvements are IN ADDITION TO the improvements needed under projected 2020 No-Build conditions (recommendations noted in *Section 6.2*):

- SR 316 (University Parkway) at SR 8 / US 29 (Winder Highway) (Int. #3)
 - Construct an interchange to eliminate at-grade crossings and related delays.
 - Provide four through lanes along SR 8 at the new interchange (four-lane bridge across SR 316).
 - Provide dual left-turn lanes and a single channelized right-turn lane along each of the SR 316 off-ramp approaches to SR 8.
 - Provide a single southbound left-turn lane and a single northbound channelized right-turn lane along the SR 8 approaches to each of the SR 316 on-ramps.

Or, alternatively (not recommended):

- Construct a third through lane along the SR 316 eastbound approach.
- Construct a third through lane along the SR 316 westbound approach.
- Construct one additional northbound left-turn lane along SR 8 to SR 316, resulting in dual left-turn lanes.
- Construct one additional westbound left-turn lane along SR 316 to SR 8, resulting in dual left-turn lanes.
- Convert the existing northbound channelized right-turn along SR 8 from yield-control to continuous free-flow. Construct an eastbound receiving lane along SR 316, departing from SR 8, to accommodate this free-flow right-turn implementation.
- Convert the existing southbound channelized right-turn along SR 8 from yield-control to continuous free-flow. Construct a westbound receiving lane along SR 316, departing from SR 8, to accommodate this free-flow right-turn implementation.

An interchange is recommended due to the significant roadway geometry (intersection laneage) improvements otherwise required at this intersection. An interchange (grade separation) was once identified as a TIA project, and has been suggested in the City of Dacula's Comprehensive Plan Update (2014).

- Roadway segment of SR 8 / US 29 (Winder Highway) between Alcovy Industrial Blvd. / Proposed Site Driveway (Int. #8) and SR 316 (University Parkway) (Int. #3)
 - Widen SR 8 between Alcovy Industrial Blvd. / Proposed Site Driveway and SR 316 from a 2-lane section to a 4-lane section, to serve the projected Sugarloaf Crossing DRI project traffic between its access point (site driveway) and SR 316.
 - At the southern end of this segment, the additional northbound through travel lane begins with the recommended westbound free-flow right-turn from the Proposed Site Driveway onto SR 8 (see details for Int. #8 below).

- SR 8 / US 29 (Winder Highway) at Alcovy Industrial Blvd. / Proposed Site Driveway (Int. #8)
 - Install a traffic signal (when warranted). It should be noted that for all requests for new traffic signals, an alternative solution that considers a roundabout is required to be investigated, per GDOT policy.
 - Construct an exclusive northbound left-turn lane along SR 8 to Alcovy Industrial Blvd.
 - Construct an exclusive northbound channelized, yield-controlled right-turn lane along SR 8 to the Proposed Site Driveway.
 - Construct exclusive dual southbound left-turn lanes along SR 8 to the Proposed Site Driveway, and convert the existing southbound shared left-turn/through lane to be a through-only lane.
 - Construct an exclusive eastbound left-turn lane along Alcovy Industrial Blvd to SR 8, and convert the existing approach lane to be a through-only lane (while preserving the channelized right-turn).
 - Construct the Proposed Site Driveway to have dual ingress lanes (to receive the recommended dual southbound left-turn lanes) and dual egress lanes.
 - Provide one exclusive left-turn lane, one through lane, and one channelized, continuous free-flow right-turn add lane along the Proposed Site Driveway westbound approach to SR 8. It is recommended that the right-turn lane be formed from the outside egress through travel lane exiting the site.

The recommended roadway improvements are also illustrated on **Figure 9**. The results of the capacity analyses for the project 2020 Build conditions with both the 2020 No-Build recommended improvements (Section 6.2) and the 2020 Build recommended improvements stated above are shown in **Table 11**.

Table 11 Projected 2020 Build Intersection Levels-of-Service – IMPROVED LOS (delay in seconds)				
Intersection	Control	LOS Std.	AM Peak Hour	PM Peak Hour
1. SR 316 (University Pkwy) at Cedars Road	Signal	AM – D PM – E	D (36.0)	E (71.8)
3. SR 316 (University Pkwy) EB Ramps at SR 8 / US 29 (Winder Hwy)*	Signal	D	C (33.6)*	C (31.4)*
3. SR 316 (University Pkwy) WB Ramps at SR 8 / US 29 (Winder Hwy)*	Signal	D	C (34.1)*	B (19.7)*
3. SR 316 (University Pkwy) at SR 8 / US 29 (Winder Hwy)**	Signal	D	D (48.8)**	D (49.7)**
4. SR 316 (University Pkwy) at Harbins Road	Signal	AM – E PM – D	E (75.2)	D (43.5)
8. SR 8 / US 29 (Winder Hwy) at Alcovy Industrial Blvd / Proposed Site Driveway	Signal	D	C (26.4)	D (46.3)

* Recommended solution: interchange with signalized ramp intersections

** Alternative solution: significant intersection laneage improvements

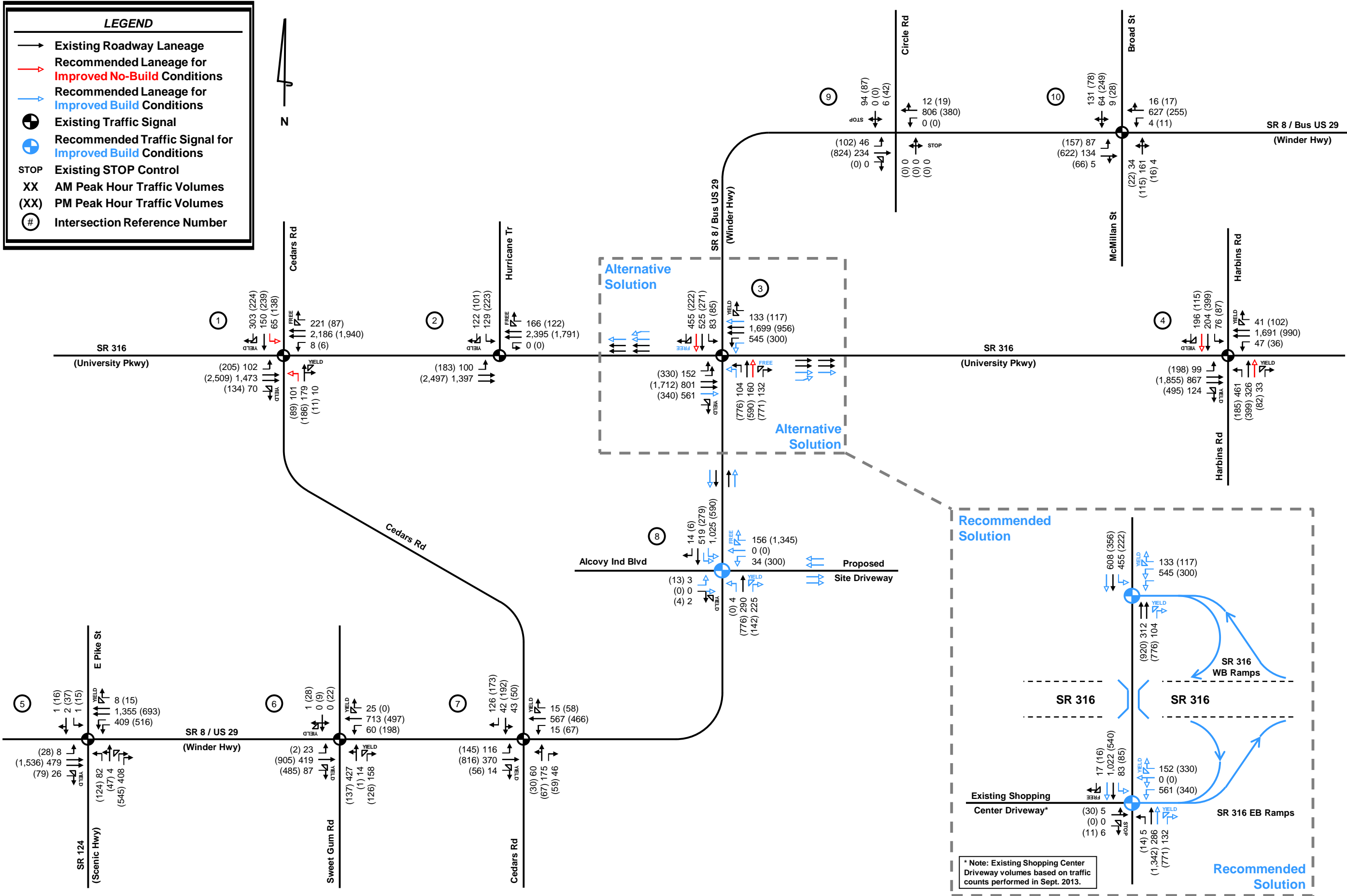


Figure 9

Projected 2020 Build Conditions

Sugarloaf Crossing DRI #2472 Transportation Analysis

7.0 INGRESS/EGRESS ANALYSIS

Vehicular access to the Sugarloaf Crossing DRI development is proposed at one (1) location. The one site driveway is proposed to tie into the existing T-intersection of SR 8 / US 29 (Winder Hwy) at Alcovy Industrial Blvd (Intersection #8) as the fourth, eastern leg to the intersection.

The proposed site driveway provides vehicular access to the entire development. Internal private roadways throughout the site provide access to all buildings and parking facilities.

Capacity analyses were performed for the Proposed Site Driveway intersection (Int. #8) using *Synchro* 8.0. The results of the capacity analyses for this intersection (LOS, delay, and recommended laneage) are reported in *Section 6.3* of this report. Based on the projected 2020 Build conditions, the Proposed Site Driveway intersection is anticipated to operate at an acceptable level-of-service, assuming implementation of the recommended laneage, signalization, and roadway improvements.

8.0 IDENTIFICATION OF PROGRAMMED PROJECTS

According to ARC's Transportation Improvement Program, the Regional Transportation Improvement Program, GDOT's Construction Work Program (none at this time), Gwinnett County's programmed projects, and the STIP, the following projects are programmed or planned to be completed by the respective years within the vicinity of the proposed development. The identified projects are listed in **Table 12** below.

Table 12 Programmed Improvements			
#	Year	Project ID	Project Description
1	2020	GW-342	Downtown Lawrenceville Pedestrian Improvements and One-Way Pair Conversion (Last Mile Connectivity / Complete Street Retrofit) – includes conversion of SR 20 (Clayton Street) and SR 20 (Perry Street) to two-way operation.
2	2020	GW-390D	Gwinnett County ATMS/ITS Infrastructure Expansion along SR 316 from SR 20 Interchange to the Barrow County Line (Apalachee River) – includes interconnecting traffic signals.
3	2030	GW-364	SR 20 (Buford Drive) Widening from SR 124 (Braselton Highway) to Hurricane Shoals Road (4 lanes to 6 lanes); 1.2 miles.
4	2040	GW-308B	Sugarloaf Parkway Extension: Phase 2 – New four-lane alignment from SR 316 east of Lawrenceville to SR 20 (Buford Drive / Mall of Georgia Parkway) near intersection with SR 324 (Gravel Springs Road); 8.5 miles.
5	2040+	ASP-AR-ML-440	SR 316 Managed Lanes from I-85 to High Hope Road; 8.1 miles.
6	2040+	ASP-AR-ML-450	SR 316 Managed Lanes from High Hope Road to SR 81; 13.3 miles.
7	2040+	ASP-GW-369	US 29 (Winder Highway) Widening from SR 124 (Scenic Highway) to SR 316 (2 lanes to 4 lanes); 3.6 miles.
8	2040+	ASP-GW-370	US 29 (Winder Highway) Widening from SR 316 to Apalachee Church Road (2 lanes to 4 lanes); 5.1 miles.
9	2040+	ASP-GW-361	SR 124 (Braselton Highway) Widening from SR 20 (Buford Drive) to Hamilton Mill Road (2 lanes to 4 lanes); 7.6 miles.
10	2040+	ASP-AR-441	Innovation Crescent Commuter Rail Service from Downtown Atlanta Multimodal Center to Lawrenceville.
11	2040+	ASP-AR-442	Innovation Crescent Commuter Rail Service Extension from Lawrenceville to Athens.

Please refer to **Figure 10** for an overview map of the above programmed improvement projects' locations within the vicinity of the proposed development. Fact sheets for projects 1-4 can be found in **Appendix F**.

Additionally, an interchange (grade separation) at the intersection of SR 316 (University Parkway) at SR 8 / US 29 (Winder Highway) (Int. #3) was once identified as a TIA project, and has been suggested in the City of Dacula's Comprehensive Plan Update (2014).

9.0 INTERNAL CIRCULATION ANALYSIS

Internal roadways throughout the site provide vehicular access to all buildings and parking on the site. The proposed site driveway on Winder Highway will provide access to buildings on the northern portion of the site, and will terminate at a proposed roundabout, which connects additional internal roadways to provide access to the other buildings and parking areas. A detailed copy of the proposed site plan with internal site roadways can be found in Appendix C and a full-sized site plan is attached to the report.

Mixed-use vehicle trip reductions were taken according to the *ITE Trip Generation Handbook, Third Edition, 2014*. Total internal capture and vehicle trip reduction between the proposed land uses is expected to be 15.9% for the weekday, 9.5% for the AM peak hour, and 4.6% for the PM peak hour as a result of the anticipated interaction between the various land uses within the proposed development.

10.0 COMPLIANCE WITH COMPREHENSIVE PLAN ANALYSIS

The property is currently an undeveloped land tract which is zoned to the Agriculture-Residence District (RA-200) and General Business District (C-2) classifications, according to the Sugarloaf Crossing site plan. The proposed development will require a rezoning with Gwinnett County and is proposed to be changed to Regional Mixed-Use District (MU-R).

The Gwinnett County 2030 Unified Plan Future Development Map identifies the project site as being located in an R & D Corridor character area type. The ARC PLAN 2040 Unified Growth Policy Map identifies the project site as being located in a Developing Suburbs area type and a Regionally Important Resources place type. The Sugarloaf Crossing development plan appears to be consistent with the area type and future land use identified. The land use maps can be found in Appendix B.

Appendix A

Site Photo Log

Sugarloaf Crossing DRI #2472

Photo No. 1



Comments: SR 8 (Winder Highway) northbound approach to approximate location of future SR 316 (University Parkway) interchange ramp (if an interchange were to be constructed). Photo looking to the southwest.

Photo No. 2



Comments: SR 8 (Winder Highway) southbound approach to approximate location of future SR 316 (University Parkway) interchange ramp (if an interchange were to be constructed). Photo looking to the northeast.

Sugarloaf Crossing DRI #2472

Photo No. 3



Comments: SR 8 (Winder Highway) northbound approach to approximate location of proposed site driveway. Photo looking to the southwest.

Photo No. 4



Comments: SR 8 (Winder Highway) southbound approach to approximate location of proposed site driveway. Photo looking to the northeast.

Sugarloaf Crossing DRI #2472

Photo No. 5



Comments: Alcovy Industrial Boulevard eastbound approach to SR 8 (Winder Highway). Photo looking to the northwest.

Photo No. 6

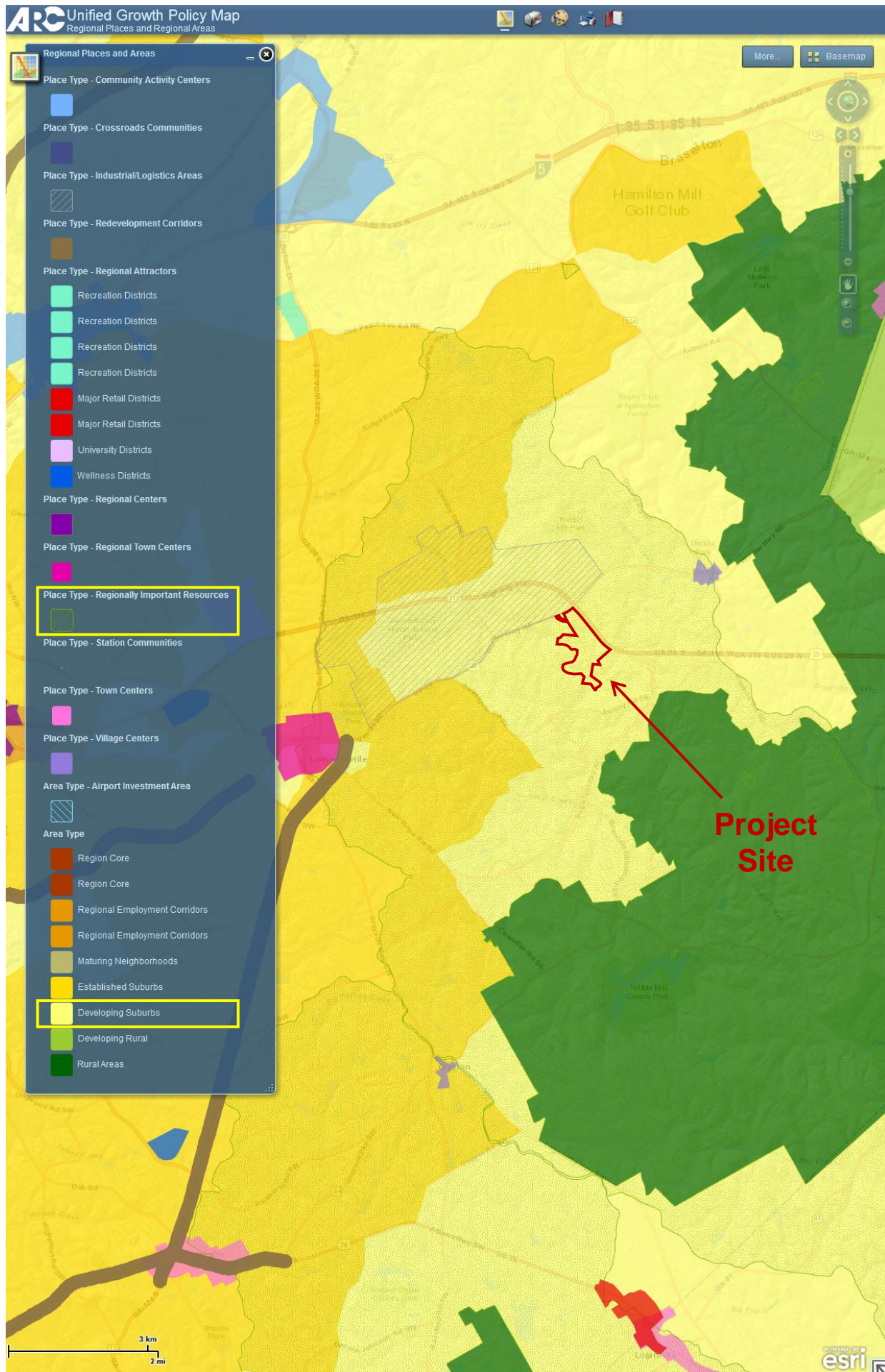


Comments: SR 8 (Winder Highway) southbound departure from SR 316 (University Parkway). Photo looking to the southwest.

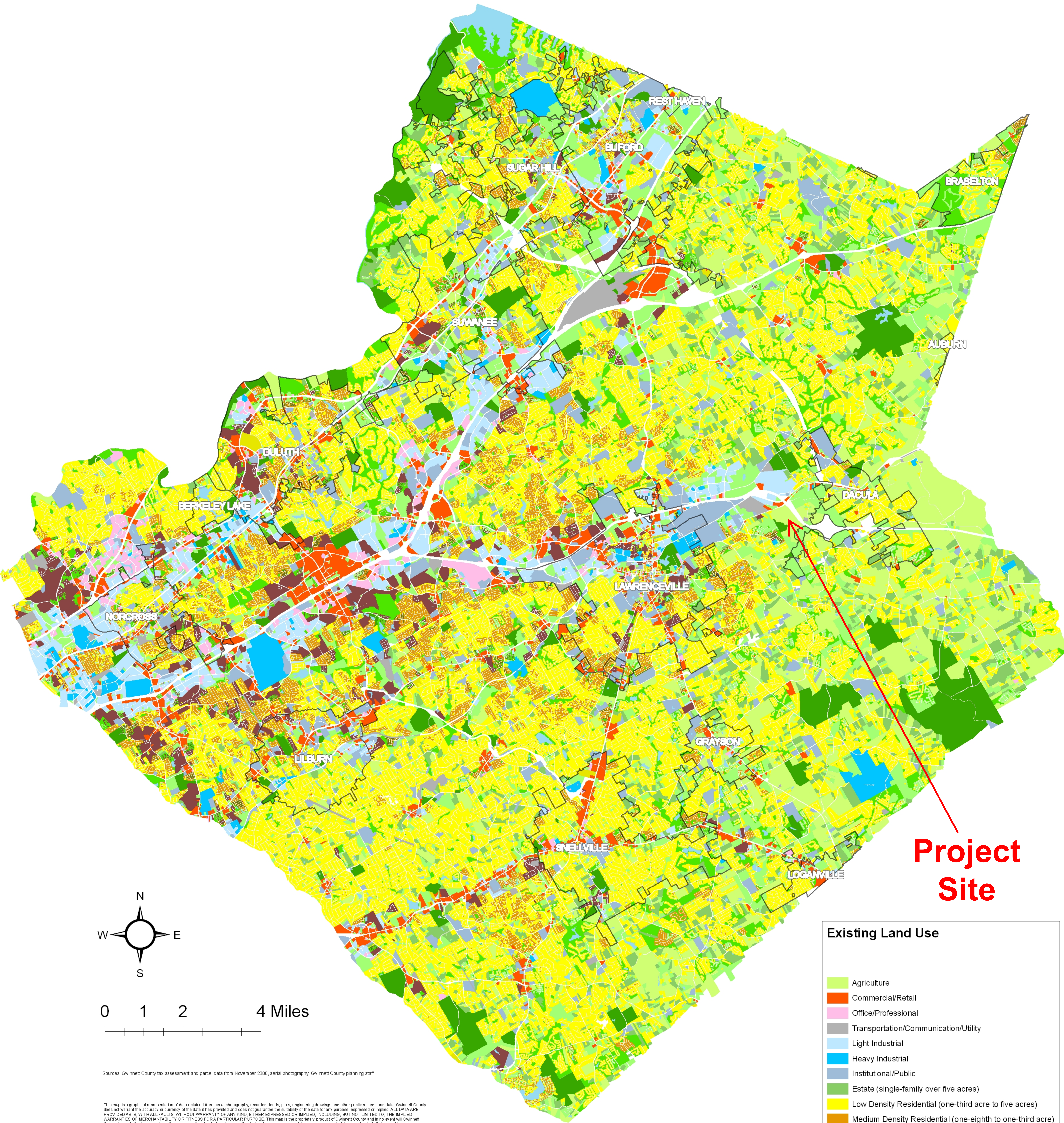
Appendix B

Land Use and Zoning Maps

ARC Unified Growth Policy Map



2009 Existing Land Use in Gwinnett County



**Project
Site**

Existing Land Use	
	Agriculture
	Commercial/Retail
	Office/Professional
	Transportation/Communication/Utility
	Light Industrial
	Heavy Industrial
	Institutional/Public
	Estate (single-family over five acres)
	Low Density Residential (one-third acre to five acres)
	Medium Density Residential (one-eighth to one-third acre)
	High Density Residential (eight or more units/acre)
	Mixed Use
	Multifamily Dwellings (includes mobile home parks)
	Park (government parkland)
	Park/Recreation/Conservation
	Undeveloped
	Right of Way
	Water
	Municipalities

Sources: Gwinnett County tax assessment and parcel data from November 2008, aerial photography, Gwinnett County planning staff

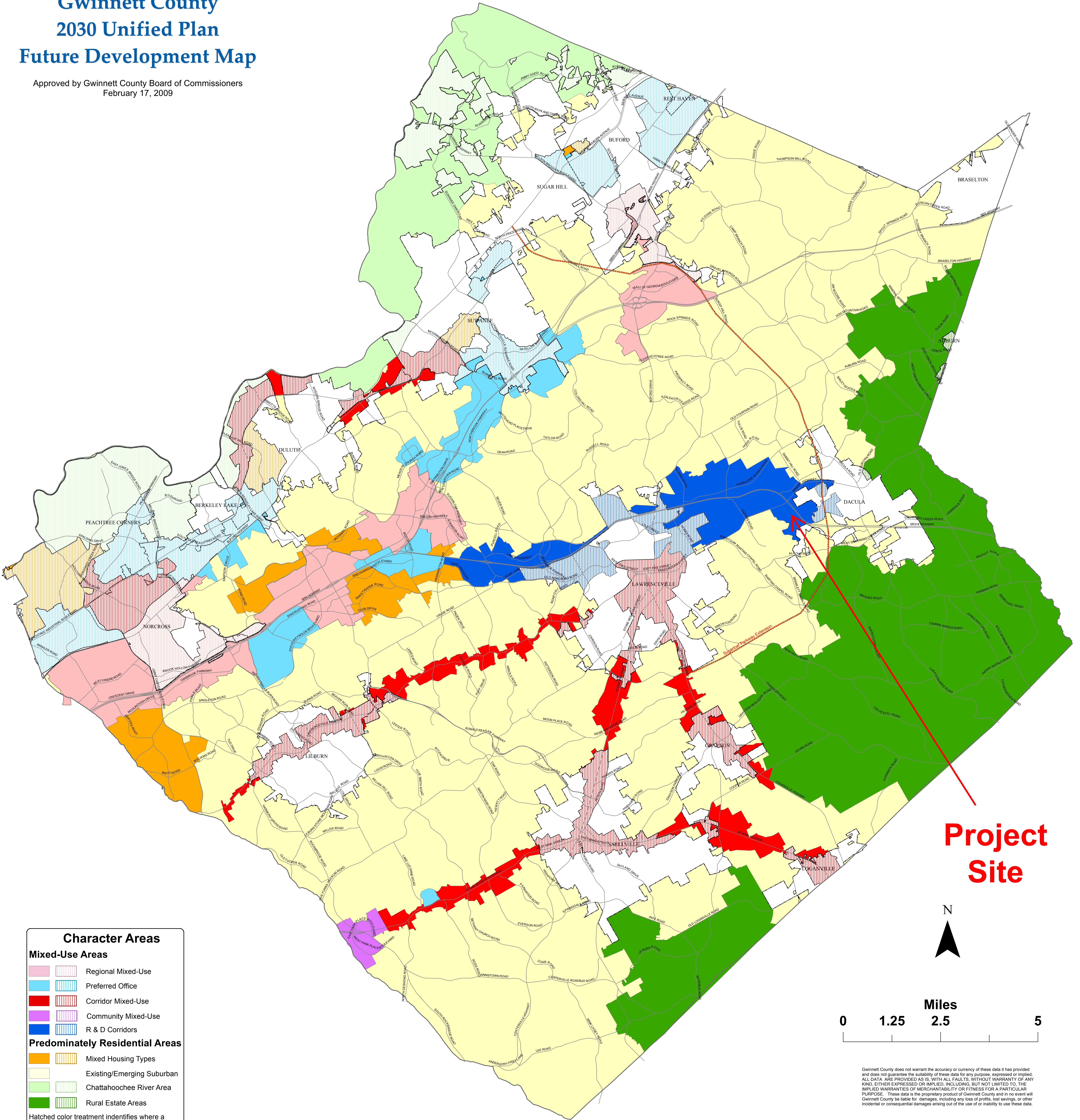
This map is a graphical representation of data obtained from aerial photography, recorded deeds, plats, engineering drawings and other public records and data. Gwinnett County does not warrant the accuracy or currency of the data it has provided and does not guarantee the suitability of the data for any purpose, expressed or implied. ALL DATA ARE PROVIDED AS IS, WITH ALL FAULTS, WITHOUT WARRANTY OF ANY KIND, EITHER EXPRESSED OR IMPLIED, INCLUDING, BUT NOT LIMITED TO, THE IMPLIED WARRANTIES OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE. This map is the proprietary product of Gwinnett County and in no event will Gwinnett County be liable for damages, including any loss of profits, lost savings, or other incidental or consequential damages arising out of the use of or inability to use this map.

The parcels in some zoning districts have been classified as Low Density Residential regardless of actual lot size. Districts with suffixes CSO, MOD, or CLU are required to average three units or less per acre.

Gwinnett County Department of Planning & Development
Printed July 2009

Gwinnett County 2030 Unified Plan Future Development Map

Approved by Gwinnett County Board of Commissioners
February 17, 2009



This map shows those areas that will be most subject to major policy interventions to achieve Unified Plan goals.

Gwinnett County Department of Planning and Development
Long Range Planning Section
Planning Data Services Section
Date Printed: February 2009

Gwinnett County - Functional Classification Review

Project Site

Legend

2013 Functional Class

Interstate (FC-1)

Other Frwy and Expr (FC-2)

Other Principal Arterials (FC-3)

Minor Arterials (FC-4)

Major Collectors (FC-5)

Minor Collectors (FC-6)

Local Roads (FC-7)

Regional Thoroughfares Network

Proposed FC Changes

From FC-4 to FC-3

From FC-3 to FC-4

Appendix C

Proposed Site Plan

- LEGEND
- ATF - ANGLE IRON FOUND
 - BLA - BEGIN LIMITED ACCESS
 - CLP - CLAMP TOP PIPE
 - CMF - CENTERLINE
 - CMF - CONCRETE MONUMENT FOUND
 - DB PG - DEED BOOK, PAGE
 - ELEV - ELEVATION
 - FR - FIRE HYDRANT
 - GA - GAS LINE
 - GV - GAS VALVE
 - GW - GUY WIRE
 - HWF - HOG WIRE FENCE
 - IPF - IRON PIN FOUND
 - IPS - IRON PIN SET
 - 1/2" - 1/2" REBAR
 - WITH YELLOW PLASTIC CAP
 - STAMPED RAI LSF000484
 - LL - LAND LOT
 - LLL - LAND LOT LINE
 - LP - LIGHT POLE
 - N/F - NOW OR FORMERLY
 - OB - OUTBUILDING
 - OH - OVERHEAD
 - OTP - OPEN TOP PIPE
 - P - P
 - P/T - POWER & TELEPHONE LINE
 - PB PG - PLAT BOOK, PAGE
 - PBX - POWER BOX
 - PL - PROPERTY LINE
 - PB - POINT OF BEGINNING
 - PP - POWER POLE
 - PTP - POWER & TELEPHONE POLE
 - R/W - RIGHT OF WAY
 - TSB - TELEPHONE BOX
 - TMH - TELEPHONE MAINHOLE
 - TPB - TRUE POINT OF BEGINNING
 - TSB - TRAFFIC SIGNAL BOX
 - TSR - TRAFFIC SIGNAL POLE
 - TS - TELEPHONE LINE
 - W - WOOD FENCE
 - WM - WATER METER
 - WV - WATER VALVE
 - YR - YEAR
 - III- - LIMITED ACCESS

INTERSECTION 3
EXISTING LANEAGE
EXISTING TRAFFIC SIGNAL

GEORGIA HIGHWAY 316
U.S. HIGHWAY 19
UNIVERSITY PARKWAY
R/W VARIES

MAINTENANCE
EASEMENT
DB 5167, PG 93

University Parkway- Hwy 316

Proposed DOT
Right of Way

Assumed alignment of future
on and off ramp for SR 316

Nature and amenity trails winding
through the trees and along creeks

Use parking decks to take up the slope
and create podiums for office towers

Focal office building at roundabout

Roundabout as
focal feature and
for traffic calming

Create formal green spaces as functional
areas for office users

Buildings grouped in areas to work with topography
Each office tower to be built with it's own parking to
allow phasing of development

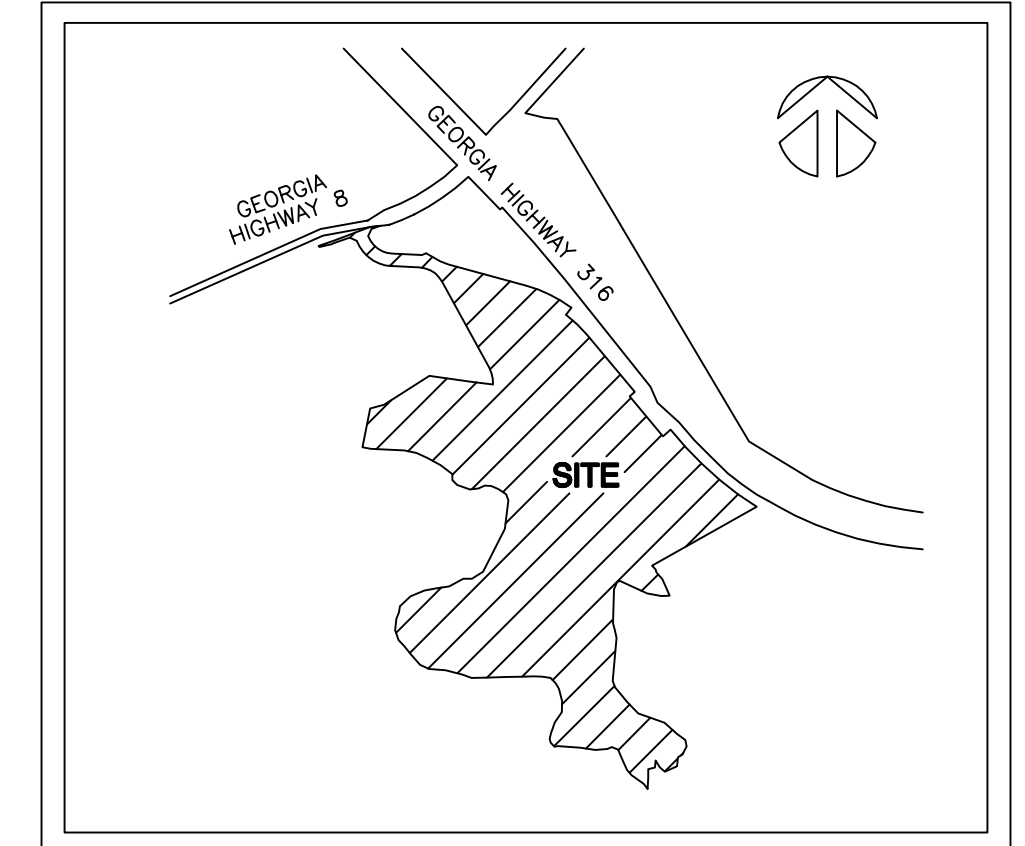
PLAN KEY

- (A) PROPERTY LINE
- (B) PROPOSED CURB & GUTTER
- (C) PROPOSED 4' SIDEWALK
- (D) PROPOSED TRAIL
- (E) 75' IMPERVIOUS BUFFER
- (F) 50' UNDISTURBED BUFFER

INTERSECTION 8
PROPOSED LANEAGE
PROPOSED TRAFFIC SIGNAL
(IF WARRANTED)

VICINITY MAP

SCALE: NTS



THIS PROPERTY IS NOT LOCATED WITHIN A FLOOD HAZARD
ZONE "AE" AS PER FEMA FLOOD INSURANCE RATE MAP OF
GWINNETT COUNTY, GEORGIA AS SHOWN ON MAP NO. S
13135C0075F, 13135C0076F, EFFECTIVE DATE: SEPTEMBER
29, 2006.

NOTES:

1. TOTAL SITE AREA: 159.871 ACRES
2. ZONING:
CURRENT ZONING: RA 200 & C-2 (GWINNETT COUNTY)
PROPOSED ZONING: MU-R (GWINNETT COUNTY)
3. UTILITY PROVIDERS:
A. WATER - GWINNETT COUNTY
B. SANITARY SEWER - GWINNETT COUNTY
C. GAS - ATLANTIC GAS LIGHT
D. TELEPHONE/CABLE/INTERNET - CHARTER/AT&T
E. POWER - JACKSON EMC
4. SITE IS IN UNINCORPORATED GWINNETT COUNTY

GENERAL INFORMATION:

APPLICANT: WALTON DEVELOPMENT & MANAGEMENT (USA), INC.
295 WEST CROSSVILLE ROAD
BUILDING 700, SUITE 710
ROSWELL, GEORGIA 30075
PHONE: 770.642.7750
CONTACT: MR. RICK SHMURAK, PE
PHONE: 770.642.7750

ENGINEER: ROCHESTER & ASSOCIATES, INC.
425 OAK STREET
GAINESVILLE, GA. 30501
PHONE: (770) 718-0600

TRAFFIC ENGINEER: KIMLEY-HORN AND ASSOCIATES, INC.
2 SUN COURT, SUITE 450
PEACHTREE CORNERS, GA. 30092
CONTACT: JOHN WALKER, P.E., PTOE
PHONE: (770) 825-0744

LAND USE:

PROPOSED USES:
RETAIL 275,000 SF
OFFICE 1,100,000 SF
FLOOR AREA RATIO: 1,375,000 SF / 6,963,981 SF = 20%
MAXIMUM NUMBER OF STORIES IS 12.

OPEN SPACE:
OPEN SPACE/GREEN SPACE 35.2 ACRES (22%)

PARKING SUMMARY:

SPACES PROVIDED: RETAIL - 1,100
OFFICE - 2,750

SPACES REQUIRED: 3,850 SPACES

BUILDING LETTER	BUILDING USE	NUMBER OF STORIES	SQUARE FEET/ UNITS
A	RETAIL	2 (MAX.)	150,000 S.F.
B	RETAIL	2 (MAX.)	125,000 S.F.
C	OFFICE	12 (MAX.)	120,000 S.F.
D	OFFICE	12 (MAX.)	120,000 S.F.
E	OFFICE	12 (MAX.)	120,000 S.F.
F	OFFICE	12 (MAX.)	120,000 S.F.
G	OFFICE	12 (MAX.)	120,000 S.F.
H	OFFICE	12 (MAX.)	120,000 S.F.
I	OFFICE	12 (MAX.)	120,000 S.F.
J	OFFICE	12 (MAX.)	120,000 S.F.
K	OFFICE	12 (MAX.)	140,000 S.F.

OWNER & DEVELOPER:

WUSF 2 SUGARLOAF LLC
4800 N SCOTTS DALE RD.
SUITE 4000
SCOTTS DALE, AZ 85251

DATE: 6/23/15

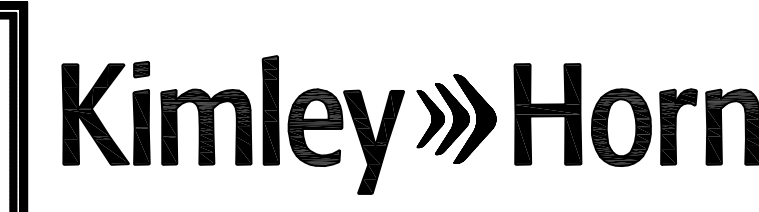
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100' 0' 200' 400'

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FILE NO.: 14-718 HC.dwg
DRAWN BY: SWS

SUGARLOAF CROSSING GWINNETT COUNTY, GEORGIA

DRI # 2472 - Plan

This plan is conceptual in nature and subject to change without notice.



Appendix D

Trip Generation Analysis

Trip Generation Analysis (9th Ed.) Surgarloaf Crossing DRI Gwinnett County, Georgia								
Land Use	Intensity	Daily Trips	AM Peak Hour			PM Peak Hour		
			Total	In	Out	Total	In	Out
Proposed Site Traffic								
710 General Office Building	1,100,000 s.f.	8,122	1,303	1,147	156	1,310	223	1,087
820 Shopping Center / Retail	275,000 s.f.	13,107	289	179	110	1,180	566	614
Gross Trips		21,229	1,592	1,326	266	2,490	789	1,701
Office Trips		8,122	1,303	1,147	156	1,310	223	1,087
Mixed-Use Reductions		-1,686	-76	-32	-44	-57	-12	-45
Alternative Mode Reductions		0	0	0	0	0	0	0
Adjusted Office Trips		6,436	1,227	1,115	112	1,253	211	1,042
Retail Trips		13,107	289	179	110	1,180	566	614
Mixed-Use Reductions		-1,686	-76	-44	-32	-57	-45	-12
Alternative Mode Reductions		0	0	0	0	0	0	0
Pass By Reductions (Limited by GRTA 15% Rule)		-1,671	0	0	0	-153	-77	-77
Adjusted Retail Trips		9,750	213	135	78	970	444	525
Mixed-Use Reductions - TOTAL		-3,372	-152	-76	-76	-114	-57	-57
Alternative Mode Reductions - TOTAL		0	0	0	0	0	0	0
Pass-By Reductions - TOTAL		-1,671	0	0	0	-153	-77	-77
New Trips		16,186	1,440	1,250	190	2,223	655	1,567
Driveway Volumes		17,857	1,440	1,250	190	2,376	732	1,644

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Appendix E

Intersection Volume Worksheets

INTERSECTION VOLUME DEVELOPMENT

SR 316 (University Parkway) at Cedars Road AM PEAK HOUR

Description	Cedars Rd <u>Northbound</u>			Cedars Rd <u>Southbound</u>			SR 316 (University Pkwy) <u>Eastbound</u>			SR 316 (University Pkwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	90	164	9	37	128	281	95	950	42	7	1,966	201
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	5%	2%	3%	5%	14%	2%	14%	4%	3%
Peak Hour Factor	0.88			0.93			0.89			0.93		
Adjustment												
Adjusted 2015 Volumes	90	164	9	37	128	281	95	950	42	7	1966	201
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	97	177	10	40	138	303	102	1,023	45	8	2,118	217
Project Trips												
Trip Distribution IN				2%	1%			36%	2%			
Trip Distribution OUT	2%	1%									36%	2%
Office Trips	2	1	0	22	11	0	0	401	22	0	40	2
Trip Distribution IN				2%	1%			36%	2%			
Trip Distribution OUT	2%	1%									36%	2%
Retail Trips	2	1	0	3	1	0	0	49	3	0	28	2
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	4	2	0	25	12	0	0	450	25	0	68	4
2020 Buildout Total	101	179	10	65	150	303	102	1,473	70	8	2,186	221

PM PEAK HOUR

Description	Cedars Rd <u>Northbound</u>			Cedars Rd <u>Southbound</u>			SR 316 (University Pkwy) <u>Eastbound</u>			SR 316 (University Pkwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	53	159	10	116	216	208	190	2,110	112	6	1,277	51
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	3%	2%	5%	4%	3%	2%	2%	7%	8%
Peak Hour Factor	0.91			0.88			0.94			0.85		
Adjustment												
Adjusted 2015 Volumes	53	159	10	116	216	208	190	2110	112	6	1277	51
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	57	171	11	125	233	224	205	2,273	121	6	1,376	55
Project Trips												
Trip Distribution IN				2%	1%			36%	2%			
Trip Distribution OUT	2%	1%									36%	2%
Office Trips	21	10	0	4	2	0	0	76	4	0	375	21
Trip Distribution IN				2%	1%			36%	2%			
Trip Distribution OUT	2%	1%									36%	2%
Retail Trips	11	5	0	9	4	0	0	160	9	0	189	11
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	32	15	0	13	6	0	0	236	13	0	564	32
2020 Buildout Total	89	186	11	138	239	224	205	2,509	134	6	1,940	87

INTERSECTION VOLUME DEVELOPMENT

SR 316 (University Parkway) at Hurricane Trail AM PEAK HOUR

Description	N/A Northbound			Hurricane Tr Southbound			SR 316 (University Pkwy) Eastbound			SR 316 (University Pkwy) Westbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes				97		113	93	856			2,155	150
Pedestrians		0			0			0			0	
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %				18%		25%	29%	10%			3%	5%
Peak Hour Factor					0.83			0.92			0.97	
Adjustment												
Adjusted 2015 Volumes	0	0	0	97	0	113	93	856	0	0	2155	150
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	0	0	0	104	0	122	100	922	0	0	2,322	162
Project Trips												
Trip Distribution IN				2%				38%				
Trip Distribution OUT										38%	2%	
Office Trips	0	0	0	22	0	0	0	424	0	0	43	2
Trip Distribution IN				2%				38%				
Trip Distribution OUT										38%	2%	
Retail Trips	0	0	0	3	0	0	0	51	0	0	30	2
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	0	25	0	0	0	475	0	0	73	4
2020 Buildout Total	0	0	0	129	0	122	100	1,397	0	0	2,395	166

PM PEAK HOUR

Description	N/A Northbound			Hurricane Tr Southbound			SR 316 (University Pkwy) Eastbound			SR 316 (University Pkwy) Westbound		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes				195		94	170	2,087			1,109	84
Pedestrians		0			0			0			0	
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %				8%		19%	15%	3%			8%	16%
Peak Hour Factor					0.94			0.97			0.91	
Adjustment												
Adjusted 2015 Volumes	0	0	0	195	0	94	170	2087	0	0	1109	84
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	0	0	0	210	0	101	183	2,248	0	0	1,195	90
Project Trips												
Trip Distribution IN				2%				38%				
Trip Distribution OUT										38%	2%	
Office Trips	0	0	0	4	0	0	0	80	0	0	396	21
Trip Distribution IN				2%				38%				
Trip Distribution OUT										38%	2%	
Retail Trips	0	0	0	9	0	0	0	169	0	0	200	11
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	0	13	0	0	0	249	0	0	596	32
2020 Buildout Total	0	0	0	223	0	101	183	2,497	0	0	1,791	122

INTERSECTION VOLUME DEVELOPMENT

SR 316 (University Parkway) at SR 8 / US 29 (Winder Highway) AM PEAK HOUR

Description	SR 8 / US 29 (Winder Hwy)			SR 8 / US 29 (Winder Hwy)			SR 316 (University Pkwy)			SR 316 (University Pkwy)		
	<u>Northbound</u>			<u>Southbound</u>			<u>Eastbound</u>			<u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	26	131	66	77	370	422	141	744	57	135	1,577	123
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	27%	2%	18%	9%	2%	4%	17%	10%	18%	2%	3%	4%
Peak Hour Factor	0.74			0.95			0.87			0.96		
Adjustment												
Adjusted 2015 Volumes	26	131	66	77	370	422	141	744	57	135	1577	123
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	28	141	71	83	399	455	152	801	61	145	1,699	133
Project Trips												
Trip Distribution IN					10%				40%	32%		
Trip Distribution OUT	40%	10%	32%									
Office Trips	45	11	36	0	112	0	0	0	446	357	0	0
Trip Distribution IN					10%				40%	32%		
Trip Distribution OUT	40%	10%	32%									
Retail Trips	31	8	25	0	14	0	0	0	54	43	0	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	76	19	61	0	126	0	0	0	500	400	0	0
2020 Buildout Total	104	160	132	83	525	455	152	801	561	545	1,699	133

PM PEAK HOUR

Description	SR 8 / US 29 (Winder Hwy)			SR 8 / US 29 (Winder Hwy)			SR 316 (University Pkwy)			SR 316 (University Pkwy)		
	<u>Northbound</u>			<u>Southbound</u>			<u>Eastbound</u>			<u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	127	402	228	79	191	206	306	1,611	50	72	899	109
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	4%	2%	3%	2%	4%	8%	4%	3%	10%	3%	4%	5%
Peak Hour Factor	0.87			0.92			0.98			0.92		
Adjustment												
Adjusted 2015 Volumes	127	402	228	79	191	206	306	1611	50	72	899	109
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	137	433	246	85	206	222	330	1,736	54	78	968	117
Project Trips												
Trip Distribution IN					10%				40%	32%		
Trip Distribution OUT	40%	10%	32%									
Office Trips	417	104	333	0	21	0	0	0	84	68	0	0
Trip Distribution IN					10%				40%	32%		
Trip Distribution OUT	40%	10%	32%									
Retail Trips	210	53	168	0	44	0	0	0	178	142	0	0
Pass-By Trips	12	0	24	0	0	0	0	-24	24	12	-12	0
Total Project Trips	639	157	525	0	65	0	0	-24	286	222	-12	0
2020 Buildout Total	776	590	771	85	271	222	330	1,712	340	300	956	117

INTERSECTION VOLUME DEVELOPMENT

SR 316 (University Parkway) at Harbins Road AM PEAK HOUR

Description	Harbins Rd <u>Northbound</u>			Harbins Rd <u>Southbound</u>			SR 316 (University Pkwy) <u>Eastbound</u>			SR 316 (University Pkwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	417	303	31	71	189	171	90	787	113	44	1,453	38
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	3%	7%	3%	7%	2%	2%	11%	4%	2%	3%	3%
Peak Hour Factor	0.92			0.83			0.93			0.91		
Adjustment												
Adjusted 2015 Volumes	417	303	31	71	189	171	90	787	113	44	1453	38
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	449	326	33	76	204	184	97	848	122	47	1,565	41
Project Trips												
Trip Distribution IN	1%					1%					10%	
Trip Distribution OUT							1%	10%	1%			
Office Trips	11	0	0	0	0	11	1	11	1	0	112	0
Trip Distribution IN	1%					1%					10%	
Trip Distribution OUT							1%	10%	1%			
Retail Trips	1	0	0	0	0	1	1	8	1	0	14	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	12	0	0	0	0	12	2	19	2	0	126	0
2020 Buildout Total	461	326	33	76	204	196	99	867	124	47	1,691	41

PM PEAK HOUR

Description	Harbins Rd <u>Northbound</u>			Harbins Rd <u>Southbound</u>			SR 316 (University Pkwy) <u>Eastbound</u>			SR 316 (University Pkwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	166	370	76	81	370	101	170	1,576	446	33	859	95
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	3%	2%	2%	2%	3%	2%	2%	2%	3%	2%	5%	2%
Peak Hour Factor	0.87			0.78			0.94			0.91		
Adjustment												
Adjusted 2015 Volumes	166	370	76	81	370	101	170	1576	446	33	859	95
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	179	399	82	87	399	109	183	1,698	480	36	925	102
Project Trips												
Trip Distribution IN	1%					1%					10%	
Trip Distribution OUT							1%	10%	1%			
Office Trips	2	0	0	0	0	2	10	104	10	0	21	0
Trip Distribution IN	1%					1%					10%	
Trip Distribution OUT							1%	10%	1%			
Retail Trips	4	0	0	0	0	4	5	53	5	0	44	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	6	0	0	0	0	6	15	157	15	0	65	0
2020 Buildout Total	185	399	82	87	399	115	198	1,855	495	36	990	102

INTERSECTION VOLUME DEVELOPMENT

SR 8 / US 29 (E Crogan Street) at SR 124 (Scenic Highway) / E Pike Street AM PEAK HOUR

Description	SR 124 (Scenic Hwy) <u>Northbound</u>			E Pike St <u>Southbound</u>			SR 8 / US 29 (E Crogan St) <u>Eastbound</u>			SR 8 / US 29 (E Crogan St) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	76	4	320	1	2	1	7	364	24	370	1,246	7
Pedestrians	0			1			0			0		
Conflicting Pedestrians	0		0	0		0	1		0	0		1
Heavy Vehicle %	12%	2%	7%	2%	2%	2%	2%	5%	13%	6%	2%	2%
Peak Hour Factor	0.91			0.25			0.78			0.96		
Adjustment												
Adjusted 2015 Volumes	76	4	320	1	2	1	7	364	24	370	1246	7
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	82	4	345	1	2	1	8	392	26	399	1,342	8
Project Trips												
Trip Distribution IN			5%					7%				
Trip Distribution OUT										5%	7%	
Office Trips	0	0	56	0	0	0	0	78	0	6	8	0
Trip Distribution IN			5%					7%				
Trip Distribution OUT										5%	7%	
Retail Trips	0	0	7	0	0	0	0	9	0	4	5	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	63	0	0	0	0	87	0	10	13	0
2020 Buildout Total	82	4	408	1	2	1	8	479	26	409	1,355	8

PM PEAK HOUR

Description	SR 124 (Scenic Hwy) <u>Northbound</u>			E Pike St <u>Southbound</u>			SR 8 / US 29 (E Crogan St) <u>Eastbound</u>			SR 8 / US 29 (E Crogan St) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	115	44	475	14	34	15	26	1,383	73	407	541	14
Pedestrians	3			1			0			1		
Conflicting Pedestrians	0		1	1		0	1		3	3		1
Heavy Vehicle %	7%	2%	3%	7%	2%	7%	4%	2%	6%	3%	2%	14%
Peak Hour Factor	0.97			0.80			0.94			0.91		
Adjustment												
Adjusted 2015 Volumes	115	44	475	14	34	15	26	1383	73	407	541	14
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	124	47	512	15	37	16	28	1,490	79	438	583	15
Project Trips												
Trip Distribution IN			5%					7%				
Trip Distribution OUT										5%	7%	
Office Trips	0	0	11	0	0	0	0	15	0	52	73	0
Trip Distribution IN			5%					7%				
Trip Distribution OUT										5%	7%	
Retail Trips	0	0	22	0	0	0	0	31	0	26	37	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	33	0	0	0	0	46	0	78	110	0
2020 Buildout Total	124	47	545	15	37	16	28	1,536	79	516	693	15

INTERSECTION VOLUME DEVELOPMENT

SR 8 / US 29 (Winder Highway) at Sweet Gum Road AM PEAK HOUR

Description	Sweet Gum Rd <u>Northbound</u>			Sweet Gum Rd <u>Southbound</u>			SR 8 / US 29 (Winder Hwy) <u>Eastbound</u>			SR 8 / US 29 (Winder Hwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	396	13	123	0	0	1	21	250	81	52	641	23
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	13%	17%	2%	3%	2%
Peak Hour Factor	0.81			0.25			0.87			0.87		
Adjustment												
Adjusted 2015 Volumes	396	13	123	0	0	1	21	250	81	52	641	23
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	427	14	133	0	0	1	23	269	87	56	691	25
Project Trips												
Trip Distribution IN			2%					12%				
Trip Distribution OUT										2%	12%	
Office Trips	0	0	22	0	0	0	0	134	0	2	13	0
Trip Distribution IN			2%					12%				
Trip Distribution OUT										2%	12%	
Retail Trips	0	0	3	0	0	0	0	16	0	2	9	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	25	0	0	0	0	150	0	4	22	0
2020 Buildout Total	427	14	158	0	0	1	23	419	87	60	713	25

PM PEAK HOUR

Description	Sweet Gum Rd <u>Northbound</u>			Sweet Gum Rd <u>Southbound</u>			SR 8 / US 29 (Winder Hwy) <u>Eastbound</u>			SR 8 / US 29 (Winder Hwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	127	1	105	20	8	26	2	768	450	154	287	0
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	2%	2%	4%	2%	2%	2%	2%	2%	2%
Peak Hour Factor	0.93			0.52			0.86			0.93		
Adjustment												
Adjusted 2015 Volumes	127	1	105	20	8	26	2	768	450	154	287	0
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	137	1	113	22	9	28	2	827	485	166	309	0
Project Trips												
Trip Distribution IN			2%					12%				
Trip Distribution OUT										2%	12%	
Office Trips	0	0	4	0	0	0	0	25	0	21	125	0
Trip Distribution IN			2%					12%				
Trip Distribution OUT										2%	12%	
Retail Trips	0	0	9	0	0	0	0	53	0	11	63	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	13	0	0	0	0	78	0	32	188	0
2020 Buildout Total	137	1	126	22	9	28	2	905	485	198	497	0

INTERSECTION VOLUME DEVELOPMENT

SR 8 / US 29 (Winder Highway) at Cedars Road AM PEAK HOUR

Description	Cedars Rd <u>Northbound</u>			Cedars Rd <u>Southbound</u>			SR 8 / US 29 (Winder Hwy) <u>Eastbound</u>			SR 8 / US 29 (Winder Hwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	56	162	32	6	39	117	108	181	13	12	501	9
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	5%	2%	3%	17%	8%	2%	3%	9%	8%	2%	4%	11%
Peak Hour Factor	0.88			0.75			0.87			0.95		
Adjustment												
Adjusted 2015 Volumes	56	162	32	6	39	117	108	181	13	12	501	9
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	60	175	34	6	42	126	116	195	14	13	540	10
Project Trips												
Trip Distribution IN			1%	3%				14%				
Trip Distribution OUT										1%	14%	3%
Office Trips	0	0	11	33	0	0	0	156	0	1	16	3
Trip Distribution IN			1%	3%				14%				
Trip Distribution OUT										1%	14%	3%
Retail Trips	0	0	1	4	0	0	0	19	0	1	11	2
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	12	37	0	0	0	175	0	2	27	5
2020 Buildout Total	60	175	46	43	42	126	116	370	14	15	567	15

PM PEAK HOUR

Description	Cedars Rd <u>Northbound</u>			Cedars Rd <u>Southbound</u>			SR 8 / US 29 (Winder Hwy) <u>Eastbound</u>			SR 8 / US 29 (Winder Hwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	28	62	49	29	178	161	135	672	52	48	228	10
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	3%	2%	2%	2%	2%	2%	2%	3%	30%
Peak Hour Factor	0.85			0.90			0.91			0.88		
Adjustment												
Adjusted 2015 Volumes	28	62	49	29	178	161	135	672	52	48	228	10
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	30	67	53	31	192	173	145	724	56	52	246	11
Project Trips												
Trip Distribution IN			1%	3%				14%				
Trip Distribution OUT										1%	14%	3%
Office Trips	0	0	2	6	0	0	0	30	0	10	146	31
Trip Distribution IN			1%	3%				14%				
Trip Distribution OUT										1%	14%	3%
Retail Trips	0	0	4	13	0	0	0	62	0	5	74	16
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	6	19	0	0	0	92	0	15	220	47
2020 Buildout Total	30	67	59	50	192	173	145	816	56	67	466	58

INTERSECTION VOLUME DEVELOPMENT

SR 8 / US 29 (Winder Highway) at Alcovy Industrial Boulevard / Proposed Site Driveway AM PEAK HOUR

Description	SR 8 / US 29 (Winder Hwy) <u>Northbound</u>			SR 8 / US 29 (Winder Hwy) <u>Southbound</u>			Alcovy Ind Blvd <u>Eastbound</u>			Proposed Site Dwy <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	4	269			482	13	3		2			
Pedestrians		0			0			0			0	
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	12%	2%	2%	5%	2%	50%	2%	2%	2%	2%	2%
Peak Hour Factor		0.88			0.91			0.42			0.88	
Adjustment												
Adjusted 2015 Volumes	4	269	0	0	482	13	3	0	2	0	0	0
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	4	290	0	0	519	14	3	0	2	0	0	0
Project Trips												
Trip Distribution IN			18%	82%								
Trip Distribution OUT										18%		82%
Office Trips	0	0	201	914	0	0	0	0	0	20	0	92
Trip Distribution IN			18%	82%								
Trip Distribution OUT										18%		82%
Retail Trips	0	0	24	111	0	0	0	0	0	14	0	64
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	225	1,025	0	0	0	0	0	34	0	156
2020 Buildout Total	4	290	225	1,025	519	14	3	0	2	34	0	156

Note: Assumed PHF = 0.88 for Proposed Site Driveway.

PM PEAK HOUR

Description	SR 8 / US 29 (Winder Hwy) <u>Northbound</u>			SR 8 / US 29 (Winder Hwy) <u>Southbound</u>			Alcovy Ind Blvd <u>Eastbound</u>			Proposed Site Dwy <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	0	743			275	6	12		4			
Pedestrians		0			0			0			0	
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	2%	3%	50%	2%	2%	2%	2%	2%	2%
Peak Hour Factor		0.91			0.91			0.67			0.88	
Adjustment												
Adjusted 2015 Volumes	0	743	0	0	275	6	12	0	4	0	0	0
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	0	800	0	0	296	6	13	0	4	0	0	0
Project Trips												
Trip Distribution IN			18%	82%								
Trip Distribution OUT										18%		82%
Office Trips	0	0	38	173	0	0	0	0	0	188	0	854
Trip Distribution IN			18%	82%								
Trip Distribution OUT										18%		82%
Retail Trips	0	0	80	364	0	0	0	0	0	95	0	431
Pass-By Trips	0	-24	24	53	-17	0	0	0	0	17	0	60
Total Project Trips	0	-24	142	590	-17	0	0	0	0	300	0	1,345
2020 Buildout Total	0	776	142	590	279	6	13	0	4	300	0	1,345

INTERSECTION VOLUME DEVELOPMENT

SR 8 / Business US 29 (Winder Highway) at Circle Road AM PEAK HOUR

Description	Circle Rd <u>Northbound</u>			Circle Rd <u>Southbound</u>			SR 8 / US 29 (Winder Hwy) <u>Eastbound</u>			SR 8 / US 29 (Winder Hwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	0	0	0	6	0	76	41	201	0	0	644	11
Pedestrians	0			0			0			1		
Conflicting Pedestrians	0		1	1		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	7%	2%	2%	3%	2%
Peak Hour Factor	0.25			0.85			0.77			0.90		
Adjustment												
Adjusted 2015 Volumes	0	0	0	6	0	76	41	201	0	0	644	11
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	0	0	0	6	0	82	44	217	0	0	694	12
Project Trips												
Trip Distribution IN						1%					9%	
Trip Distribution OUT							1%	9%				
Office Trips	0	0	0	0	0	11	1	10	0	0	100	0
Trip Distribution IN						1%					9%	
Trip Distribution OUT							1%	9%				
Retail Trips	0	0	0	0	0	1	1	7	0	0	12	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	0	0	0	12	2	17	0	0	112	0
2020 Buildout Total	0	0	0	6	0	94	46	234	0	0	806	12

PM PEAK HOUR

Description	Circle Rd <u>Northbound</u>			Circle Rd <u>Southbound</u>			SR 8 / US 29 (Winder Hwy) <u>Eastbound</u>			SR 8 / US 29 (Winder Hwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	0	0	0	39	0	75	81	634	0	0	298	18
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	6%	2%
Peak Hour Factor	0.25			0.84			0.93			0.96		
Adjustment												
Adjusted 2015 Volumes	0	0	0	39	0	75	81	634	0	0	298	18
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	0	0	0	42	0	81	87	683	0	0	321	19
Project Trips												
Trip Distribution IN						1%					9%	
Trip Distribution OUT							1%	9%				
Office Trips	0	0	0	0	0	2	10	94	0	0	19	0
Trip Distribution IN						1%					9%	
Trip Distribution OUT							1%	9%				
Retail Trips	0	0	0	0	0	4	5	47	0	0	40	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	0	0	0	0	0	6	15	141	0	0	59	0
2020 Buildout Total	0	0	0	42	0	87	102	824	0	0	380	19

INTERSECTION VOLUME DEVELOPMENT

SR 8 / Business US 29 (Winder Highway) at Broad Street / McMillan Street AM PEAK HOUR

Description	McMillan St <u>Northbound</u>			Broad St <u>Southbound</u>			SR 8 / US 29 (Winder Hwy) <u>Eastbound</u>			SR 8 / US 29 (Winder Hwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	20	149	4	8	59	98	77	113	3	4	512	15
Pedestrians	0			0			0			10		
Conflicting Pedestrians	0		10	10		0	0		0	0		0
Heavy Vehicle %	5%	2%	2%	2%	2%	2%	3%	10%	2%	2%	5%	2%
Peak Hour Factor	0.68			0.54			0.78			0.87		
Adjustment												
Adjusted 2015 Volumes	20	149	4	8	59	98	77	113	3	4	512	15
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	22	161	4	9	64	106	83	122	3	4	552	16
Project Trips												
Trip Distribution IN	1%					2%					6%	
Trip Distribution OUT							2%	6%	1%			
Office Trips	11	0	0	0	0	22	2	7	1	0	67	0
Trip Distribution IN	1%					2%					6%	
Trip Distribution OUT							2%	6%	1%			
Retail Trips	1	0	0	0	0	3	2	5	1	0	8	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	12	0	0	0	0	25	4	12	2	0	75	0
2020 Buildout Total	34	161	4	9	64	131	87	134	5	4	627	16

PM PEAK HOUR

Description	McMillan St <u>Northbound</u>			Broad St <u>Southbound</u>			SR 8 / US 29 (Winder Hwy) <u>Eastbound</u>			SR 8 / US 29 (Winder Hwy) <u>Westbound</u>		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Observed 2015 Traffic Volumes	15	107	15	26	231	60	116	489	47	10	200	16
Pedestrians	0			0			0			0		
Conflicting Pedestrians	0		0	0		0	0		0	0		0
Heavy Vehicle %	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	5%	2%
Peak Hour Factor	0.80			0.83			0.87			0.83		
Adjustment												
Adjusted 2015 Volumes	15	107	15	26	231	60	116	489	47	10	200	16
Annual Growth Rate	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%	1.5%
Growth Factor	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077	1.077
New Road Adjustment												
Other Proposed Developments												
2020 Background Traffic	16	115	16	28	249	65	125	527	51	11	215	17
Project Trips												
Trip Distribution IN	1%					2%					6%	
Trip Distribution OUT							2%	6%	1%			
Office Trips	2	0	0	0	0	4	21	63	10	0	13	0
Trip Distribution IN	1%					2%					6%	
Trip Distribution OUT							2%	6%	1%			
Retail Trips	4	0	0	0	0	9	11	32	5	0	27	0
Pass-By Trips	0	0	0	0	0	0	0	0	0	0	0	0
Total Project Trips	6	0	0	0	0	13	32	95	15	0	40	0
2020 Buildout Total	22	115	16	28	249	78	157	622	66	11	255	17

Appendix F

Programmed Project Fact Sheets

Short Title

SUGARLOAF PARKWAY EXTENSION: PHASE 2 - NEW ALIGNMENT FROM SR 316 EAST OF LAWRENCEVILLE TO SR 20 (BUFORD DRIVE / MALL OF GEORGIA PARKWAY) NEAR INTERSECTION WITH SR 324 (GRAVEL SPRINGS ROAD)

GDOT Project No.

0006924

Federal ID No.

CSSTP-0006-00(924)

Status

Programmed

Service Type

Roadway / General Purpose Capacity

Sponsor

Gwinnett County

Jurisdiction

Regional - Northeast

Analysis Level

In the Region's Air Quality Conformity Analysis

Existing Thru Lane

0

Planned Thru Lane

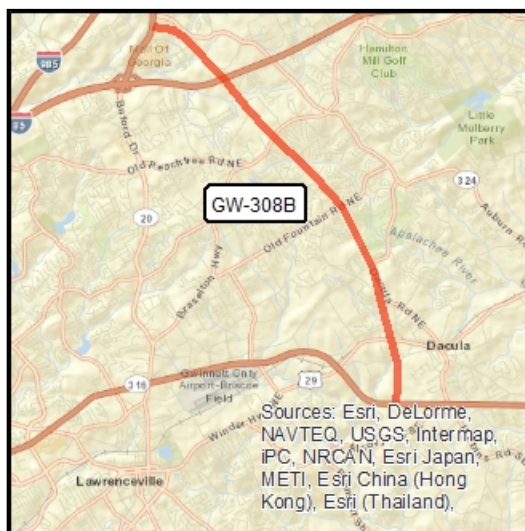
4

Network Year

2040

Corridor Length

8.5 miles



Detailed Description and Justification

This Buford/Dacula/East-Cross County Connector project consists of constructing a new 8.5 miles roadway from SR 316 east of Lawrenceville to SR 20 (Buford Dr.). The road will include a 4 lane divided highway with a raised median, bicycle and pedestrian facilities, turn lanes as well as grade separation at SR 20, I-85, SR 124, Old Fountain Rd., Old Peachtree Rd, Fence Rd, SR 8, and SR 316. The project will add need roadway capacity and address peak period congestion in the northern part of the county experiencing rapid population and employment growth.

Phase Status & Funding Information		Status	FISCAL YEAR	TOTAL PHASE COST	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
					FEDERAL	STATE	BONDS	LOCAL/PRIVATE
PE	Local Jurisdiction/Municipality Funds	AUTH	2006	\$10,000,000	\$0,000	\$0,000	\$0,000	\$10,000,000
PE-OV	STP - Statewide Flexible (GDOT)	AUTH	2011	\$50,000	\$40,000	\$10,000	\$0,000	\$0,000
ROW	Local Jurisdiction/Municipality Funds	AUTH	2010	\$17,000,000	\$0,000	\$0,000	\$0,000	\$17,000,000
ROW	Federal Earmark Funding		2015	\$5,624,375	\$4,499,500	\$0,000	\$0,000	\$1,124,875
ROW	Local Jurisdiction/Municipality Funds		2015	\$30,542,625	\$0,000	\$0,000	\$0,000	\$30,542,625
UTL	Local Jurisdiction/Municipality Funds		LR 2031-2040	\$10,355,000	\$0,000	\$0,000	\$0,000	\$10,355,000
CST	General Federal Aid 2020-2040		LR 2031-2040	\$179,647,295	\$143,717,836	\$35,929,459	\$0,000	\$0,000
				\$253,219,295	\$148,257,336	\$35,939,459	\$0,000	\$69,022,500

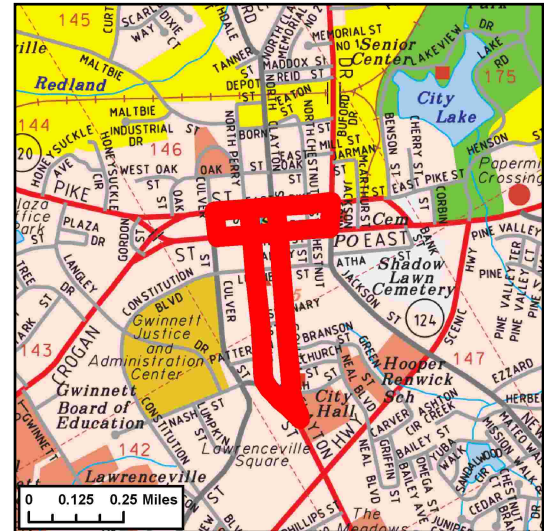
SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquisition
 UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases



For additional information about this project, please call (404) 463-3100 or email transportation@atlantaregional.com.



Short Title	DOWNTOWN LAWRENCEVILLE PEDESTRIAN IMPROVEMENTS & ONE-WAY PAIR CONVERSION
GDOT Project No.	0008963
Federal ID No.	CSSTP-0008-00(963)
Status	Programmed
Service Type	Last Mile Connectivity / Complete Street Retrofit
Sponsor	City of Lawrenceville
Jurisdiction	Gwinnett County
Analysis Level	In the Region's Air Quality Conformity Analysis
Existing Thru Lane	N/A
Planned Thru Lane	N/A



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Network Year	2020
Corridor Length	TBD miles

Detailed Description and Justification

This project will provide for pedestrian improvements in downtown Lawrenceville and will also include the conversion of SR 20/Clayton Street and SR 20/Perry Street to a two-way operation. Clayton Street and Perry Street exist as a north-south one-way pair that is currently designated as SR 20. Clayton Street consists of three (3) northbound travel lanes and Perry Street consists of three (3) southbound travel lanes. The proposed project will convert Perry Street and Clayton Street from three (3) one-way travel lanes to two-way operation. The typical section for two-way operation will consist of one (1) through-lane in each direction and one (1) center two-way left-turn lane. Existing signals along Perry and Clayton will be re-configured at Nash Street, Luckie Street, Crogan Street, and Pike Street to accommodate the two-way operation.

In order to convert Perry Street and Clayton Street, these two facilities will need to be removed from the state system. GDOT has committed to re-designating SR 20 around the Downtown area by using SR 124/Scenic Highway to SR 8/124/East Crogan Street as the re-designated SR 20 route. These re-designations will occur separate from this project and be conducted by GDOT staff prior to project implementation.

Phase Status & Funding Information		Status	FISCAL YEAR	TOTAL PHASE COST	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
					FEDERAL	STATE	BONDS	LOCAL/PRIVATE
PE	STP - Urban (>200K) (ARC)	AUTH	2009	\$577,500	\$461,960	\$0,000	\$0,000	\$115,540
ROW	Local Jurisdiction/Municipality Funds	AUTH	2015	\$1,867,000	\$0,000	\$0,000	\$0,000	\$1,867,000
CST	STP - Urban (>200K) (ARC)		2017	\$4,252,796	\$2,628,240	\$0,000	\$0,000	\$1,624,556
				\$6,697,296	\$3,090,200	\$0,000	\$0,000	\$3,607,096

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquisition
 UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases

Short Title

SR 20 (BUFORD DRIVE) WIDENING FROM SR 124 (BRASELTON HIGHWAY) TO HURRICANE SHOALS ROAD

GDOT Project No.

TBD

Federal ID No.

N/A

Status

Long Range

Service Type

Roadway / General Purpose Capacity

Sponsor

GDOT

Jurisdiction

Gwinnett County

Analysis Level

In the Region's Air Quality Conformity Analysis

Existing Thru Lane

4

Planned Thru Lane

6

Network Year

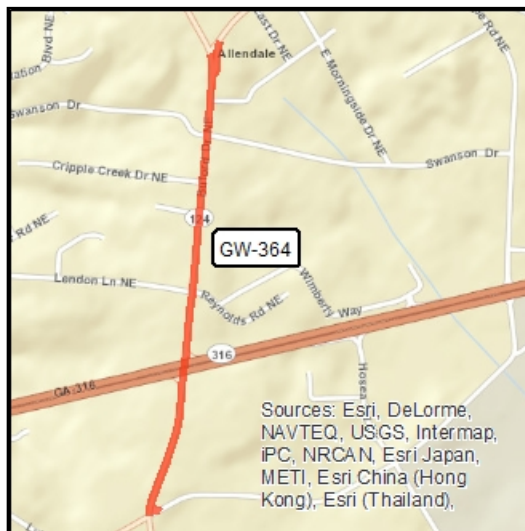
2030

Corridor Length

1.2 miles

Detailed Description and Justification

This project will widen SR 20 (Buford Drive) from SR 124 (Braselton Highway) to Hurricane Shoals Road from 4 to 6 lanes.



Phase Status & Funding Information		Status	FISCAL YEAR	TOTAL PHASE COST	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
					FEDERAL	STATE	BONDS	LOCAL/PRIVATE
ALL	General Federal Aid 2020-2040		LR 2020-2030	\$16,400,000	\$13,100,000	\$3,300,000	\$0,000	\$0,000
				\$16,400,000	\$13,100,000	\$3,300,000	\$0,000	\$0,000

SCP: Scoping PE: Preliminary engineering / engineering / design / planning PE-OV: GDOT oversight services for engineering ROW: Right-of-way Acquisition
 UTL: Utility relocation CST: Construction / Implementation ALL: Total estimated cost, inclusive of all phases



For additional information about this project, please call (404) 463-3100 or email transportation@atlantaregional.com.



Short Title

GWINNETT COUNTY ATMS/ITS INFRASTRUCTURE EXPANSION - SR 316 FROM SR 20 INTERCHANGE TO BARROW COUNTY LINE (APALACHEE RIVER)

GDOT Project No.

0013325

Federal ID No.

N/A

Status

Programmed

Service Type

Roadway / Operations & Safety

Sponsor

Gwinnett County

Jurisdiction

Gwinnett County

Analysis Level

Exempt from Air Quality Analysis (40 CFR 93)

Existing Thru Lane

N/A

Planned Thru Lane

N/A



Network Year

2020

Corridor Length

TBD miles

Detailed Description and Justification

In addition to the ATMS/ITS infrastructure already in place, the proposed CMAQ-funded ATMS/ITS expansion enables critical monitoring ability of almost every major travel corridor in Gwinnett County, significantly improving travel in the northeast Atlanta region. Traffic signalization and intersection improvement projects are designed to reduce traffic congestion, increase travel speeds, and/or reduce delay thus meeting both goals of the CMAQ program: decreasing congestion and reducing air pollution. Interconnecting traffic signals improves both peak and off peak travel speeds and reduces congestion at intersections. Fiber optic cable installation for traffic signal optimization will occur along four major travel corridors in Gwinnett County: Old Peachtree Road, from North Brown Road to Sugarloaf Parkway; Ronald Reagan Parkway, from SR 124 to US 29; Five Forks Trickum Road, from Sugarloaf Parkway to Rockbridge Road; SR 316, from Hi-Hope Road to Barrow County line.

Phase Status & Funding Information		Status	FISCAL YEAR	TOTAL PHASE COST	BREAKDOWN OF TOTAL PHASE COST BY FUNDING SOURCE			
					FEDERAL	STATE	BONDS	LOCAL/PRIVATE
PE	STP - Urban (>200K) (ARC)		2015	\$76,875	\$61,500	\$0,000	\$0,000	\$15,375
CST	Congestion Mitigation & Air Quality Improvement (CMAQ)		2017	\$2,005,800	\$1,604,600	\$0,000	\$0,000	\$401,200
				\$2,082,675	\$1,666,100	\$0,000	\$0,000	\$416,575

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Available Upon Request
Synchro Capacity Analyses
Raw Traffic Count Data